



**H30SI-12 PRETENSIONED
PRESTRESSED CONCRETE
BEAM BRIDGE STANDARDS**

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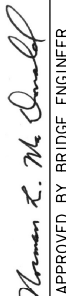

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STRUCTURAL RESISTANCE LEVEL-1 (SRL-1) REPLACES THE 50 TON STEEL PILE DESIGNATION.

STRUCTURAL RESISTANCE LEVEL-2 (SRL-2) REPLACES THE 75 TON STEEL PILE DESIGNATION.

FOR MORE INFORMATION ON STRUCTURAL RESISTANCE LEVELS (SRL-1 & SRL-2), SEE THE BRIDGE DESIGN MANUAL, LOCATED ON THE IOWA DEPARTMENT OF TRANSPORTATIONS, OFFICE OF BRIDGES AND STRUCTURES, WEBSITE.

REVISED 05-13 - THE GENERAL NOTES AND SPECIFICATIONS MOVED TO STANDARD SHEET H30-01A-06. SRL NOTATION ADDED.

| | | | |
|-------------------------------|--|--|-------------|
| 05-13 LATEST REVISION DATE |  APPROVED BY BRIDGE ENGINEER |  | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | INDEX SHEET | H30SI-01-12 |

EXAMPLES OF BRIDGE SEAT AND STEP CALCULATIONS:

THE DESIGNER SHALL SHOW ON THE PLANS THE 5 ELEVATIONS AND THE 4 STEP DIMENSIONS REQUIRED FOR THE ABUTMENT BRIDGE SEATS.

THE BOXED IN DETAILS IN THE FOLLOWING EXAMPLES SHOW HOW THE INFORMATION SHOULD BE INDICATED ON THE PLANS.

EXAMPLE NO. 1

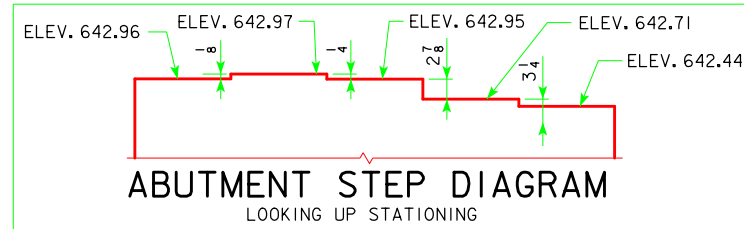
A STRAIGHT GRADE OF -3.25% WITH THE P.I. STATION OF 103+75.00 AND ELEVATION OF 653.29. THE BRIDGE LENGTH IS 80'-0" ϕ TO ϕ OF ABUTMENT BEARINGS WITH 30° SKEW RIGHT AHEAD.

| STATIONS | |
|-------------------------------------|-----------------------|
| ϕ BRIDGE STA. | = 105+85.00 |
| ϕ $\frac{1}{2}$ OF SPAN LENGTH | \pm 40.00 |
| ϕ ABUT. BRGS. | = 106+25.00 105+45.00 |

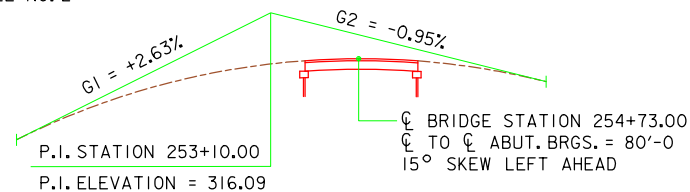
| ELEVATIONS ALONG PROFILE GRADE LINE (P.G.L. ELEV.) | |
|--|----------|
| ϕ ABUT. BRG. = 653.29 - [(105+45.00) - (103+75.00)](0.0325) | = 647.77 |
| ϕ ABUT. BRG. = 653.29 - [(106+25.00) - (103+75.00)](0.0325) | = 645.17 |

ELEVATIONS TOP OF SLAB FACING ALONG THE STATIONING
(BEAM SPACING X TAN. SK. Δ) X GRADE = (7.0) TAN 30° (0.0325) = 0.13'

| ABUTMENT NO. 1 | EXTERIOR | INTERIOR | CENTER | INTERIOR | EXTERIOR |
|------------------------------|----------|----------|--------|----------|----------|
| BEAMS | | | | | |
| PGL ELEV. | 647.77 | 647.77 | 647.77 | 647.77 | 647.77 |
| SK. Δ CORRECT | + 0.26 | + 0.13 | 0.00 | - 0.13 | - 0.26 |
| SLAB CROWN | - 0.25 | - 0.11 | 0.00 | - 0.11 | - 0.25 |
| TOP SLAB ELEV. | 647.78 | 647.79 | 647.77 | 647.53 | 647.26 |
| - "U" (4'-9 $\frac{1}{8}$ ") | - 4.82 | - 4.82 | - 4.82 | - 4.82 | - 4.82 |
| BR. SEAT ELEV. | 642.96 | 642.97 | 642.95 | 642.71 | 642.44 |



EXAMPLE NO. 2

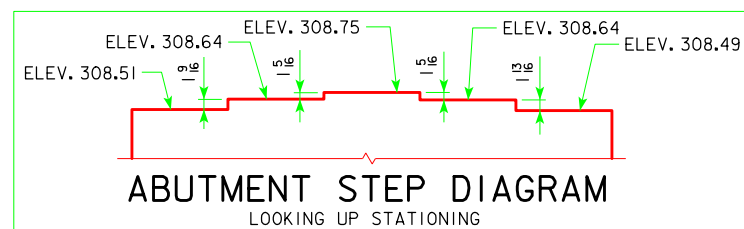


FROM SHEET H30SI-1-12 { LENGTH OF VERTICAL CURVE = (20000)(0.0358) = 716 FEET
M.O. = (0.0358)(716)($\frac{1}{8}$) = 3.204 FEET

| STATIONS | | P.I. STA. | 253+10.00 |
|-------------------------------------|-----------------------|--------------------------------------|---------------------|
| ϕ BRIDGE STA. | = 254+73.00 | | |
| ϕ $\frac{1}{2}$ OF SPAN LENGTH | \pm 40.00 | \pm ($\frac{1}{2}$)(LENGTH V.C.) | $\frac{3+58.00}{2}$ |
| ϕ ABUT. BRGS. | = 255+13.00 254+33.00 | P.C. STA. | 249+52.00 |
| | | P.T. STA. | 256+68.00 |

ELEVATIONS TOP OF SLAB FACING ALONG THE STATIONING
(BEAM SPACING X TAN. SK. Δ) = (7.0) TAN 15° = 1.88'

| ABUTMENT NO. 1 | EXTERIOR | INTERIOR | CENTER | INTERIOR | EXTERIOR |
|-------------------------------|-----------|-----------|-----------|-----------|-----------|
| BEAMS | | | | | |
| STATION | 254+36.75 | 254+34.88 | 254+33.00 | 254+31.12 | 254+29.25 |
| PGL ELEV. | 313.55 | 313.54 | 313.54 | 313.54 | 313.53 |
| SLAB CROWN | - 0.25 | - 0.11 | 0.00 | - 0.11 | - 0.25 |
| TOP SLAB ELEV. | 313.30 | 313.43 | 313.54 | 313.43 | 313.28 |
| - "U" (4'-9 $\frac{7}{16}$ ") | - 4.79 | - 4.79 | - 4.79 | - 4.79 | - 4.79 |
| BR. SEAT ELEV. | 308.51 | 308.64 | 308.75 | 308.64 | 308.49 |



GENERAL NOTES:

THE H30SI-12 BRIDGE STANDARDS, IF PROPERLY USED, PROVIDE THE STRUCTURAL PLANS NECESSARY TO CONSTRUCT SINGLE SPAN 30' ROADWAY PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES WITH LENGTHS OF 46'-8", 55'-0", 67'-6", 80'-0", 90'-0", 100'-0", AND 110'-0".

THESE BRIDGES MAY BE BUILT ON A 0°, 15° OR 30° SKEW. THESE PLANS SHOW THE BRIDGES SKEWED IN ONE DIRECTION, BUT ALL DIMENSIONS AND DETAILS WOULD BE THE SAME FOR THE OPPOSITE SKEW.

NOTE THAT WHEN APPROACH PAVEMENT IS TO BE PLACED, THE TEMPORARY PAVING BLOCKS SHALL BE REMOVED AND A PROPER JOINT FOR EXPANSION SHALL BE PROVIDED BETWEEN THE BRIDGE AND THE APPROACH PAVING.

THE ABUTMENTS FOR THESE STANDARDS HAVE BEEN DESIGNED FOR FRICTION OR POINT BEARING PILES. IT IS NECESSARY THAT THE LENGTH OF THE ABUTMENT PILES BE DESIGNATED ON THE FRONT SHEET OF THE PLANS.

THE INTEGRAL ABUTMENTS FOR THESE H30SI STANDARDS HAVE BEEN DESIGNED FOR THE USE OF VARIOUS TYPES OF PILE FOOTINGS AS FOLLOWS.

- INTEGRAL ABUTMENTS: TIMBER PILES (LIMITED BY BRIDGE LENGTH) OR HP10x57 PILES AT BRIDGE DESIGN MANUAL (BDM) ARTICLE 6.2.6.1 STRUCTURAL RESISTANCE LEVEL-1 (SRL-1)

STRUCTURAL RESISTANCE LEVEL-1 (SRL-1) REPLACES THE 50 TON STEEL PILE DESIGNATION.

FOR MORE INFORMATION ON SRL-1 AND SRL-2, SEE THE BRIDGE DESIGN MANUAL, LOCATED ON THE IOWA DEPARTMENT OF TRANSPORTATION, OFFICE OF BRIDGES AND STRUCTURES WEB SITE.

THESE STANDARDS GIVE MOST OF THE INFORMATION NECESSARY TO BUILD THESE BRIDGES ON EITHER A CREST VERTICAL CURVE OR A STRAIGHT GRADE. BECAUSE OF THE INFINITE NUMBER OF GRADE POSSIBILITIES IT WILL BE NECESSARY TO SHOW ON THE PLANS THE ABUTMENT STEP DIMENSIONS. TO HELP IN OBTAINING THIS STEP INFORMATION SEE "EXAMPLES OF BRIDGE SEAT AND STEP CALCULATIONS" ON THIS SHEET.

PROVIDE TOP OF SLAB ELEVATIONS AND WING ELEVATIONS A, B, AND C AS NOTED ON THE STANDARD SHEETS (LONGITUDINAL SECTION).

KEYWAY DIMENSIONS SHOWN ON THE PLANS ARE BASED ON NOMINAL DIMENSIONS UNLESS STATED OTHERWISE. IN ADDITION, THE BEVEL USED ON THE KEYWAY SHALL BE LIMITED TO A MAXIMUM OF 10 DEGREES FROM VERTICAL.

THESE BRIDGE PLANS LABEL ALL REINFORCING STEEL WITH ENGLISH NOTATION (5# IS $\frac{5}{8}$ INCH DIAMETER BAR). ENGLISH REINFORCING STEEL RECEIVED IN THE FIELD MAY DISPLAY THE FOLLOWING "BAR DESIGNATION". THE "BAR DESIGNATION" IS THE STAMPED IMPRESSION ON THE REINFORCING BARS, AND IS EQUIVALENT TO THE BAR DIAMETER IN MILLIMETERS.

| | | | | | | | | | |
|-----------------|----|----|----|----|----|----|----|----|----|
| ENGLISH SIZE | 3 | 4 | 5 | 6 | 7 | 8 | 9 | 10 | 11 |
| BAR DESIGNATION | 10 | 13 | 16 | 19 | 22 | 25 | 29 | 32 | 36 |

BECAUSE THESE BRIDGE STANDARDS HAVE BEEN REVISED FOR LRFD BASED ON 2012-COMPLETED IOWA STATE UNIVERSITY RESEARCH, FOR PILE FOUNDATIONS THE DESIGNER WILL NEED TO DETERMINE THE CONSTRUCTION CONTROL METHOD, CONTRACT LENGTH, AND DRIVING TARGET AND GIVE THAT INFORMATION ON THE FRONT SHEET OF THE PLANS. BRIDGE DESIGN MANUAL CADD NOTES E177, E718, E719, E818, AND E819 ARE APPROPRIATE FOR THAT PURPOSE. THE NOTES, AS WELL AS THE BRIDGE DESIGN MANUAL AND DESIGN EXAMPLES, ARE AVAILABLE ON THE OFFICE OF BRIDGES AND STRUCTURES WEB SITE: [HTTP://WWW.IOWADOT.GOV/BRIDGE/INDEX.HTM](http://www.iowadot.gov/bridge/index.htm).

THESE STANDARDS CAN BE USED FOR BRIDGES WITH OR WITHOUT EPOXY COATED REINFORCING. REINFORCING BAR LAP LENGTHS ARE BASED ON THE USE OF EPOXY COATED REINFORCING, BUT NEED NOT BE MODIFIED IF NON-COATED BARS ARE TO BE USED. THE DESIGNER SHALL SPECIFY THE APPROPRIATE BID ITEM NO. FOR THE EPOXY COATED OR NON-EPOXY COATED REINFORCING.

IT IS RECOMMENDED THAT THE EPOXY COATED REINFORCING OPTION BE USED IF IT IS ANTICIPATED THAT THE BRIDGE DECK AND/OR THE BRIDGE APPROACHES WILL BE CHEMICALLY TREATED FOR THE REMOVAL OF ICE OR SNOW.

IF EPOXY COATED BARS ARE USED IN THE DECK, THEN ALL BARS USED IN THE ABUTMENT (FOOTING AND BACKWALL) AND BARRIER RAILS SHALL BE EPOXY COATED.

CONCRETE INTERMEDIATE DIAPHRAGMS SHALL BE USED FOR OVERPASS BRIDGES. THE DESIGNER SHALL ADJUST THE CONCRETE AND REINFORCING QUANTITIES ACCORDINGLY.

DESIGN STRESSES:

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS 5th Ed, SERIES OF 2010.
REINFORCING STEEL IN ACCORDANCE WITH LRFD AASHTO SECTION 5, GRADE 60.
CONCRETE IN ACCORDANCE WITH LRFD AASHTO SECTION 5, f'_c = 4.0 KSI.
FOR 30' STANDARD PRESTRESSED CONCRETE BEAMS, SEE SHEETS H30SI-21-12, H30SI-23-12, H30SI-25-12, AND H30SI-27-12.

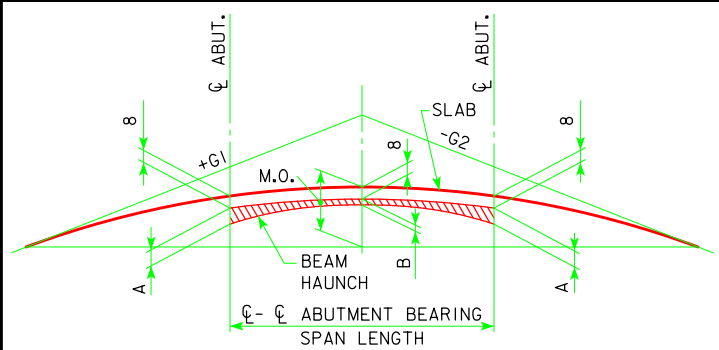
SPECIFICATIONS:

DESIGN:
AASHTO, SERIES OF 2010.

CONSTRUCTION:
IOWA DEPARTMENT OF TRANSPORTATION SPECIFICATIONS FOR HIGHWAY AND BRIDGE CONSTRUCTION, SERIES 2012, PLUS APPLICABLE GENERAL SUPPLEMENTAL SPECIFICATIONS, DEVELOPMENTAL SPECIFICATIONS, SUPPLEMENTAL SPECIFICATIONS AND SPECIAL PROVISIONS SHALL APPLY TO CONSTRUCTION WORK ON THIS PROJECT.

| | | |
|--|---|---------------------|
| 05-13 LATEST REVISION DATE <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | GENERAL NOTES & GENERAL INFORMATION | H30SI-01A-12 |

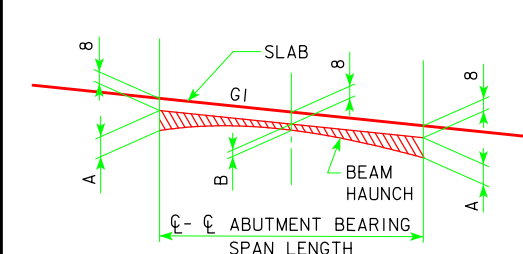
REVISED 06-12 - I.M. REQUIREMENT ADDED TO BAR CHAIR NOTE. CONCRETE SEALER AREA CHANGED AND SEALER NOTES WERE CHANGED.



| SPAN THICKNESS | 46'-8" | 55'-0" | 67'-6" | 80'-0" | 90'-0" | 100'-0" | 110'-0" |
|----------------|--------|--------|--------|--------|--------|---------|---------|
| A | 1 1/8 | 1 1/2 | 1 7/8 | 1 7/16 | 1 3/16 | 1 11/16 | 2 5/16 |
| B | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 |

LENGTH OF VERTICAL CURVE REQUIRED = $(20,000)(G1-G2)$
 $M.O. = \frac{(G1-G2)(LENGTH OF V.C.)}{8}$
 (G1-G2) IS THE ALGEBRAIC DIFFERENCE OF THE APPROACH GRADES EXPRESSED IN DECIMAL FORM. G1 NEED NOT HAVE THE SAME VALUE AS G2. MAXIMUM VALUE OF G1 OR G2 IS 5%. LENGTH OF CURVE AND M.O. ARE IN FEET.

SLAB AND HAUNCH THICKNESS AT BEAMS FOR VERTICAL CURVE

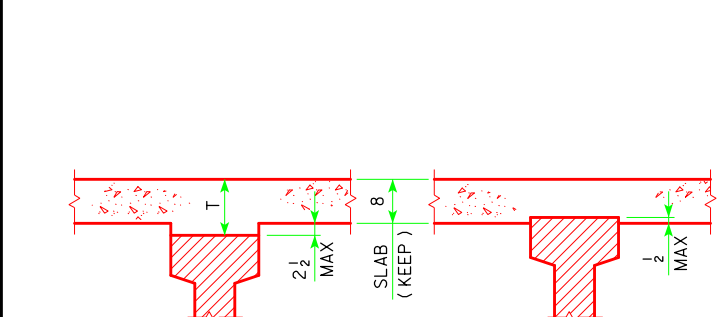


| LENGTH OF S3 x 7.5 (ABUTMENT BEAM SEAT) | |
|---|--------------------|
| BEAM BOTTOM FLANGE WIDTH | LENGTH OF S3 x 7.5 |
| 1'-5" | 1'-3 1/2" |
| 1'-8" | 1'-6 1/2" |
| 1'-10" | 1'-8 1/2" |

| SPAN THICKNESS | 46'-8" | 55'-0" | 67'-6" | 80'-0" | 90'-0" | 100'-0" | 110'-0" |
|----------------|--------|--------|--------|---------|---------|---------|---------|
| A | 1 5/16 | 1 3/4 | 2 3/8 | 1 15/16 | 1 13/16 | 2 7/16 | 3 1/4 |
| B | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 | 1/2 |

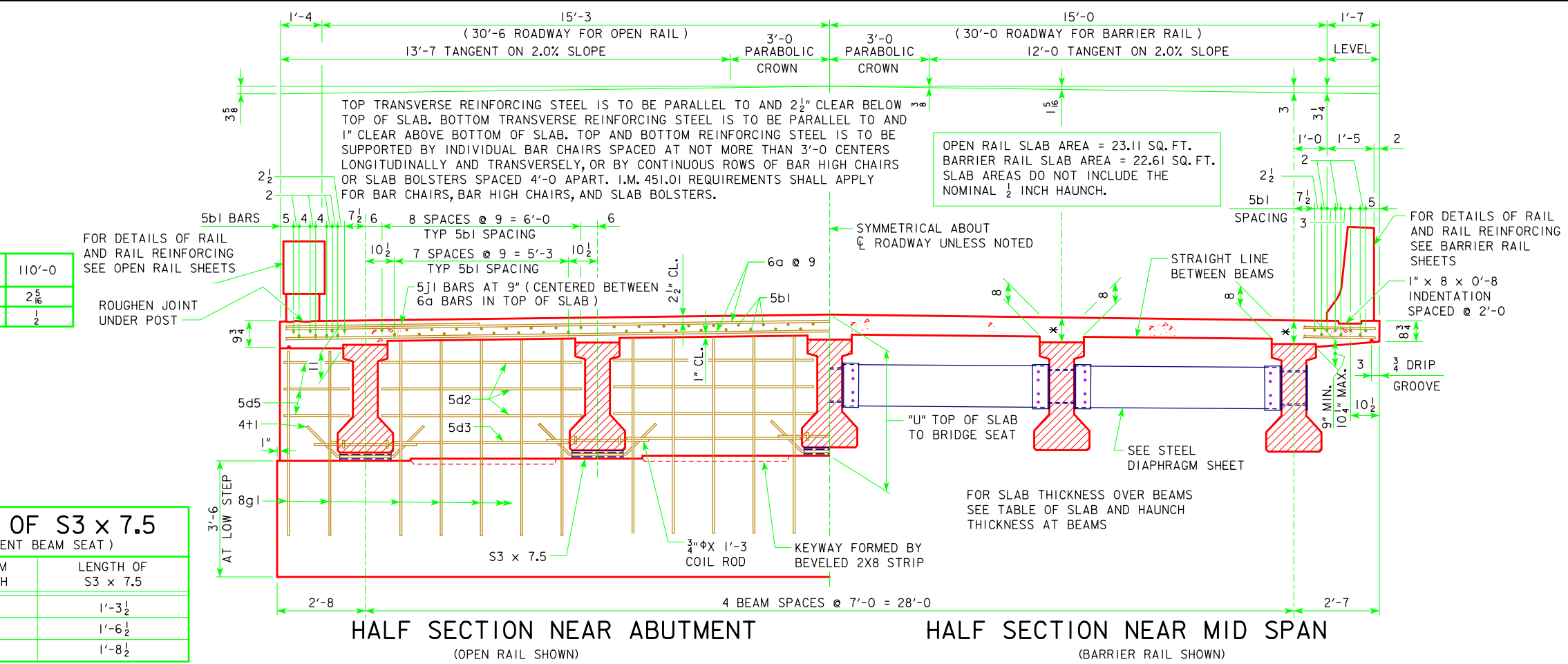
G1 MAY HAVE A + OR - SIGN. THE MINIMUM NUMERICAL VALUE OF THE GRADE IS 0.3% AND THE MAXIMUM VALUE IS 8%.

SLAB AND HAUNCH THICKNESS AT BEAMS FOR STRAIGHT GRADE



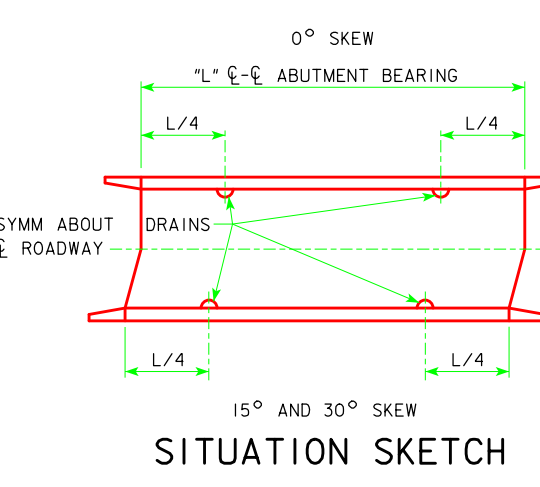
SLAB THICKNESS DETAIL

NOTE: THE SLAB THICKNESS T AT THE BEAMS, (8" SLAB PLUS HAUNCH), IS BASED ON THE ANTICIPATED BEAM CAMBER REMAINING AFTER PLACING THE SLAB, BUT IS NOT GUARANTEED FOR CONSTRUCTION. IF BEAM IS UNDER CAMBERED INCREASE THE HAUNCH THICKNESS OVER THE BEAM AT THE MIDPOINT OF THE SPANS (POINT B). IF THE BEAM IS OVER CAMBERED DECREASE THE HAUNCH THICKNESS OVER THE BEAM AT THE MIDPOINT OF THE SPANS (POINT B) TO A MAXIMUM OF 1/2" EMBEDMENT IN THE SLAB. IF MORE THAN 1/2" EMBEDMENT IS REQUIRED OR IF THE HAUNCH EXCEEDS 2 1/2", THE GRADE LINE IS TO BE REVISED.

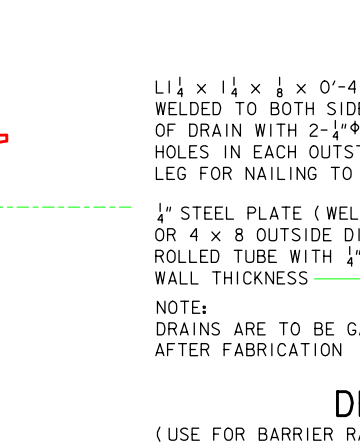


CONCRETE SEALER SHALL BE APPLIED TO BOTH SIDES OF BRIDGE SLAB ON THE TOP, EDGE OF SLAB AND UNDER THE SLAB. THE CONCRETE SEALER SHALL ALSO BE APPLIED TO THE OPEN RAIL ON THE TOP, TRAFFIC FACE SIDE, BOTTOM OF RAIL, AND ON ALL SIDES OF THE OPEN RAIL POSTS.

THE CONCRETE SEALER LIMITS ARE SHOWN IN THE DETAIL AND SHALL APPLY TO THE FULL LENGTH OF BRIDGE. CONCRETE SEALER SHALL BE APPLIED IN ACCORDANCE WITH ARTICLE 2403.03, P, 3, OF THE STANDARD SPECIFICATIONS.



CONCRETE SEALER LIMITS FOR OPEN RAILS



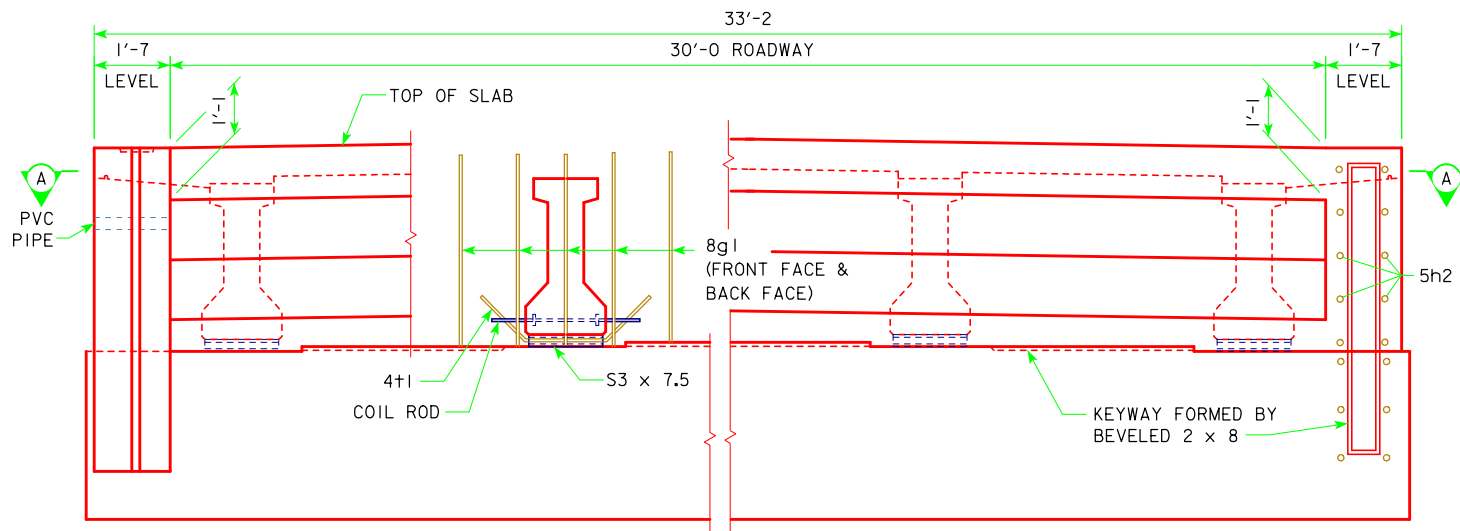
DRAIN DETAILS

NOTE: SEE SITUATION SKETCH FOR LOCATION OF DRAINS

NOTE: DRAINS ARE TO BE GALVANIZED AFTER FABRICATION

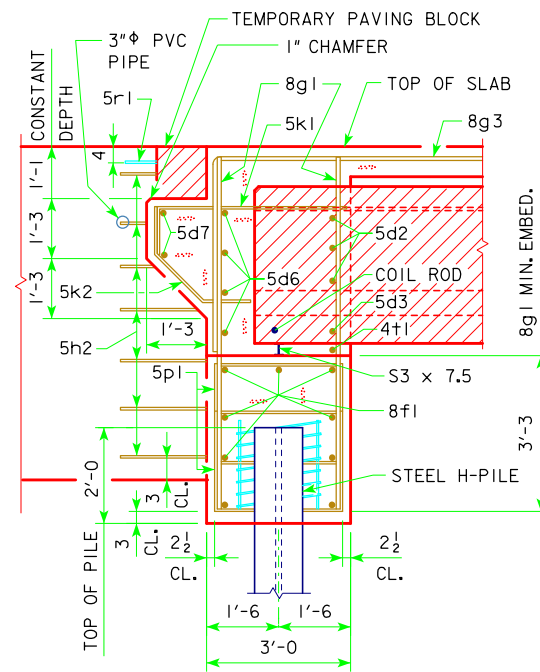
- ### GENERAL NOTES:
- THESE BRIDGES ARE DESIGNED FOR HL-93 LOADING PLUS 20 LBS. PER SQ. FT. OF ROADWAY FOR FUTURE WEARING SURFACE.
 - SLAB THICKNESS INCLUDES 1/2" INTEGRAL WEARING SURFACE.
 - CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR SHALL BE 2" UNLESS OTHERWISE NOTED OR SHOWN. ALL REINFORCING BARS ARE TO BE SECURELY WIRED IN PLACE AND ADEQUATELY SUPPORTED ON BAR CHAIRS BEFORE CONCRETE IS PLACED. I.M. 451.01 REQUIREMENTS SHALL APPLY FOR BAR CHAIRS.
 - ALL PRESTRESSED BEAMS ARE TO BE SET VERTICAL.
 - FORMS FOR THE SLAB AND RAILS ARE TO BE SUPPORTED BY THE PRESTRESSED BEAMS.
 - COST OF DRAINS IS TO BE INCLUDED IN PRICE BID FOR STRUCTURAL STEEL.
 - THE ABUTMENT DIAPHRAGM CONCRETE IS TO BE PLACED MONOLITHIC WITH THE FLOOR SLAB.
 - ALL REINFORCING STEEL IS TO BE GRADE 60.
 - COST OF BEARING MATERIAL IS TO BE INCLUDED IN THE PRICE BID FOR PRETENSIONED, PRESTRESSED CONCRETE BEAM.

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| 06-12 LATEST REVISION DATE | <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | IOWADOT Highway Division | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| SUPERSTRUCTURE DETAILS | | H30S1-02-12 | |



PART REAR ELEVATION AT ABUTMENT

NOTE: BARRIER RAIL NOT SHOWN.
(SHOWN FOR SOLID BARRIER RAIL)



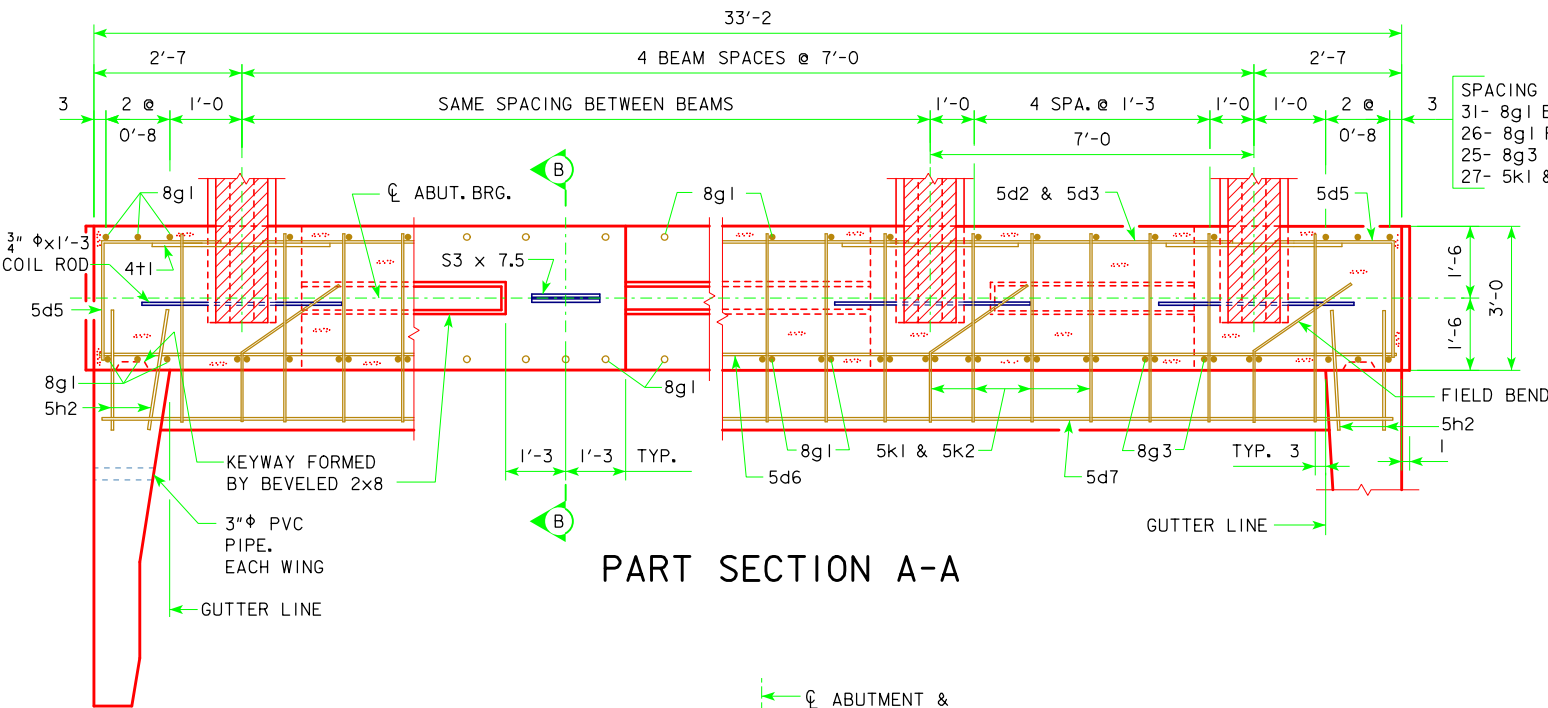
PART SECTION B-B

NOTE:
THE SPIRAL AT THE TOP OF EACH PILE TO BE 7 TURNS OF NO. 2 BAR, 21" DIAMETER, 3" PITCH WITH 2 - L₇ x 7/8 x 1/8 SPACERS PUNCHED TO HOLD SPIRAL.

ABUTMENT PILE SPACING

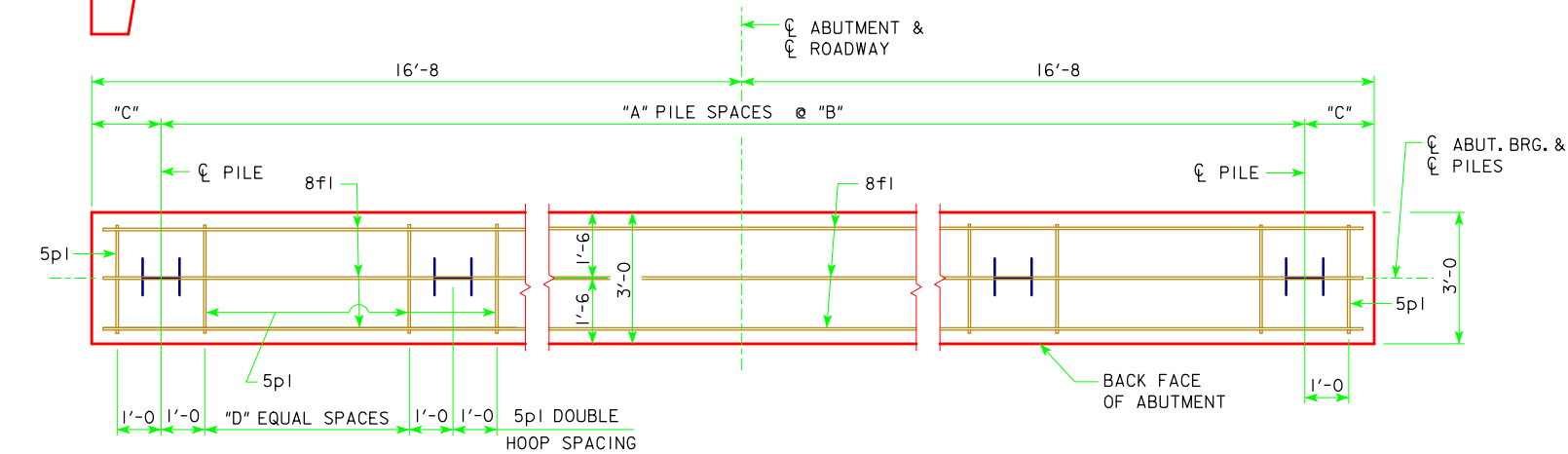
| DIMENSION OR NO. | CL TO CL ABUTMENT BEARING | | |
|-----------------------------------|---------------------------|-------|-------|
| | 46'-8 | 55'-0 | 67'-6 |
| "A" PILE SPACES | 4 | 4 | 5 |
| "B" (FT. - IN.) | 7'-6 | 7'-6 | 6'-0 |
| "C" (FT. - IN.) | 1'-8 | 1'-8 | 1'-8 |
| "D" EQUAL SPACES | 6 | 6 | 4 |
| NO. OF PILES PER ABUT. | 5 | 5 | 6 |
| PU, STRENGTH I DESIGN LOAD (KIPS) | 129 | 139 | 133 |

NOTE: PU, STRENGTH I DESIGN LOAD (KIPS) IS NOT THE VALUE USED IN THE FIELD FOR DRIVING PILES.



PART SECTION A-A

NOTE:
PLACE 5h2 BAR AT 1:6 SLOPE TO MATCH TRAFFIC SIDE OF ABUTMENT WING FACE (BOTH SIDES TYPICAL).



ABUTMENT PILE PLAN

ABUTMENT NOTES:

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.

ABUTMENT PILES SHALL BE DRIVEN TO VALUES SHOWN IN DESIGN PLANS.

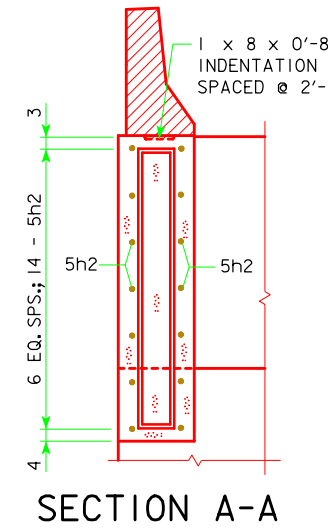
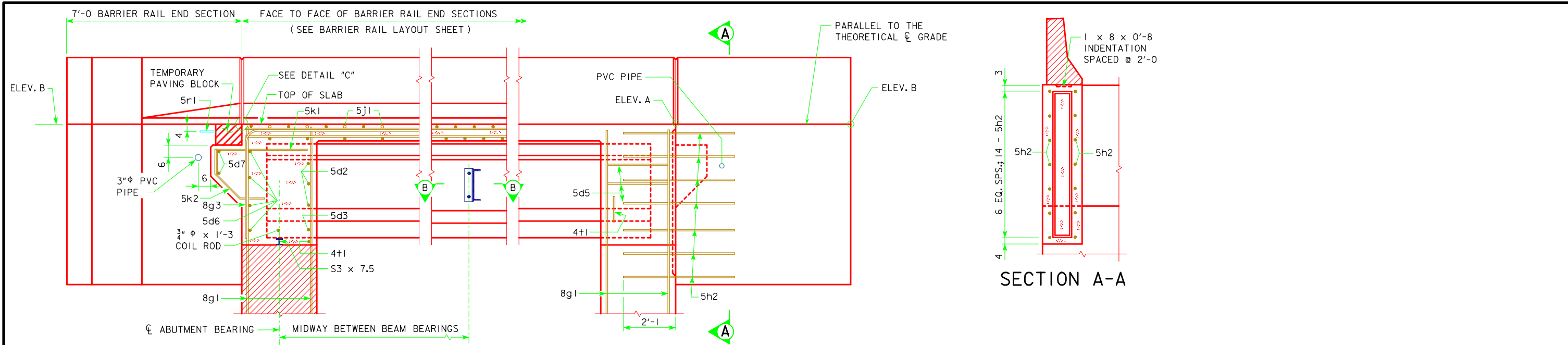
PLACE 5h2 BAR AT 1:6 SLOPE TO MATCH TRAFFIC SIDE OF ABUTMENT WING FACE. (BOTH SIDES TYPICAL)

BARRIER RAIL NOT SHOWN IN DETAILS.

IF ROCK IS CLOSER THAN 15' BELOW ABUTMENT FOOTING, SPECIAL ANALYSIS MAY BE REQUIRED.

REVISED 05-13 - REVISION FOR LRFD PILE DESIGN.

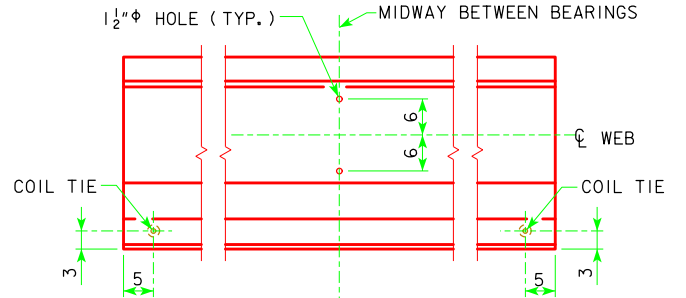
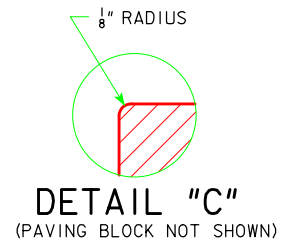
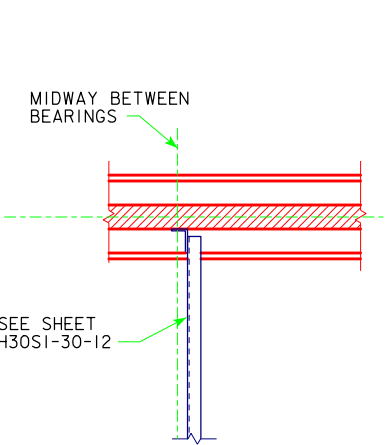
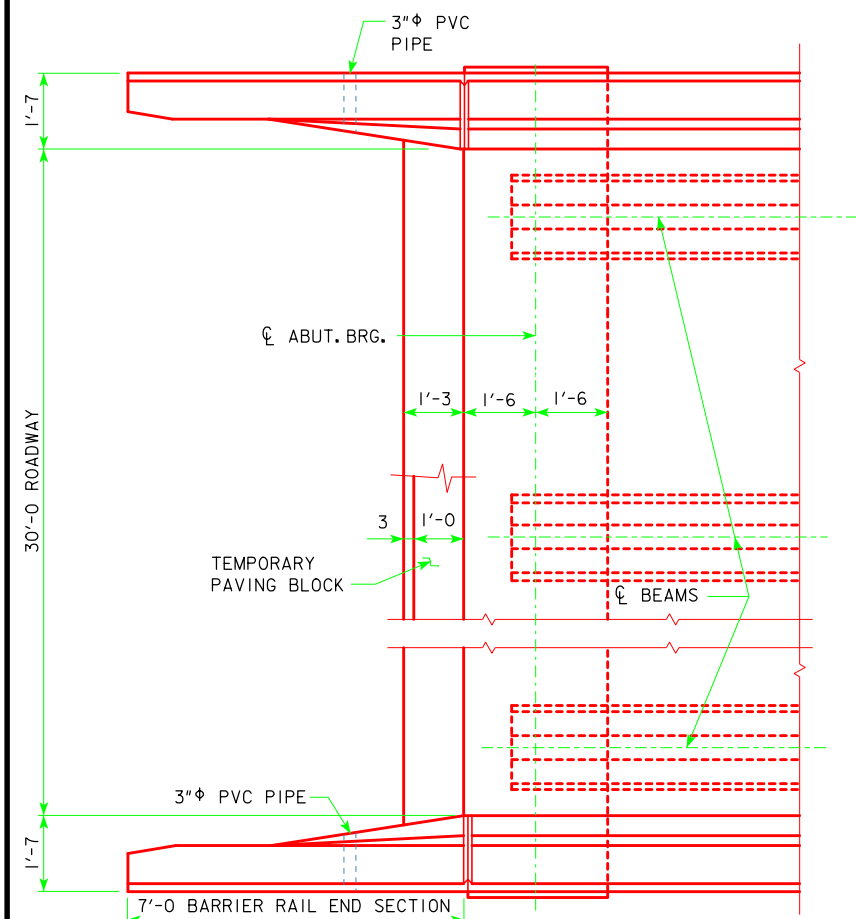
| | | |
|--|---|--------------------|
| 05-13 LATEST REVISION DATE <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | ABUTMENT DETAILS 0° SKEW A & B BEAMS | H30S1-03-12 |



PART LONGITUDINAL SECTION NEAR GUTTER
(FOR DETAILS OF INTERMEDIATE DIAPHRAGM SEE SHEET H30SI-30-12)

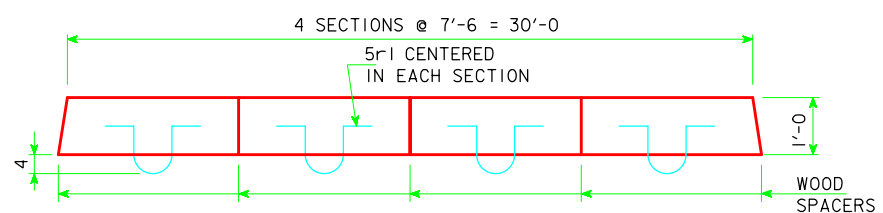
PART END VIEW AT ABUTMENT
PROVIDE ELEVATIONS A AND B IN THE BRIDGE PLAN SHEETS.

NOTE: PLUG 3" ϕ PVC PIPE WITH EXPANDING FOAM PRIOR TO BACKFILLING BEHIND ABUTMENTS.



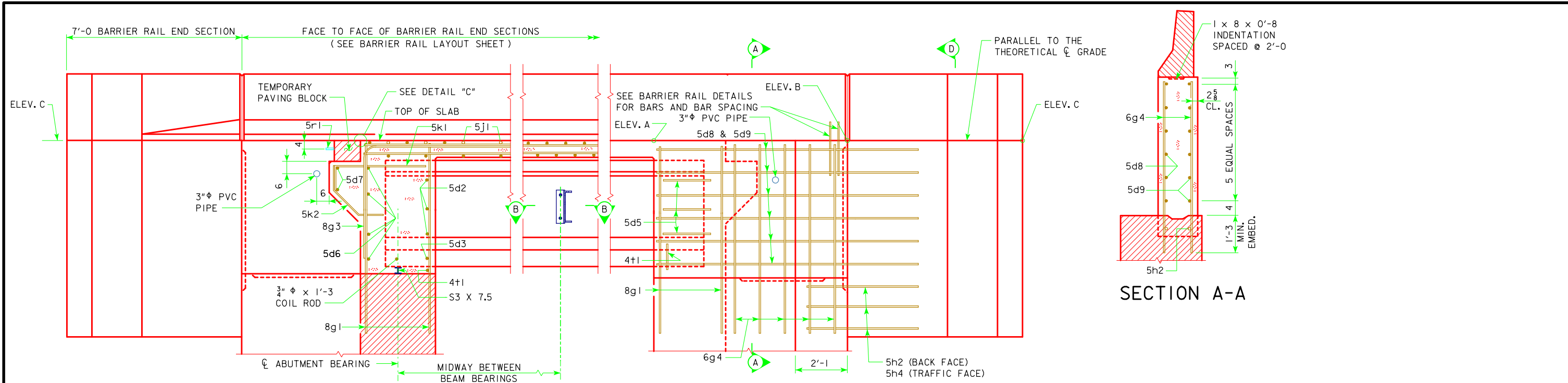
SECTION B-B

LOCATION OF BEAM COIL TIES AND STEEL DIAPHRAGM BOLT HOLES



PLAN OF TEMPORARY PAVING BLOCK
NOTE: LINE PAVING NOTCH WITH TAR PAPER BEFORE PLACING THE TEMPORARY PAVING BLOCK.

| | |
|---|---|
| LATEST REVISION DATE APPROVED BY BRIDGE ENGINEER <i>Thomas L. Mc Donald</i> | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 |
| | LONGITUDINAL SECTION H30SI-05-12 0° SKEW A & B BEAMS |



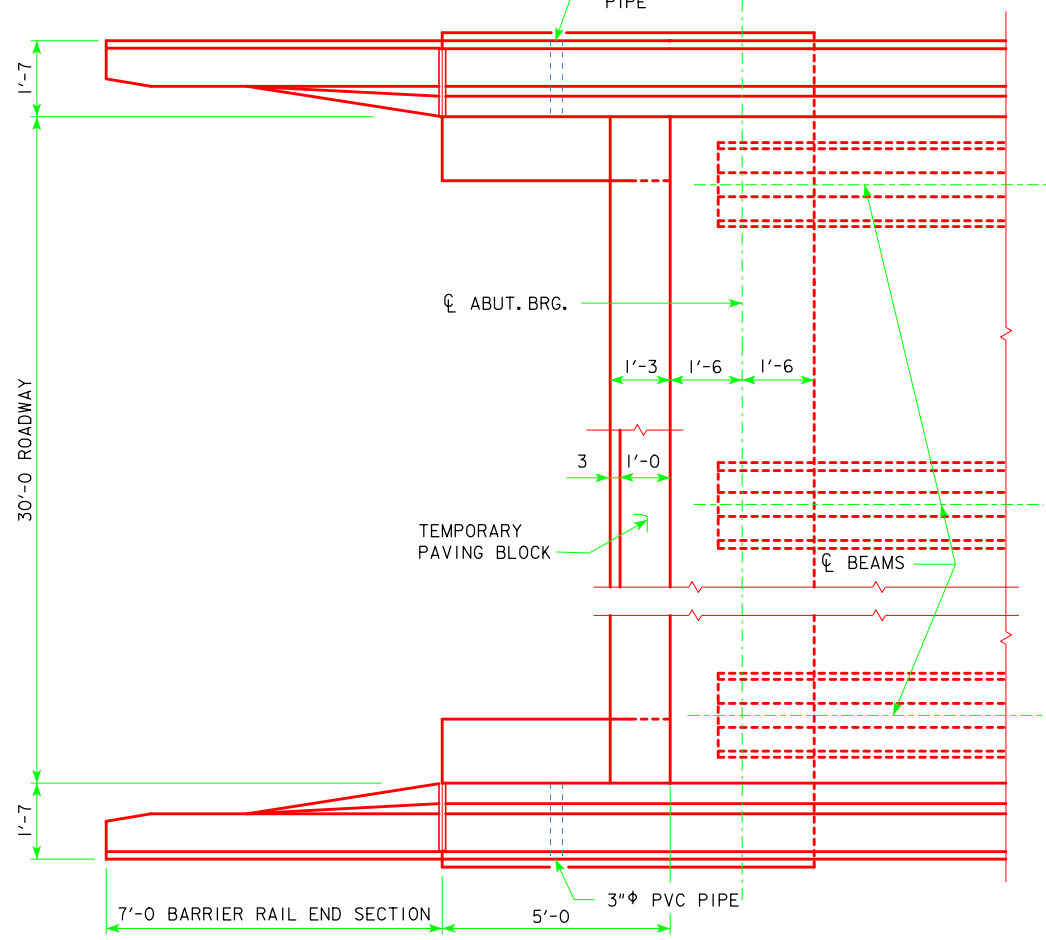
PART LONGITUDINAL SECTION NEAR GUTTER

(FOR DETAILS OF INTERMEDIATE DIAPHRAGM SEE SHEET H30SI-30-12)

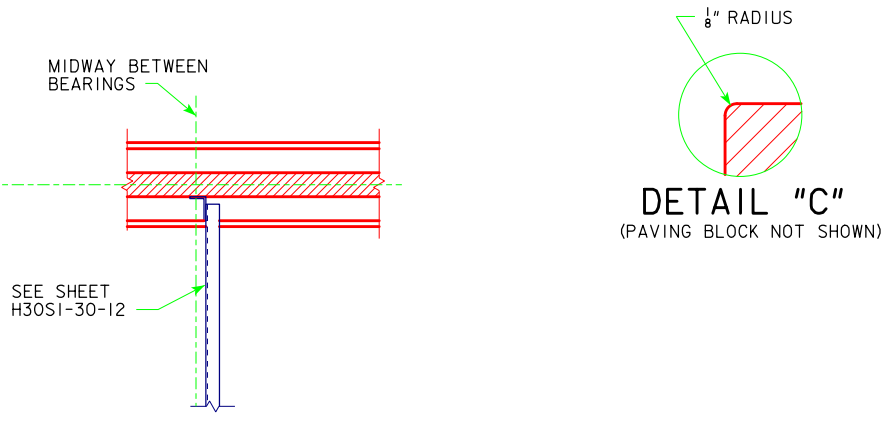
PART END VIEW AT ABUTMENT

PROVIDE ELEVATIONS A, B, AND C IN THE BRIDGE PLAN SHEETS.

NOTE: PLUG 3" PVC PIPE WITH EXPANDING FOAM PRIOR TO BACKFILLING BEHIND ABUTMENTS.

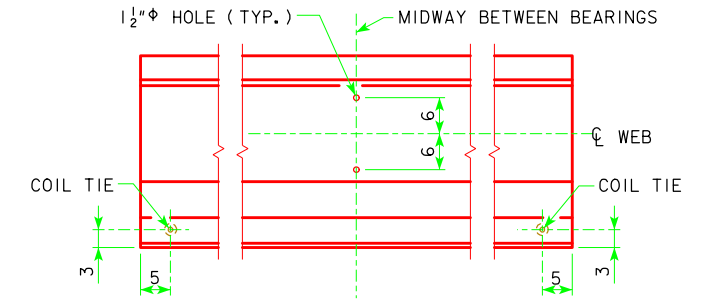


PART PLAN

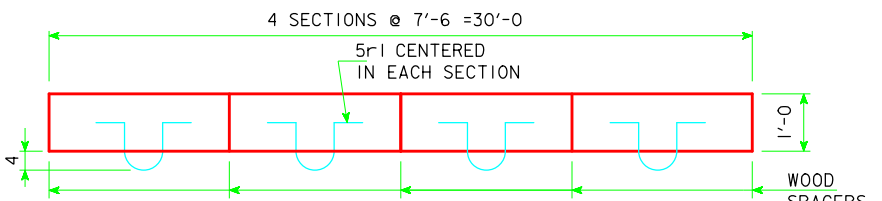


SECTION B-B

DETAIL "C"
(PAVING BLOCK NOT SHOWN)



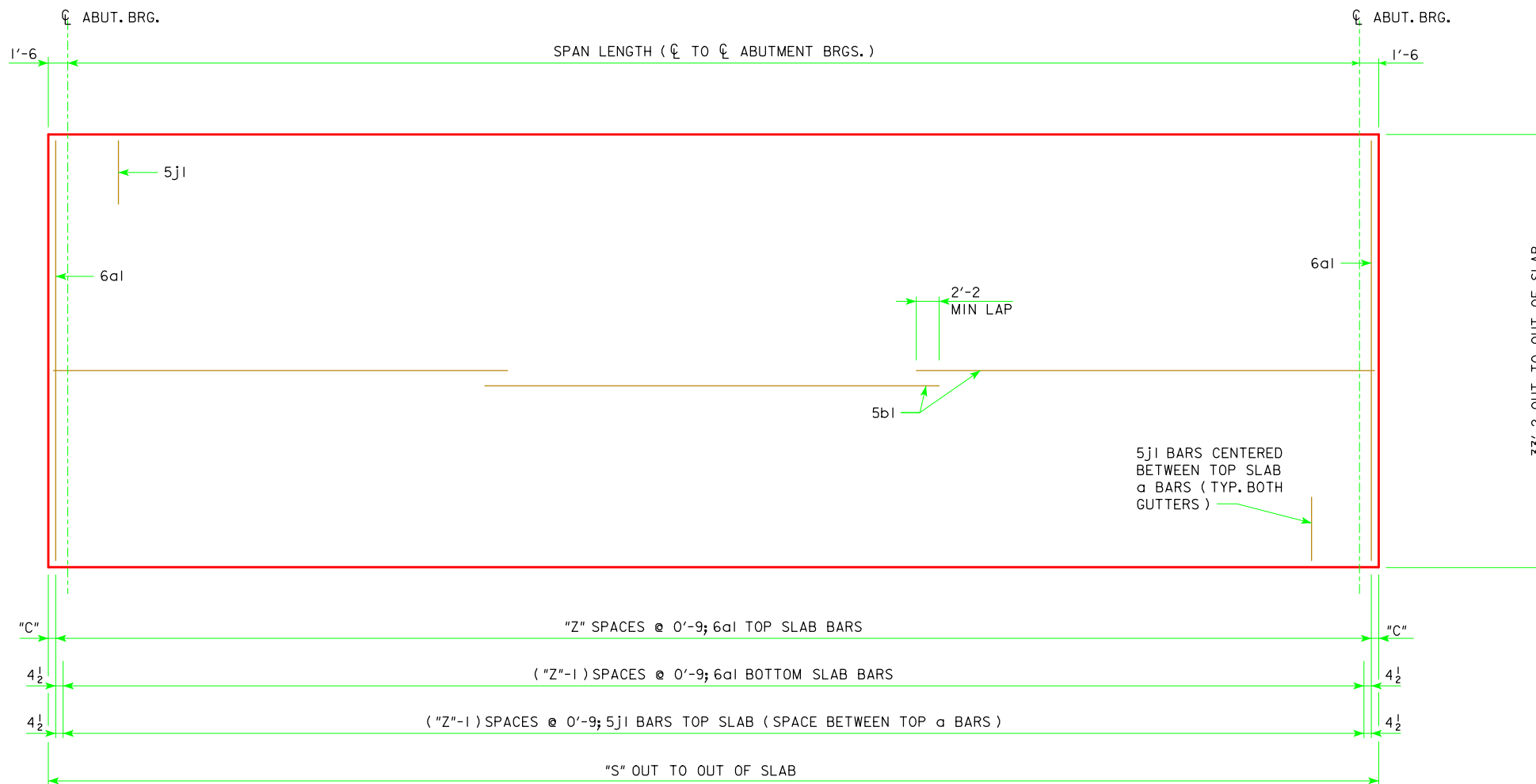
LOCATION OF BEAM COIL TIES AND STEEL DIAPHRAGM BOLT HOLES



PLAN OF TEMPORARY PAVING BLOCK

NOTE: LINE PAVING NOTCH WITH TAR PAPER BEFORE PLACING THE TEMPORARY PAVING BLOCK.

| | |
|---|---|
| LATEST REVISION DATE APPROVED BY BRIDGE ENGINEER <i>Thomas L. Mc Donald</i> | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 |
| | LONGITUDINAL SECTION H30SI-06-12 0° SKEW C & D BEAMS |



SLAB REINFORCING LAYOUT

GENERAL DATA

| SPAN LENGTH (CL - CL ABUTMENT BRGS.) | | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 |
|---|------|-----------|----------|-----------|------------|------------|-----------|-----------|
| LOCATION OF EXTREME 6a1 TOP BAR FROM END OF SLAB | "C" | 5 1/2 | 6 | 4 1/2 | 3 | 4 1/2 | 6 | 3 |
| NO. OF SPACES FOR 6a1 TOP BARS | "Z" | 65 | 76 | 93 | 110 | 123 | 136 | 150 |
| OUT TO OUT OF SLAB | "S" | 49'-8 | 58'-0 | 70'-6 | 83'-0 | 93'-0 | 103'-0 | 113'-0 |
| VERTICAL CURVE TOP OF SLAB TO ABUTMENT TOP AT CL ABUTMENT BEARING | "U" | 3'-8 1/8 | 3'-8 1/2 | 4'-3 7/8 | 4'-9 7/16 | 5'-6 3/16 | 5'-6 1/16 | 5'-7 5/16 |
| STRAIGHT GRADE TOP OF SLAB TO ABUTMENT TOP AT CL ABUTMENT BEARING | "U" | 3'-8 5/16 | 3'-8 3/4 | 4'-4 3/16 | 4'-9 15/16 | 5'-6 13/16 | 5'-7 7/16 | 5'-8 1/4 |
| SERVICE D.L. ABUTMENT REACTION (D.L. + F.W.S.) SERIVE LOADS | KIPS | 297.9 | 325.0 | 388.8 | 502.5 | 575.0 | 616.0 | 657.2 |
| SERVICE L.L. ABUTMENT REACTION (HL-93) NO IMPACT SERVICE LOADS | KIPS | 152.8 | 162.4 | 175.0 | 186.0 | 194.4 | 202.2 | 209.8 |

REVISED 05-13 - REVISION FOR LRFD PILE DESIGN.

| | | |
|--|---|--------------------|
| 05-13 LATEST REVISION DATE <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | SUPERSTRUCTURE 0° SKEW | H30SI-07-12 |

REVISED 09-14 - CORRECTED THE BARRIER RAIL REINFORCING STEEL QUANTITIES FOR ALL THE BRIDGE LENGTHS.
REVISED 05-15 - CORRECTED THE CONCRETE QUANTITIES OF THE PAVING BLOCKS FOR ALL THE BRIDGE LENGTHS. (WAS 1.5 CU.YD.)

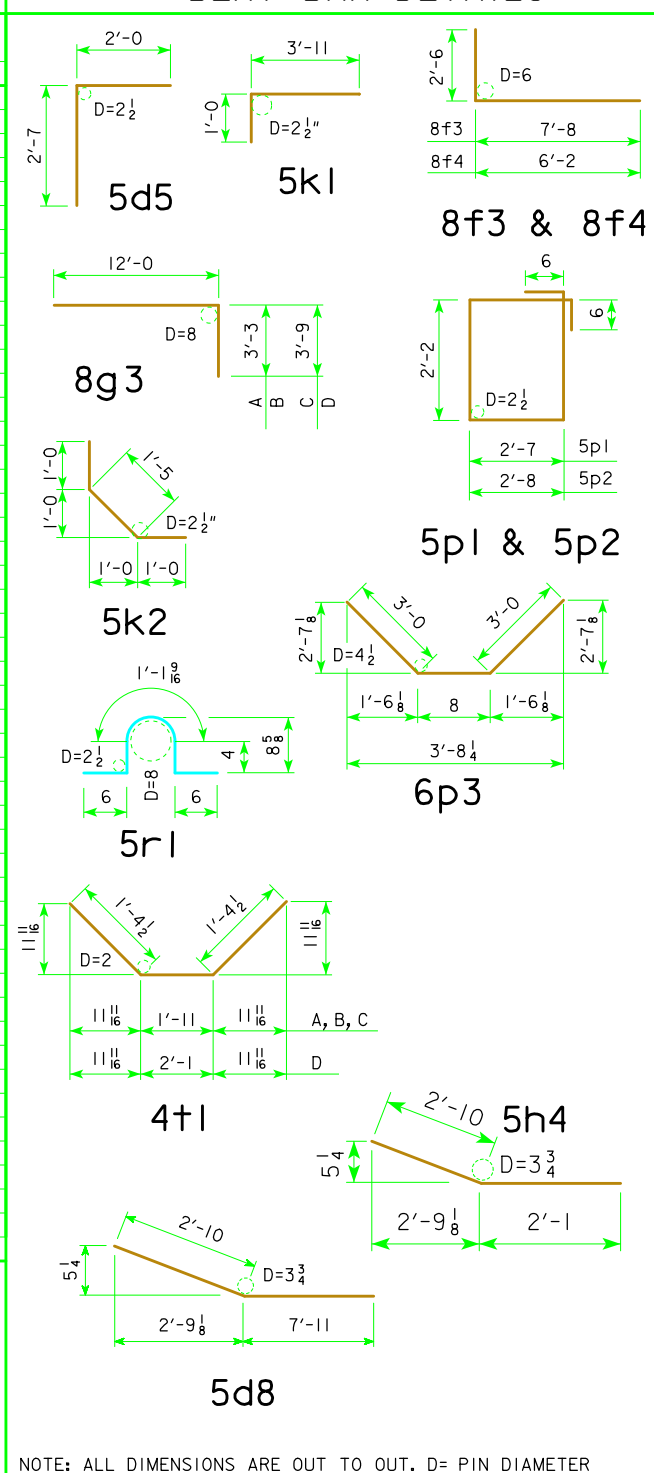
REINFORCING BAR LIST

ONE SUPERSTRUCTURE AND TWO ABUTMENTS

BRIDGE LENGTH

| BAR | LOCATION | SHAPE | BRIDGE LENGTH | | | | | | | | | | | | | | | | | | | | |
|-----|--|-------|---------------|--------|--------|-------|--------|--------|-------|--------|--------|-------|--------|--------|-------|--------|--------|--------|--------|--------|--------|--------|--------|
| | | | 46'-8 | | | 55'-0 | | | 67'-6 | | | 80'-0 | | | 90'-0 | | | 100'-0 | | | 110'-0 | | |
| | | | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT |
| 6a1 | SLAB TRANSVERSE, TOP & BOTTOM | | 131 | 32'-10 | 6461 | 153 | 32'-10 | 7546 | 187 | 32'-10 | 9223 | 221 | 32'-10 | 10,899 | 247 | 32'-10 | 12,181 | 273 | 32'-10 | 13,464 | 301 | 32'-10 | 14,845 |
| 5b1 | SLAB LONGITUDINAL TOP & BOTTOM | | 168 | 25'-9 | 4513 | 168 | 29'-11 | 5242 | 168 | 36'-2 | 6337 | 252 | 29'-0 | 7622 | 252 | 32'-4 | 8498 | 252 | 35'-8 | 9374 | 252 | 39'-0 | 10,251 |
| 5d2 | ABUTMENT DIAPHRAGM, LONGITUDINAL - F.F | | 24 | 6'-2 | 155 | 24 | 6'-2 | 155 | 24 | 5'-11 | 149 | 24 | 5'-11 | 149 | 24 | 6'-1 | 153 | 24 | 6'-1 | 153 | 24 | 6'-1 | 153 |
| 5d3 | ABUTMENT DIAPHRAGM, LONGITUDINAL - F.F | | 8 | 5'-3 | 44 | 8 | 5'-3 | 44 | 8 | 5'-0 | 42 | 8 | 5'-0 | 42 | 8 | 4'-10 | 41 | 8 | 4'-10 | 41 | 8 | 4'-10 | 41 |
| 5d5 | ABUTMENT DIAPHRAGM, LONGITUDINAL - END | | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 |
| 5d6 | ABUTMENT DIAPHRAGM LONGITUDINAL - B.F. | | 8 | 32'-9 | 274 | 8 | 32'-9 | 274 | 8 | 32'-9 | 274 | 8 | 32'-9 | 274 | 8 | 32'-9 | 274 | 8 | 32'-9 | 274 | 8 | 32'-9 | 274 |
| 5d7 | PAVING NOTCH, LONGITUDINAL | | 4 | 32'-8 | 137 | 4 | 32'-8 | 137 | 4 | 32'-8 | 137 | 4 | 32'-8 | 137 | 4 | 32'-8 | 137 | 4 | 32'-8 | 137 | 4 | 32'-8 | 137 |
| 5d8 | ABUTMENT DIAPH. WING EXT. LONGIT. | | - | - | - | - | - | - | - | - | - | 24 | 10'-9 | 269 | 24 | 10'-9 | 269 | 24 | 10'-9 | 269 | 24 | 10'-9 | 269 |
| 5d9 | ABUTMENT DIAPH. WING EXT. LONGIT. | | - | - | - | - | - | - | - | - | - | 24 | 10'-8 | 267 | 24 | 10'-8 | 267 | 24 | 10'-8 | 267 | 24 | 10'-8 | 267 |
| 8f1 | ABUTMENT FOOTING LONGITUDINAL | | 18 | 32'-11 | 1582 | 18 | 32'-11 | 1582 | 18 | 32'-11 | 1582 | 18 | 32'-11 | 1582 | 18 | 32'-11 | 1582 | 18 | 32'-11 | 1582 | 18 | 32'-11 | 1582 |
| 8f3 | ABUTMENT EXTENSION LONGITUDINAL | | - | - | - | - | - | - | - | - | - | 16 | 10'-2 | 435 | 16 | 10'-2 | 435 | 16 | 10'-2 | 435 | 16 | 10'-2 | 435 |
| 8f4 | ABUTMENT EXTENSION LONGITUDINAL | | - | - | - | - | - | - | - | - | - | 16 | 8'-8 | 371 | 16 | 8'-8 | 371 | 16 | 8'-8 | 371 | 16 | 8'-8 | 371 |
| 8g1 | ABUTMENT VERTICAL | | 114 | 6'-7 | 2004 | 114 | 6'-7 | 2004 | 114 | 7'-2 | 2181 | 110 | 7'-8 | 2252 | 110 | 8'-5 | 2472 | 110 | 8'-5 | 2472 | 110 | 8'-5 | 2472 |
| 8g3 | ABUTMENT DIAPHRAGM VERTICAL - B.F. | | 50 | 15'-3 | 2036 | 50 | 15'-3 | 2036 | 50 | 15'-3 | 2036 | 50 | 15'-9 | 2103 | 50 | 15'-9 | 2103 | 50 | 15'-9 | 2103 | 50 | 15'-9 | 2103 |
| 6g4 | ABUT. DIAPH. WING EXT. VERT. | | - | - | - | - | - | - | - | - | - | 40 | 5'-8 | 341 | 40 | 6'-5 | 386 | 40 | 6'-5 | 386 | 40 | 6'-5 | 386 |
| 5h1 | ABUTMENT TO WING ANCHOR | | 28 | 6'-8 | 196 | 28 | 6'-8 | 196 | 28 | 6'-8 | 196 | 36 | 6'-8 | 252 | 36 | 6'-8 | 252 | 36 | 6'-8 | 252 | 36 | 6'-8 | 252 |
| 5h2 | ABUTMENT TO WING ANCHOR | | 56 | 4'-11 | 288 | 56 | 4'-11 | 288 | 56 | 4'-11 | 288 | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 |
| 5h3 | ABUTMENT TO WING ANCHOR | | 28 | 6'-9 | 198 | 28 | 6'-9 | 198 | 28 | 6'-9 | 198 | 36 | 6'-9 | 256 | 36 | 6'-9 | 256 | 36 | 6'-9 | 256 | 36 | 6'-9 | 256 |
| 5h4 | ABUTMENT TO WING ANCHOR | | - | - | - | - | - | - | - | - | - | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 |
| 5j1 | SLAB TRANSV. TOP AT RAIL | | 130 | 6'-3 | 848 | 152 | 6'-3 | 991 | 186 | 6'-3 | 1213 | 220 | 6'-3 | 1435 | 246 | 6'-3 | 1604 | 272 | 6'-3 | 1774 | 300 | 6'-3 | 1956 |
| 5k1 | PAVING NOTCH, TRANSVERSE | | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 |
| 5k2 | PAVING NOTCH, TRANSVERSE | | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 |
| 5p1 | ABUTMENT HOOPS | | 120 | 10'-6 | 1314 | 120 | 10'-6 | 1314 | 108 | 10'-6 | 1183 | 104 | 10'-6 | 1139 | 120 | 10'-6 | 1315 | 104 | 10'-6 | 1139 | 104 | 10'-6 | 1139 |
| 5p2 | ABUTMENT EXTENSIONS HOOPS | | - | - | - | - | - | - | - | - | - | 24 | 10'-8 | 268 | 24 | 10'-8 | 268 | 24 | 10'-8 | 268 | 24 | 10'-8 | 268 |
| 6p3 | ABUT. BOTT. AT PILES | | - | - | - | - | - | - | - | - | - | 28 | 6'-8 | 281 | 32 | 6'-8 | 321 | 36 | 6'-8 | 361 | 36 | 6'-8 | 361 |
| 5r1 | PAVING BLOCK LIFTING HOOPS | | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 |
| 5s1 | WING, VERTICAL | | 64 | 5'-10 | 390 | 64 | 5'-10 | 390 | 64 | 6'-2 | 412 | 64 | 6'-11 | 462 | 64 | 7'-8 | 512 | 64 | 7'-8 | 512 | 64 | 7'-8 | 512 |
| 4t1 | UNDER BEAMS AT ABUTMENTS | | 10 | 4'-8 | 32 | 10 | 4'-8 | 32 | 10 | 4'-8 | 32 | 10 | 4'-8 | 32 | 10 | 4'-10 | 33 | 10 | 4'-10 | 33 | 10 | 4'-10 | 33 |
| #2 | PILE SPIRAL - NO. 2 BAR | | 10 | 38'-6 | 64 | 10 | 38'-6 | 64 | 12 | 38'-6 | 77 | 18 | 38'-6 | 115 | 20 | 38'-6 | 128 | 22 | 38'-6 | 141 | 22 | 38'-6 | 141 |
| | SPIRAL SPACER L $\frac{7}{8}$ x $\frac{1}{2}$ x $\frac{1}{8}$ x 0.70 | | 20 | 1'-10 | 27 | 20 | 1'-10 | 27 | 24 | 1'-10 | 32 | 36 | 1'-10 | 48 | 40 | 1'-10 | 53 | 44 | 1'-10 | 58 | 44 | 1'-10 | 58 |
| | REINFORCING STEEL - (LBS.) | | | | 21115 | | | 23072 | | | 26144 | | | 31707 | | | 34587 | | | 36798 | | | 39238 |
| | SEE BARRIER RAIL DETAILS (LBS.) | | | | 4054 | | | 4410 | | | 4968 | | | 6002 | | | 6438 | | | 6875 | | | 7353 |
| | SEE OPEN RAIL DETAILS (LBS.) | | | | 4179 | | | 4572 | | | 5277 | | | 6816 | | | 7478 | | | 7900 | | | 8322 |

BENT BAR DETAILS



| CONCRETE PLACEMENT QUANTITIES (SUPERSTRUCTURE PLUS INTEGRAL ABUTMENTS) | | ESTIMATED QUANTITIES (SUPERSTRUCTURE PLUS INTEGRAL ABUTMENTS) | | | | | | | | | | | | | | | | | |
|--|-------------------|---|-------|-------|-------|-------|-------|--------|--------|--|-------------------|--------|--------|--------|--------|--------|--------|--------|--------|
| SLAB, AND | WITH BARRIER RAIL | CU.YD. | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 | NO. OF STEEL H-PILES FOR TWO ABUTMENTS (HP10X57) | NO. | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 | |
| ABUT. DIAPHRAGM | WITH OPEN RAIL | CU.YD. | 69.1 | 76.3 | 91.0 | 111.1 | 126.9 | 136.1 | 145.2 | NO. | 10 | 10 | 12 | 18 | 20 | 22 | 22 | 22 | |
| PAVING BLOCKS | | CU.YD. | 2.3 | 2.3 | 2.3 | 2.3 | 2.3 | 2.3 | 2.3 | STRUCTURAL CONCRETE, (BRIDGE) | CU.YD. | 104.4 | 111.6 | 126.7 | 155.4 | 172 | 181.2 | 190.3 | |
| ABUTMENT WINGS | | CU.YD. | 7.2 | 7.2 | 7.6 | 8.4 | 9.2 | 9.2 | 9.2 | CONCRETE RAIL (BARRIER OR OPEN) | L.F. | 127.3 | 144 | 169 | 214 | 234 | 254 | 274 | |
| ABUTMENT FOOTINGS | | CU.YD. | 26.6 | 26.6 | 26.6 | 34.4 | 34.4 | 34.4 | 34.4 | REINFORCING STEEL | WITH BARRIER RAIL | LBS. | 24,969 | 27,282 | 30,912 | 37,500 | 40,816 | 43,464 | 46,382 |
| | | | | | | | | | | WITH OPEN RAIL | LBS. | 25,294 | 27,644 | 31,421 | 38,523 | 42,065 | 44,698 | 47,560 | |
| | | | | | | | | | | STRUCTURAL STEEL | WITH BARRIER RAIL | LBS. | 1480 | 1471 | 1515 | 1555 | 1642 | 1642 | 1642 |
| | | | | | | | | | | WITH OPEN RAIL | LBS. | 1190 | 1180 | 1180 | 1180 | 1212 | 1212 | 1212 | |

05-15
LATEST REVISION DATE

Thomas L. Mc Donald
APPROVED BY BRIDGE ENGINEER

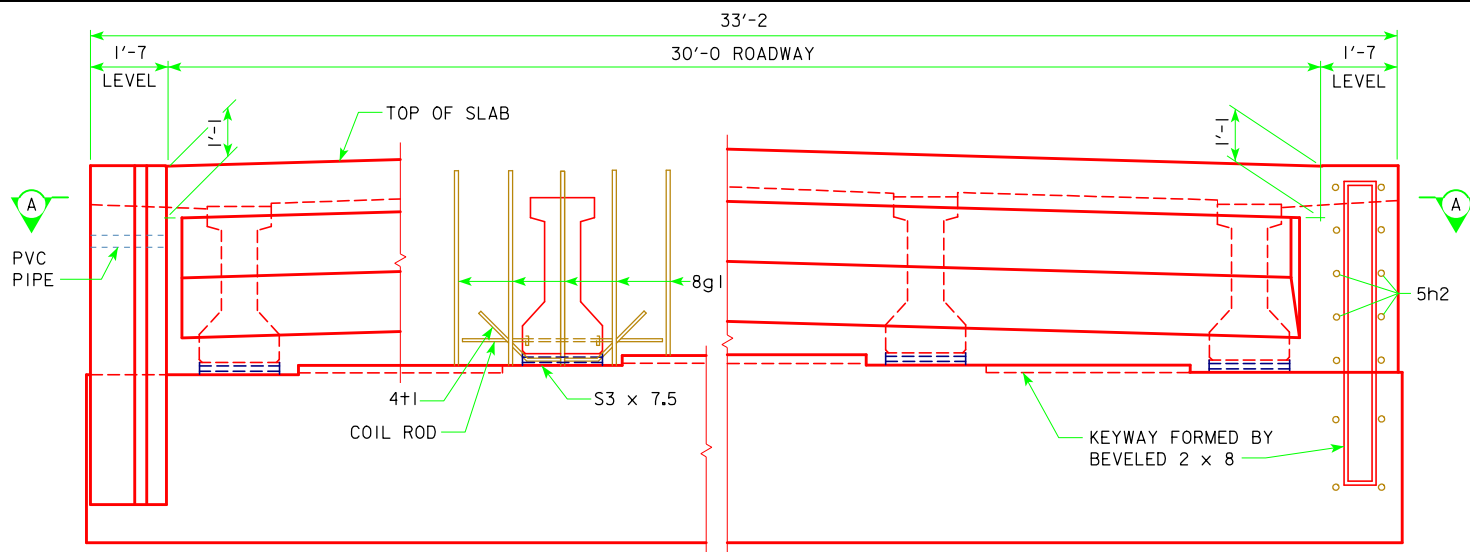
IOWADOT Highway Division

STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE

**PRETENSIONED PRESTRESSED
CONCRETE BEAM BRIDGES**

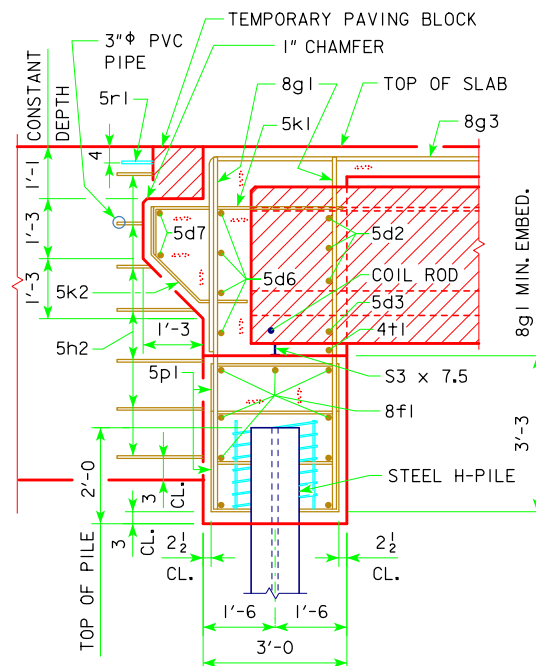
APRIL, 2012

DECK & ABUTMENT REINF. H30SI-08-12
0° SKEW



PART REAR ELEVATION AT ABUTMENT

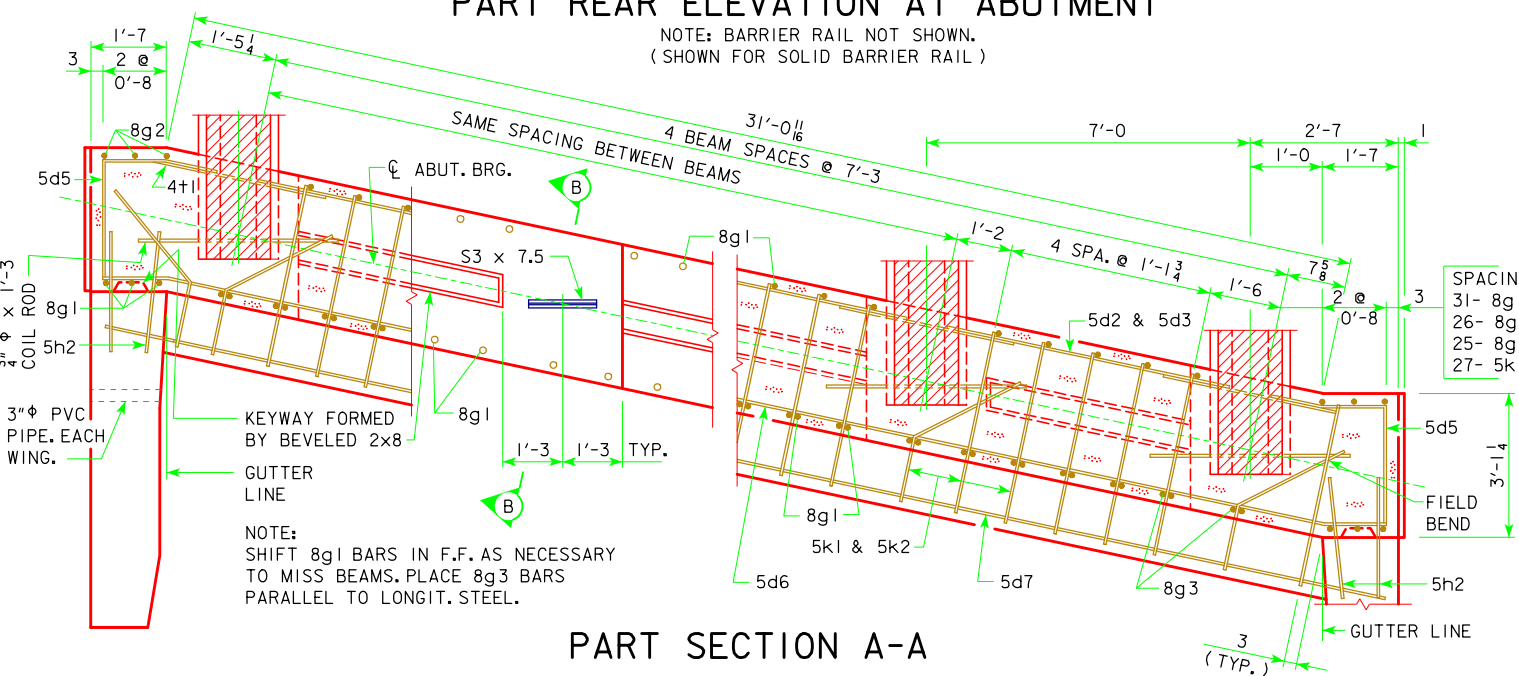
NOTE: BARRIER RAIL NOT SHOWN.
(SHOWN FOR SOLID BARRIER RAIL)



PART SECTION B-B

NOTE:
THE SPIRAL AT THE TOP OF EACH PILE TO BE 7 TURNS OF NO. 2 BAR, 21" DIAMETER, 3" PITCH WITH 2 - L₇/₈ x ₇/₈ x ₁/₈ SPACERS PUNCHED TO HOLD SPIRAL.

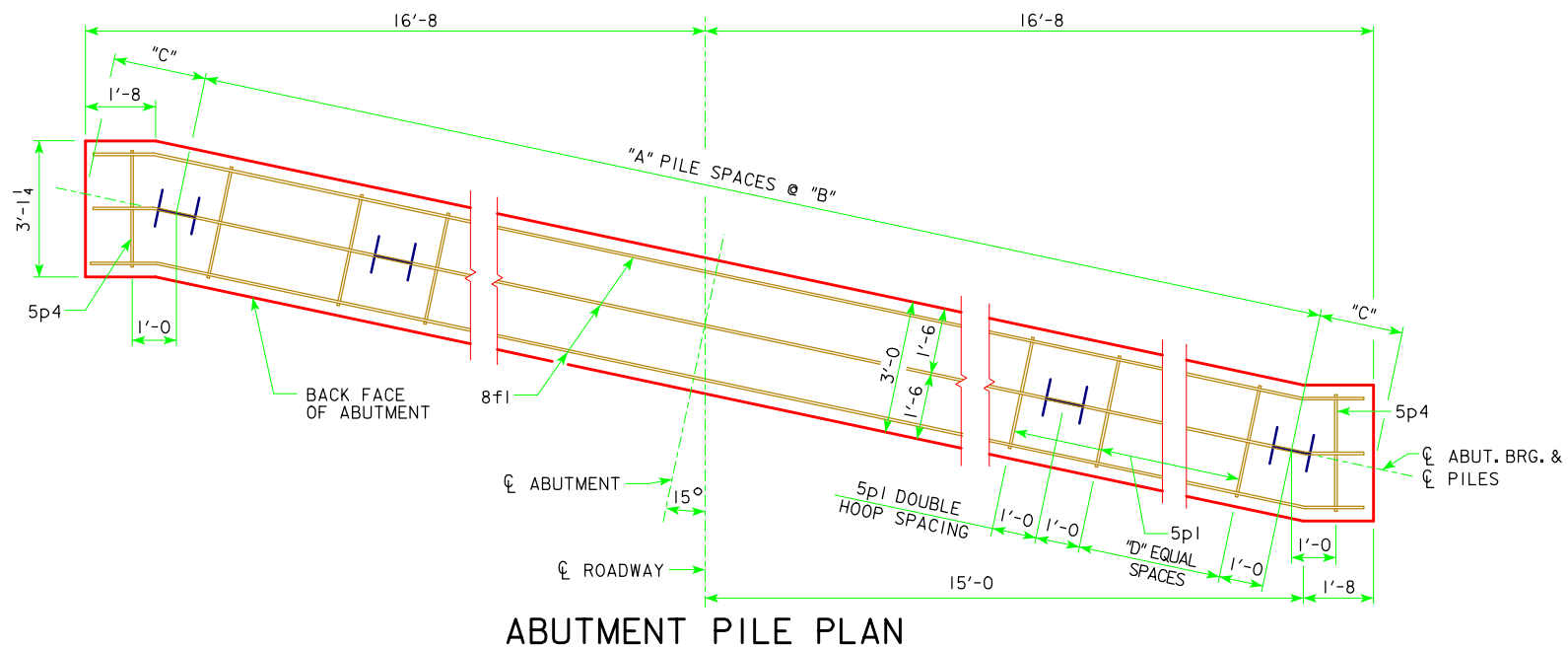
NOTE:
PLACE 5h2 BAR AT 1:6 SLOPE TO MATCH TRAFFIC SIDE OF ABUTMENT WING FACE (BOTH SIDES TYPICAL).



PART SECTION A-A

NOTE:
SHIFT 8g1 BARS IN F.F. AS NECESSARY TO MISS BEAMS. PLACE 8g3 BARS PARALLEL TO LONGIT. STEEL.

SPACING FOR:
31- 8g1 BACK FACE
26- 8g1 FRONT FACE
25- 8g3 BACK FACE
27- 5k1 & 5k2 BACK FACE



ABUTMENT PILE PLAN

ABUTMENT NOTES:

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.

ABUTMENT PILES SHALL BE DRIVEN TO VALUES SHOWN IN DESIGN PLANS.

PLACE 5h2 BAR AT 1:6 SLOPE TO MATCH TRAFFIC SIDE OF ABUTMENT WING FACE. (BOTH SIDES TYPICAL)

BARRIER RAIL NOT SHOWN IN DETAILS.

IF ROCK IS CLOSER THAN 15' BELOW ABUTMENT FOOTING, SPECIAL ANALYSIS MAY BE REQUIRED.

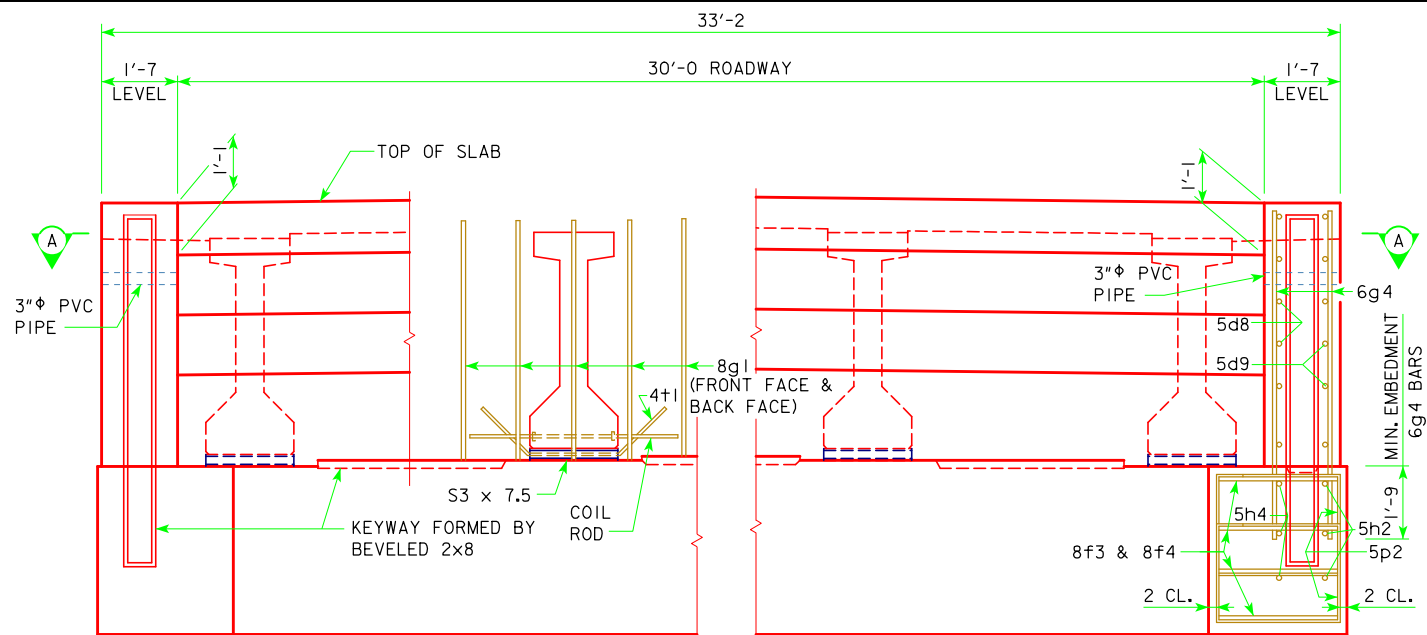
ABUTMENT PILE SPACING

| DIMENSION OR NO. | CL TO CL ABUTMENT BEARING | | |
|-------------------------------|-----------------------------------|-----------------------------------|-----------------------------------|
| | 46'-8 | 55'-0 | 67'-6 |
| "A" PILE SPACES | 4 | 4 | 5 |
| "B" (FT. - IN.) | 7'-6 | 7'-6 | 6'-0 |
| "C" (FT. - IN.) | 2'-3 ¹ / ₁₆ | 2'-3 ¹ / ₁₆ | 2'-3 ¹ / ₁₆ |
| "D" EQUAL SPACES | 6 | 6 | 4 |
| NO. OF PILES PER ABUT. | 5 | 5 | 6 |
| STRENGTH I DESIGN LOAD (KIPS) | 130 | 140 | 134 |

NOTE: P_u, STRENGTH I DESIGN LOAD (KIPS) IS NOT THE VALUE USED IN THE FIELD FOR DRIVING PILES.

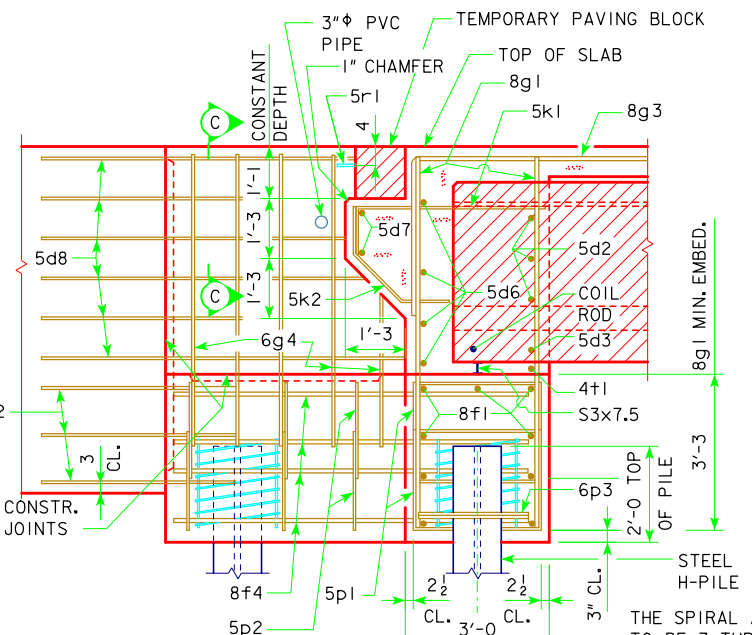
REVISED 05-13 - REVISION FOR LRFD PILE DESIGN.

| | | |
|--|---|--------------------|
| 05-13 LATEST REVISION DATE <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | ABUTMENT DETAILS 15° SKEW A & B BEAMS | H30S1-09-12 |

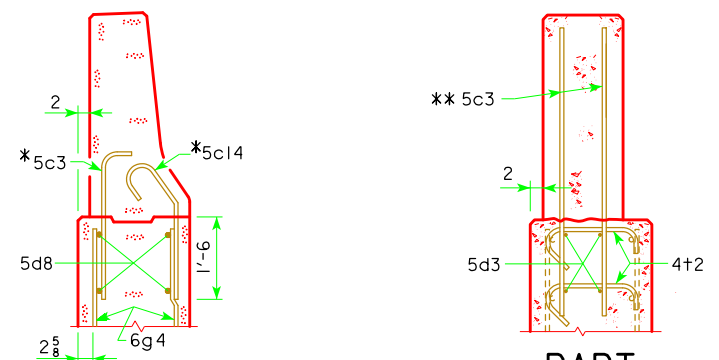


PART REAR ELEVATION AT ABUTMENT

NOTE: BARRIER RAIL AND WINGS NOT SHOWN.
(SHOWN FOR SOLID BARRIER RAIL)



PART SECTION B - B

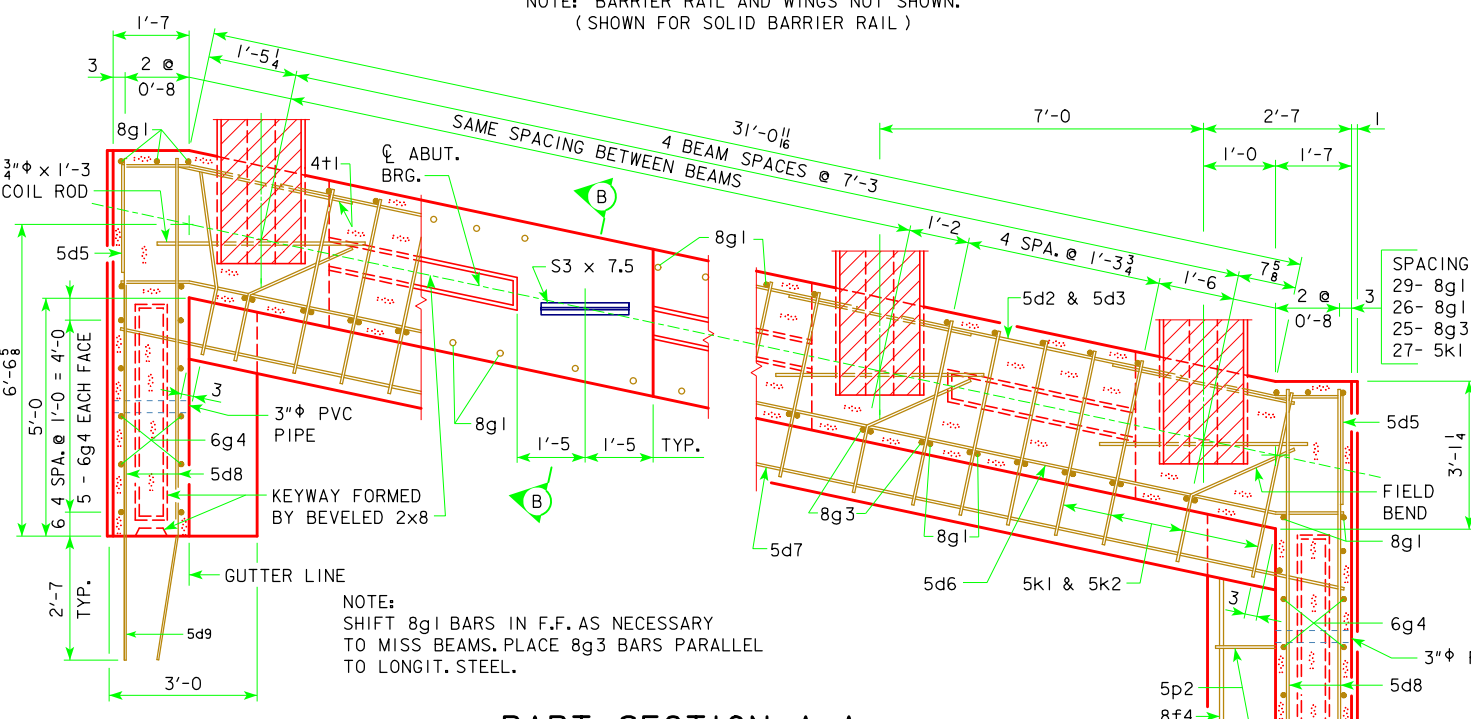


PART SECTION C-C

* NOTE: SEE BARRIER RAIL SHEET FOR DETAILS. REINFORCING BARS 5c3 AND 5c14 ARE INCLUDED IN SUPERSTRUCTURE QUANTITIES.

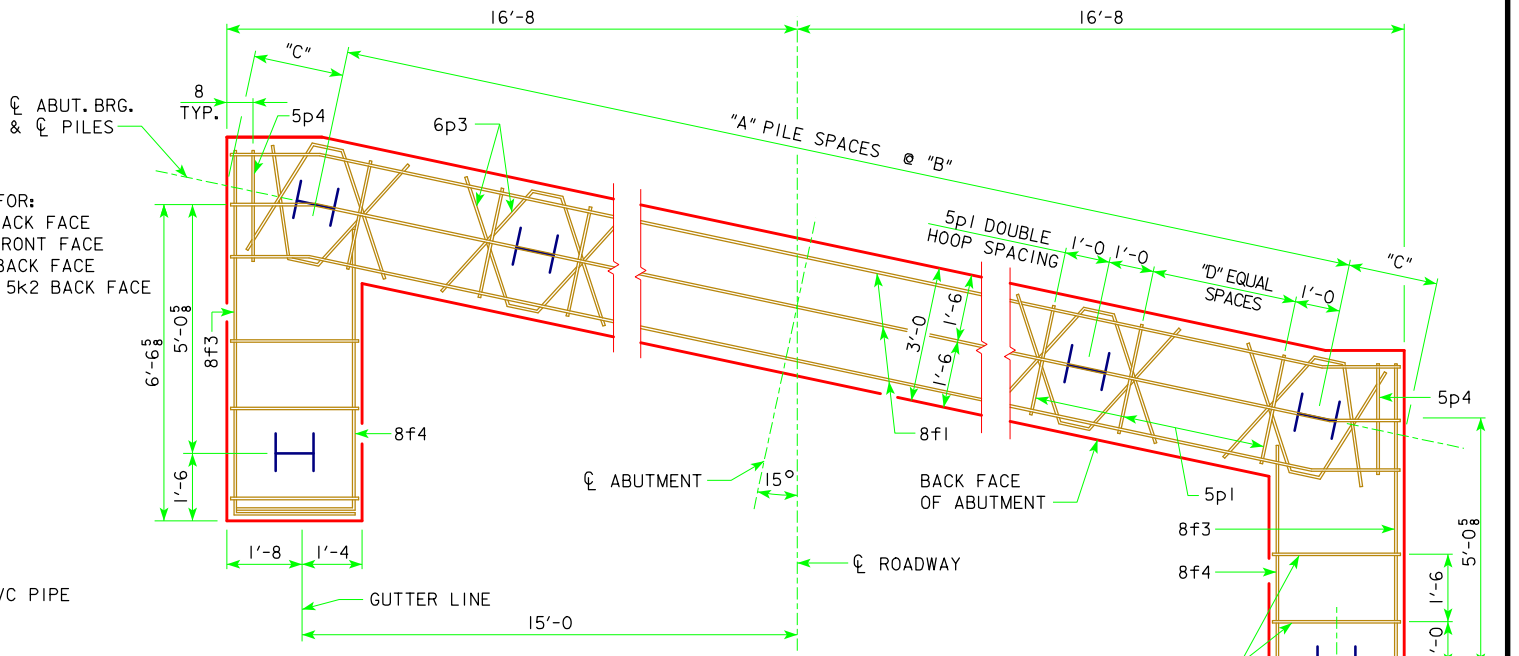
PART SECTION C-C

** NOTE: SEE OPEN RAIL SHEET FOR DETAILS. REINFORCING BARS 5c3 ARE INCLUDED IN SUPERSTRUCTURE QUANTITIES.



PART SECTION A-A

NOTE: SHIFT 8g1 BARS IN F.F. AS NECESSARY TO MISS BEAMS. PLACE 8g3 BARS PARALLEL TO LONGIT. STEEL.



ABUTMENT PILE PLAN

ABUTMENT PILE SPACING

| DIMENSION OR NO. | CL TO CL ABUTMENT BEARING | | | |
|-------------------------------|---------------------------|-----------|------------|------------|
| | 80'-0 | 90'-0 | 100'-0 | 110'-0 |
| "A" PILE SPACES | 6 | 7 | 8 | 8 |
| "B" (FT - IN) | 5'-2 | 4'-5 | 3'-10 | 3'-10 |
| "C" (FT - IN) | 1'-9 1/16 | 1'-9 3/16 | 1'-11 1/16 | 1'-11 1/16 |
| "D" EQUAL SPACES | 4 | 3 | 2 | 2 |
| NO. OF PILES PER ABUT. | 9 | 10 | 11 | 11 |
| STRENGTH I DESIGN LOAD (KIPS) | 138 | 134 | 126 | 134 |

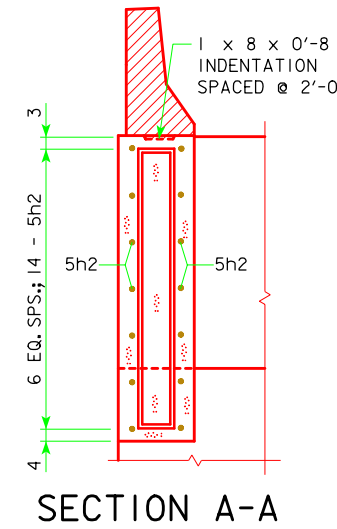
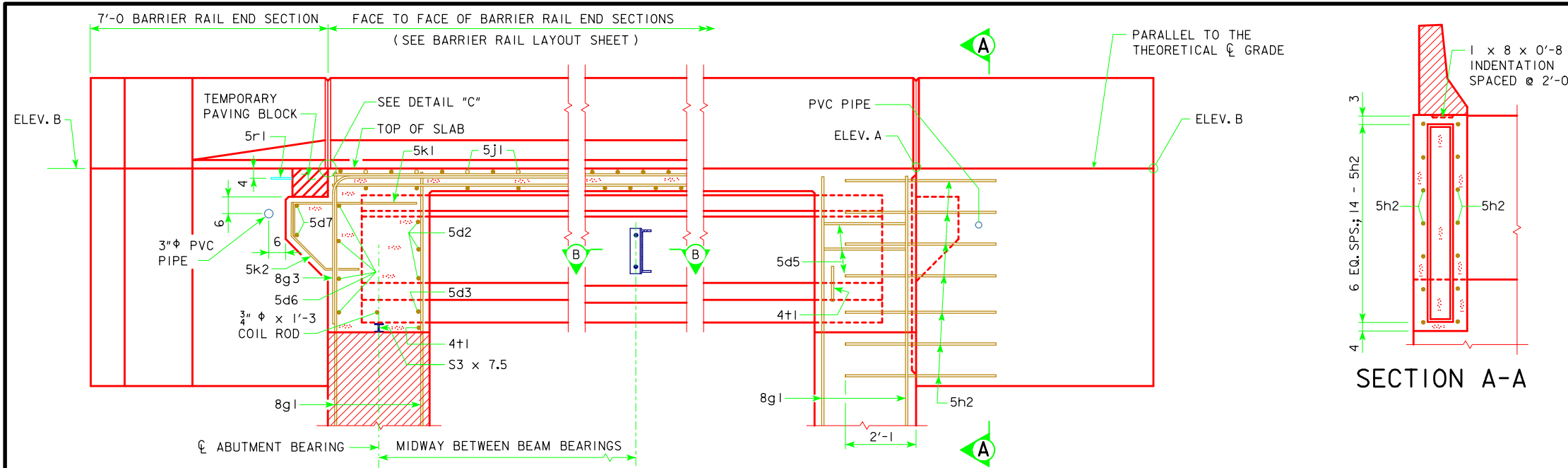
NOTE: P_u, STRENGTH I DESIGN LOAD (KIPS) IS NOT THE VALUE USED IN THE FIELD FOR DRIVING PILES.

ABUTMENT NOTES:

- MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.
- ABUTMENT PILES SHALL BE DRIVEN TO VALUES SHOWN IN DESIGN PLANS.
- BARRIER RAIL NOT SHOWN IN DETAILS.
- IF ROCK IS CLOSER THAN 15' BELOW ABUTMENT FOOTING, SPECIAL ANALYSIS MAY BE REQUIRED.

| | |
|--|---|
| 05-13 LATEST REVISION DATE <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 |
| | <div style="text-align: center;"> ABUTMENT DETAILS 15° SKEW C & D BEAMS </div> <div style="text-align: center;"> H30SI-10-12 </div> |

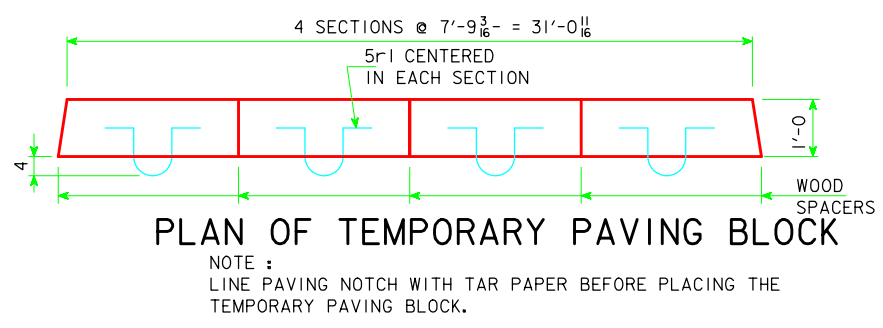
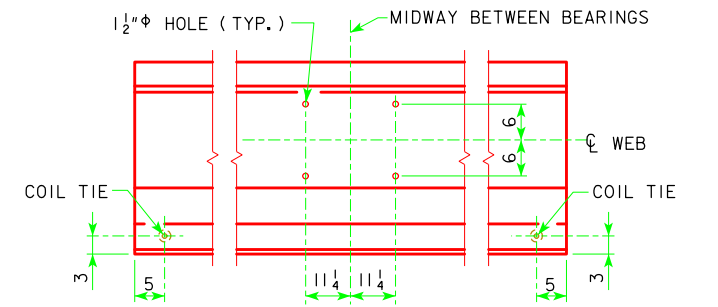
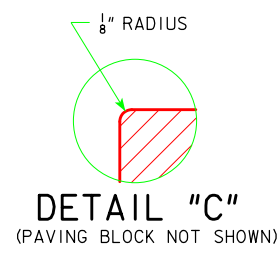
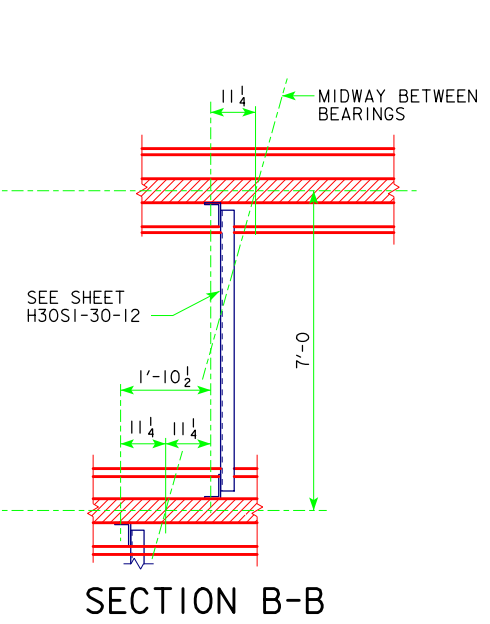
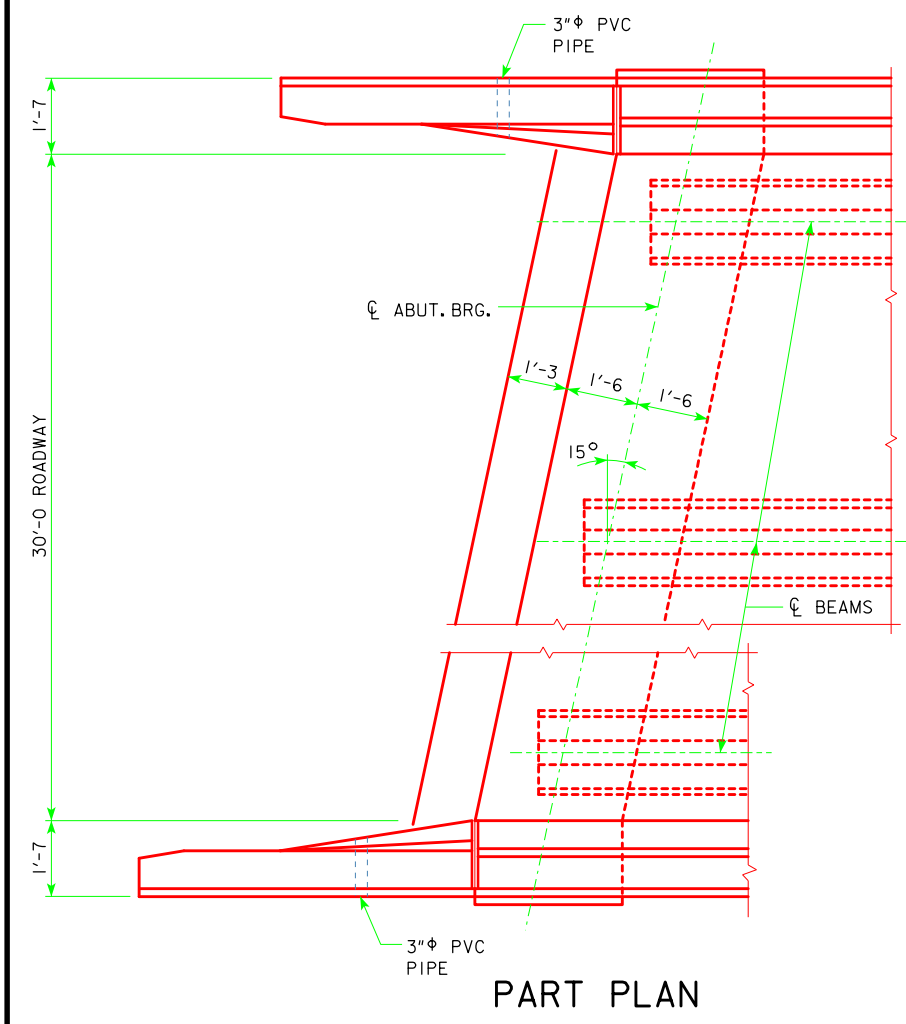
REVISED 05-13 - REVISION FOR LRFD PILE DESIGN.



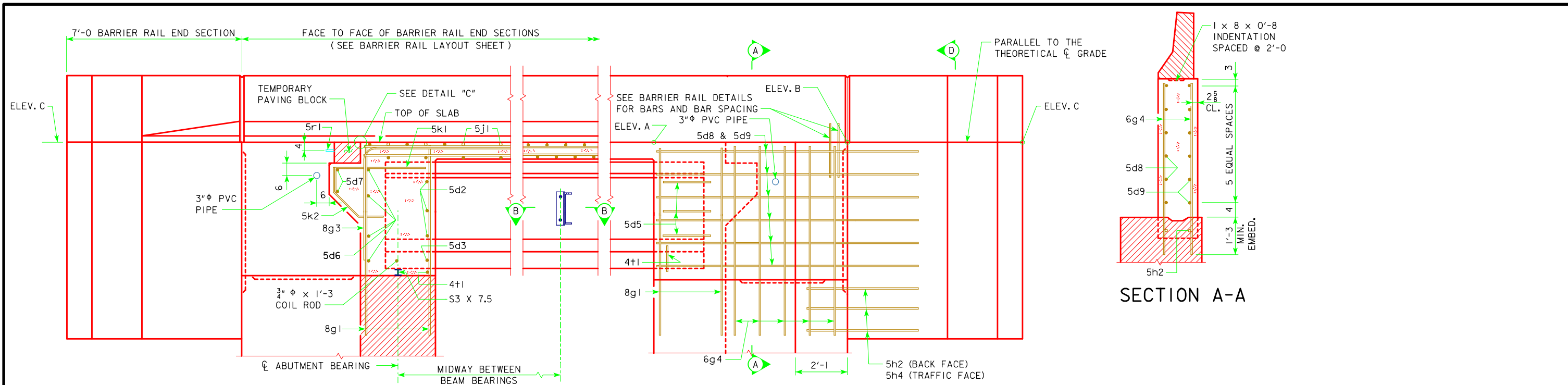
PART LONGITUDINAL SECTION NEAR GUTTER
(FOR DETAILS OF INTERMEDIATE DIAPHRAGM SEE SHEET H30SI-30-12)

PART END VIEW AT ABUTMENT
PROVIDE ELEVATIONS A AND B IN THE BRIDGE PLAN SHEETS.

NOTE: PLUG 3" ϕ PVC PIPE WITH EXPANDING FOAM PRIOR TO BACKFILLING BEHIND ABUTMENTS.



| | |
|---|---|
| LATEST REVISION DATE APPROVED BY BRIDGE ENGINEER <i>Thomas L. Mc Donald</i> | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 |
| | LONGITUDINAL SECTION 15° SKEW A & B BEAMS |
| H30SI-11-12 | |



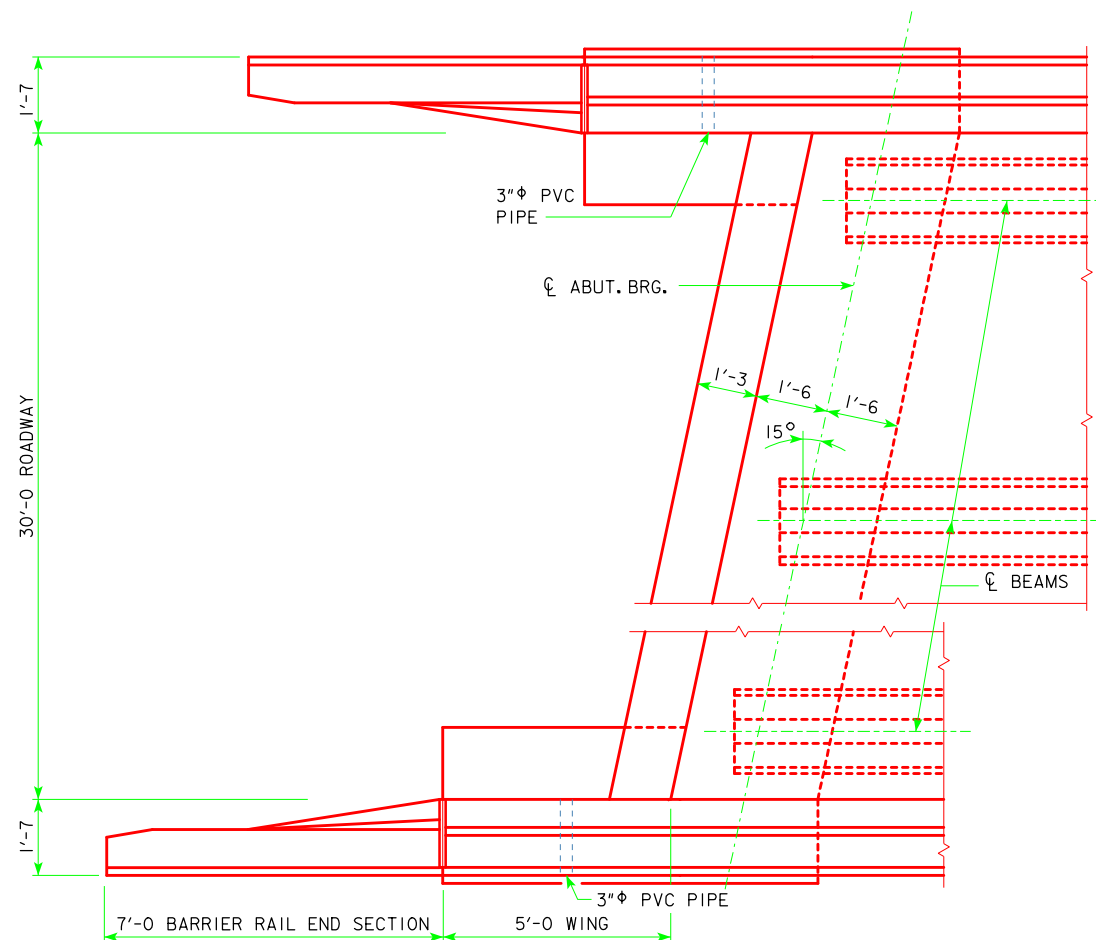
PART LONGITUDINAL SECTION NEAR GUTTER

(FOR DETAILS OF INTERMEDIATE DIAPHRAGM SEE SHEET H30SI-30-12)

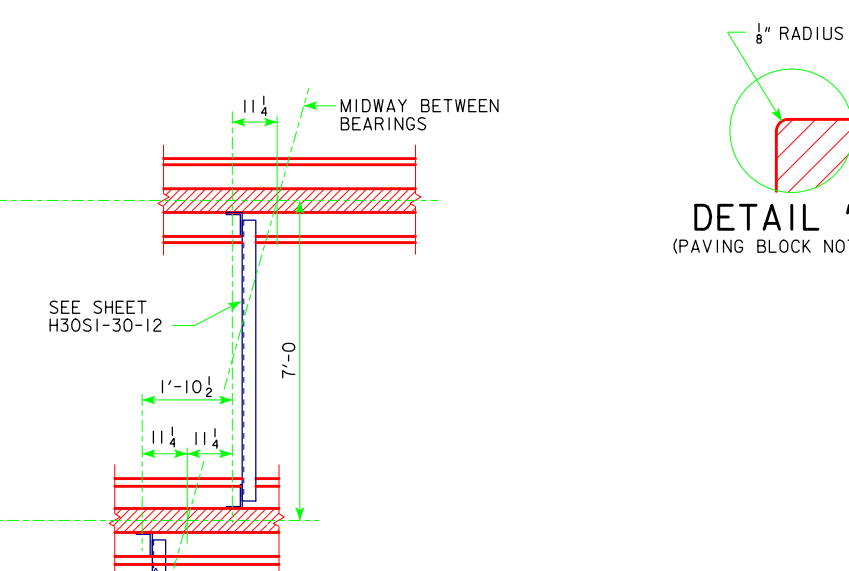
PART END VIEW AT ABUTMENT

PROVIDE ELEVATIONS A, B, AND C IN THE BRIDGE PLAN SHEETS.

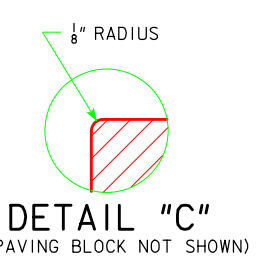
NOTE: PLUG 3" ϕ PVC PIPE WITH EXPANDING FOAM PRIOR TO BACKFILLING BEHIND ABUTMENTS.



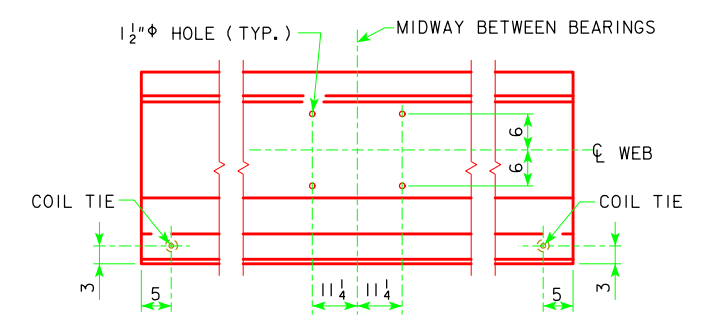
PART PLAN



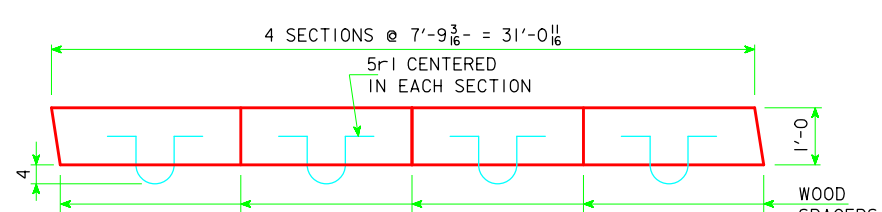
SECTION B-B



DETAIL "C"
(PAVING BLOCK NOT SHOWN)



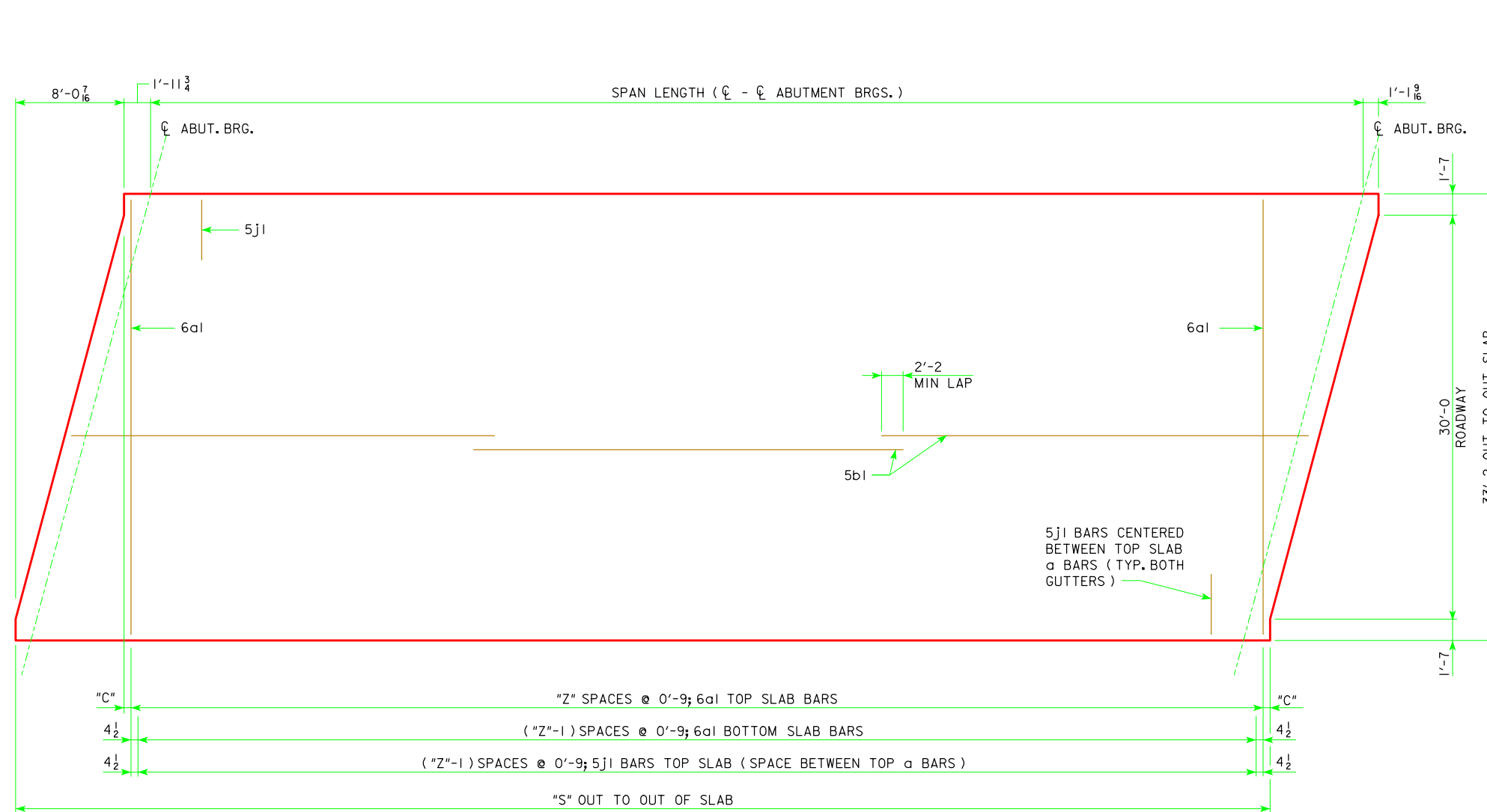
LOCATION OF BEAM COIL TIES AND STEEL DIAPHRAGM BOLT HOLES



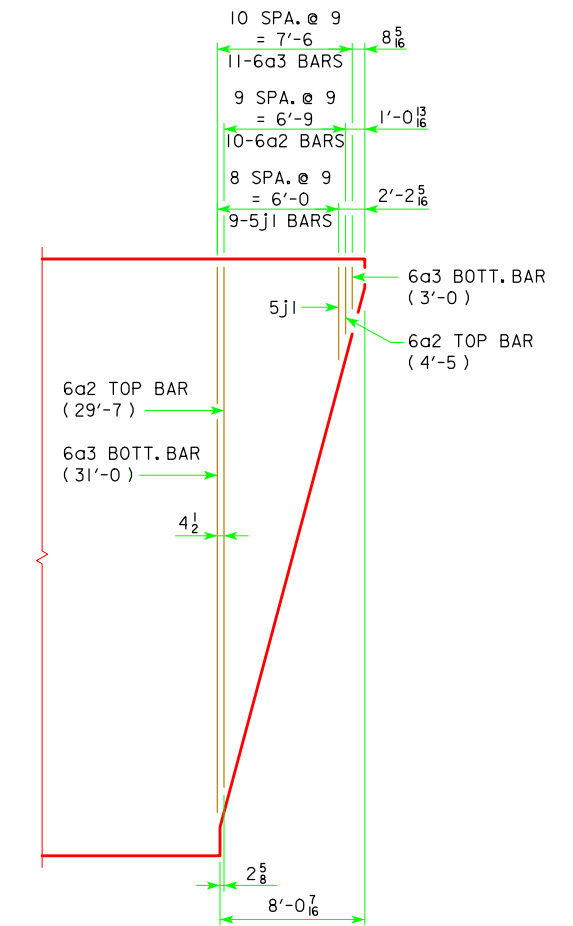
PLAN OF TEMPORARY PAVING BLOCK

NOTE: LINE PAVING NOTCH WITH TAR PAPER BEFORE PLACING THE TEMPORARY PAVING BLOCK.

| | | | |
|----------------------|---------------------------------|---|--------------------|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | LONGITUDINAL SECTION 15° SKEW C & D BEAMS | H30SI-12-12 |



SLAB LAYOUT
(LEFT AHEAD SKEW SHOWN, RIGHT AHEAD SKEW SIMILAR)



END OF SLAB REINFORCING
(TYPICAL EACH END OF DECK)

GENERAL DATA

| | | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 |
|--|---|-----------|-----------|-----------|-----------|------------|------------|------------|
| SPAN LENGTH (℄ - ℄ ABUTMENT BRGS.) | | | | | | | | |
| LOCATION OF EXTREME 6a1 TOP BAR FROM END OF SLAB | "C" | 2 7/8 | 3 3/8 | 6 3/8 | 4 7/8 | 6 3/8 | 3 3/8 | 4 7/8 |
| NO. OF SPACES FOR 6a1 TOP BARS | "Z" | 55 | 66 | 82 | 99 | 112 | 126 | 139 |
| OUT TO OUT OF SLAB | "S" | 49'-9 1/4 | 58'-1 1/4 | 70'-7 1/4 | 83'-1 1/4 | 93'-1 1/4 | 103'-1 1/4 | 113'-1 1/4 |
| VERTICAL CURVE | TOP OF SLAB TO ABUTMENT TOP AT ℄ ABUTMENT BEARING | "U" | 3'-8 1/8 | 3'-8 1/2 | 4'-3 7/8 | 4'-9 7/16 | 5'-6 11/16 | 5'-7 5/8 |
| STRAIGHT GRADE | TOP OF SLAB TO ABUTMENT TOP AT ℄ ABUTMENT BEARING | "U" | 3'-8 5/16 | 3'-8 3/4 | 4'-4 3/16 | 4'-9 15/16 | 5'-6 13/16 | 5'-7 7/16 |
| SERVICE D.L. ABUTMENT REACTION (D.L. + F.W.S.) SERVICE LOADS | KIPS | 301.8 | 328.9 | 392.9 | 506.9 | 579.8 | 620.9 | 662.0 |
| SERVICE L.L. ABUTMENT REACTION (HL-93) NO IMPACT SERVICE LOADS | KIPS | 152.8 | 162.4 | 175.0 | 186.0 | 194.4 | 202.2 | 209.8 |

REVISED 05-13 - REVISION FOR LRED PILE DESIGN.

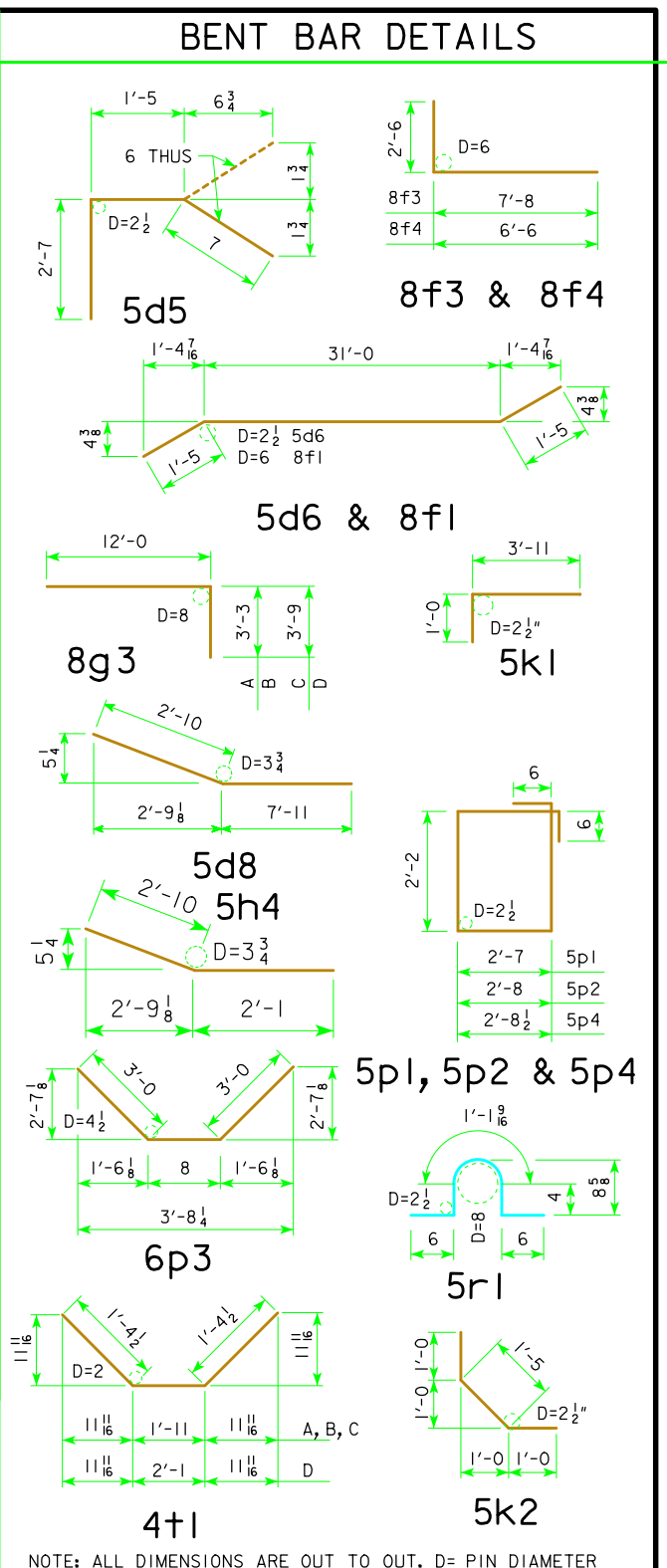
| | | | |
|-------------------------------|---------------------------------|---|--------------------|
| 05-13 LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | SUPERSTRUCTURE 15° SKEW | H30S1-13-12 |
| | | | |

REVISED 09-14 - CORRECTED THE BARRIER RAIL REINFORCING STEEL QUANTITIES FOR ALL THE BRIDGE LENGTHS.
 REVISED 05-15 - CORRECTED THE CONCRETE QUANTITIES OF THE PAVING BLOCKS FOR ALL THE BRIDGE LENGTHS.
 REVISED 11-15 - REMOVED THE NOTE "NON-EPOXY COATED" FROM THE "REINFORCING STEEL (LBS.)" LINE IN THE REINFORCING BAR CHART LIST.

| REINFORCING BAR LIST | | | BRIDGE LENGTH | | | | | | | | | | | | | | | | | | | | |
|--------------------------------------|---|-------|---------------|--------|--------|-------|--------|--------|-------|--------|--------|-------|--------|--------|-------|--------|--------|--------|--------|--------|--------|--------|--------|
| | | | 46'-8 | | | 55'-0 | | | 67'-6 | | | 80'-0 | | | 90'-0 | | | 100'-0 | | | 110'-0 | | |
| ONE SUPERSTRUCTURE AND TWO ABUTMENTS | | | A | | A | | B | | C | | D | | D | | D | | D | | | | | | |
| BAR | LOCATION | SHAPE | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | | | |
| 6a1 | SLAB TRANSVERSE, TOP & BOTTOM | | 111 | 32'-10 | 5475 | 133 | 32'-10 | 6559 | 165 | 32'-10 | 8138 | 199 | 32'-10 | 9814 | 225 | 32'-10 | 11,097 | 253 | 32'-10 | 12,477 | 279 | 32'-10 | 13,760 |
| 6a2 | SLAB TRANSVERSE END, TOP | | 20 | * | 511 | 20 | * | 511 | 20 | * | 511 | 20 | * | 511 | 20 | * | 511 | 20 | * | 511 | 20 | * | 511 |
| 6a3 | SLAB TRANSVERSE END, BOTTOM | | 22 | ** | 562 | 22 | ** | 562 | 22 | ** | 562 | 22 | ** | 562 | 22 | ** | 562 | 22 | ** | 562 | 22 | ** | 562 |
| 5b1 | SLAB LONGITUDINAL TOP & BOTTOM | | 168 | 25'-10 | 4527 | 168 | 30'-0 | 5257 | 168 | 36'-3 | 6352 | 252 | 29'-1 | 7644 | 252 | 32'-5 | 8520 | 252 | 35'-9 | 9396 | 252 | 39'-1 | 10,273 |
| 5d2 | ABUTMENT DIAPHRAGM, LONGITUDINAL - F.F. | | 24 | 6'-4 | 159 | 24 | 6'-4 | 159 | 24 | 6'-4 | 159 | 24 | 6'-1 | 153 | 24 | 6'-3 | 157 | 24 | 6'-3 | 157 | 24 | 6'-3 | 157 |
| 5d3 | ABUTMENT DIAPHRAGM, LONGITUDINAL - F.F. | | 8 | 5'-5 | 46 | 8 | 5'-5 | 46 | 8 | 5'-5 | 46 | 8 | 5'-2 | 44 | 8 | 5'-0 | 42 | 8 | 5'-0 | 42 | 8 | 5'-0 | 42 |
| 5d5 | ABUTMENT DIAPHRAGM, LONGITUDINAL - END | | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 | 12 | 4'-7 | 58 |
| 5d6 | ABUTMENT DIAPHRAGM LONGITUDINAL - B.F. | | 8 | 33'-10 | 283 | 8 | 33'-10 | 283 | 8 | 33'-10 | 283 | 8 | 33'-10 | 283 | 8 | 33'-10 | 283 | 8 | 33'-10 | 283 | 8 | 33'-10 | 283 |
| 5d7 | PAVING NOTCH, LONGITUDINAL | | 4 | 33'-11 | 142 | 4 | 33'-11 | 142 | 4 | 33'-11 | 142 | 4 | 33'-11 | 142 | 4 | 33'-11 | 142 | 4 | 33'-11 | 142 | 4 | 33'-11 | 142 |
| 5d8 | ABUTMENT DIAPH. WING EXT. LONGIT. | | - | - | - | - | - | - | - | - | - | - | - | - | 24 | 10'-9 | 269 | 24 | 10'-9 | 269 | 24 | 10'-9 | 269 |
| 5d9 | ABUT. DIAPH. WING EXT. LONGIT. | | - | - | - | - | - | - | - | - | - | - | - | - | 24 | 10'-8 | 267 | 24 | 10'-8 | 267 | 24 | 10'-8 | 267 |
| 8f1 | ABUTMENT FOOTING LONGITUDINAL | | 18 | 33'-10 | 1626 | 18 | 33'-10 | 1626 | 18 | 33'-10 | 1626 | 18 | 33'-10 | 1626 | 18 | 33'-10 | 1626 | 18 | 33'-10 | 1626 | 18 | 33'-10 | 1626 |
| 8f3 | ABUTMENT EXTENSION LONGITUDINAL | | - | - | - | - | - | - | - | - | - | - | - | - | 16 | 10'-2 | 435 | 16 | 10'-2 | 435 | 16 | 10'-2 | 435 |
| 8f4 | ABUTMENT EXTENSION LONGITUDINAL | | - | - | - | - | - | - | - | - | - | - | - | - | 16 | 9'-0 | 385 | 16 | 9'-0 | 385 | 16 | 9'-0 | 385 |
| 8g1 | ABUTMENT VERTICAL | | 114 | 6'-7 | 2004 | 114 | 6'-7 | 2004 | 114 | 7'-2 | 2181 | 110 | 7'-8 | 2252 | 110 | 8'-5 | 2472 | 110 | 8'-5 | 2472 | 110 | 8'-5 | 2472 |
| 8g3 | ABUTMENT DIAPHRAGM VERTICAL - B.F. | | 50 | 15'-3 | 2036 | 50 | 15'-3 | 2036 | 50 | 15'-3 | 2036 | 50 | 15'-9 | 2103 | 50 | 15'-9 | 2103 | 50 | 15'-9 | 2103 | 50 | 15'-9 | 2103 |
| 6g4 | ABUT. DIAPH. WING EXT. VERT. | | - | - | - | - | - | - | - | - | - | 40 | 5'-8 | 341 | 40 | 6'-5 | 386 | 40 | 6'-5 | 386 | 40 | 6'-5 | 386 |
| 5h1 | ABUTMENT TO WING ANCHOR | | 28 | 6'-8 | 196 | 28 | 6'-8 | 196 | 28 | 6'-8 | 196 | 36 | 6'-8 | 252 | 36 | 6'-8 | 252 | 36 | 6'-8 | 252 | 36 | 6'-8 | 252 |
| 5h2 | ABUTMENT TO WING ANCHOR | | 56 | 4'-11 | 288 | 56 | 4'-11 | 288 | 56 | 4'-11 | 288 | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 |
| 5h3 | ABUTMENT TO WING ANCHOR | | 28 | 6'-9 | 198 | 28 | 6'-9 | 198 | 28 | 6'-9 | 198 | 36 | 6'-9 | 256 | 36 | 6'-9 | 256 | 36 | 6'-9 | 256 | 36 | 6'-9 | 256 |
| 5h4 | ABUTMENT TO WING ANCHOR | | - | - | - | - | - | - | - | - | - | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 | 12 | 4'-11 | 62 |
| 5j1 | SLAB TRANSV. TOP AT RAIL | | 128 | 6'-3 | 835 | 150 | 6'-3 | 978 | 182 | 6'-3 | 1187 | 216 | 6'-3 | 1409 | 242 | 6'-3 | 1578 | 270 | 6'-3 | 1761 | 296 | 6'-3 | 1930 |
| 5k1 | PAVING NOTCH, TRANSVERSE | | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 | 54 | 4'-11 | 277 |
| 5k2 | PAVING NOTCH, TRANSVERSE | | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 | 54 | 3'-5 | 193 |
| 5p1 | ABUTMENT HOOPS | | 112 | 10'-6 | 1227 | 112 | 10'-6 | 1227 | 100 | 10'-6 | 1096 | 120 | 10'-6 | 1315 | 112 | 10'-6 | 1227 | 96 | 10'-6 | 1052 | 96 | 10'-6 | 1052 |
| 5p2 | ABUTMENT EXT. HOOPS | | - | - | - | - | - | - | - | - | - | 24 | 10'-8 | 268 | 24 | 10'-8 | 268 | 24 | 10'-8 | 268 | 24 | 10'-8 | 268 |
| 6p3 | ABUT. BOTT. AT PILES | | - | - | - | - | - | - | - | - | - | 28 | 6'-8 | 281 | 32 | 6'-8 | 321 | 36 | 6'-8 | 361 | 36 | 6'-8 | 361 |
| 5p4 | ABUTMENT HOOPS AT ENDS | | 8 | 10'-9 | 90 | 8 | 10'-9 | 90 | 8 | 10'-9 | 90 | 8 | 10'-9 | 90 | 8 | 10'-9 | 90 | 8 | 10'-9 | 90 | 8 | 10'-9 | 90 |
| 5r1 | PAVING BLOCK LIFTING HOOPS | | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 | 8 | 2'-10 | 24 |
| 5s1 | WING, VERTICAL | | 64 | 5'-10 | 390 | 64 | 5'-10 | 390 | 64 | 6'-2 | 412 | 64 | 6'-11 | 462 | 64 | 7'-8 | 512 | 64 | 7'-8 | 512 | 64 | 7'-8 | 512 |
| 4t1 | UNDER BEAMS AT ABUTMENTS | | 10 | 4'-8 | 32 | 10 | 4'-8 | 32 | 10 | 4'-8 | 32 | 10 | 4'-8 | 32 | 10 | 4'-10 | 33 | 10 | 4'-10 | 33 | 10 | 4'-10 | 33 |
| #2 | PILE SPIRAL - NO. 2 BAR | | 10 | 38'-6 | 64 | 10 | 38'-6 | 64 | 12 | 38'-6 | 77 | 18 | 38'-6 | 115 | 20 | 38'-6 | 128 | 22 | 38'-6 | 141 | 22 | 38'-6 | 141 |
| | SPIRAL SPACER L _{7/8} x 7/8 x 1/8 x 0.70 | | 20 | 1'-10 | 26 | 20 | 1'-10 | 26 | 24 | 1'-10 | 32 | 36 | 1'-10 | 48 | 40 | 1'-10 | 53 | 44 | 1'-10 | 59 | 44 | 1'-10 | 59 |
| | REINFORCING STEEL (LBS.) | | | | 21269 | | | 23226 | | | 26196 | | | 32035 | | | 34651 | | | 36974 | | | 39303 |
| | SEE BARRIER RAIL DETAILS (LBS.) | | | | 4054 | | | 4410 | | | 4968 | | | 6002 | | | 6438 | | | 6875 | | | 7353 |
| | SEE OPEN RAIL DETAILS (LBS.) | | | | 4179 | | | 4572 | | | 5277 | | | 6816 | | | 7478 | | | 7900 | | | 8322 |

* VARIES FROM 4'-5 TO 29'-7 ** VARIES FROM 3'-0 TO 31'-0

| CONCRETE PLACEMENT QUANTITIES (SUPERSTRUCTURE PLUS INTEGRAL ABUTMENTS) | | | | | | | | | ESTIMATED QUANTITIES (SUPERSTRUCTURE PLUS INTEGRAL ABUTMENTS) | | | | | | | | | |
|--|-------------------|--------|-------|-------|-------|-------|-------|--------|---|---|--------|--------|--------|--------|--------|--------|--------|--------|
| SLAB, AND ABUT. DIAPHRAGM | WITH BARRIER RAIL | CU.YD. | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 | NO. OF STEEL H-PILES FOR TWO ABUTS. (HP10X57) | NO. | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 |
| SLAB, AND ABUT. DIAPHRAGM | WITH BARRIER RAIL | CU.YD. | 70.1 | 77.4 | 92.3 | 112.5 | 128.4 | 137.7 | 146.8 | NO. | 10 | 10 | 12 | 18 | 20 | 22 | 22 | 22 |
| ABUT. DIAPHRAGM | WITH OPEN RAIL | CU.YD. | 71.6 | 79.1 | 94.4 | 114.9 | 131.2 | 140.7 | 150.2 | STRUCTURAL CONCRETE, (BRIDGE) | CU.YD. | 106.4 | 113.7 | 129 | 157.8 | 174.5 | 183.8 | 192.9 |
| PAVING BLOCKS | | CU.YD. | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 | 2.5 | CONCRETE RAIL (BARRIER OR OPEN) | L.F. | 127.5 | 144.2 | 169.2 | 214.2 | 234.2 | 254.2 | 274.2 |
| ABUTMENT WINGS | | CU.YD. | 7.2 | 7.2 | 7.6 | 8.4 | 9.2 | 9.2 | 9.2 | REINFORCING STEEL | LBS. | 25,123 | 27,436 | 30,964 | 37,828 | 40,880 | 43,640 | 46,447 |
| ABUTMENT FOOTINGS | | CU.YD. | 27.6 | 27.6 | 27.6 | 35.3 | 35.3 | 35.3 | 35.3 | WITH BARRIER RAIL | LBS. | 25,448 | 27,798 | 31,473 | 38,851 | 42,129 | 44,874 | 47,625 |
| | | | | | | | | | | WITH OPEN RAIL | LBS. | 1532 | 1526 | 1570 | 1610 | 1695 | 1695 | 1695 |
| | | | | | | | | | | STRUCTURAL STEEL | LBS. | 1242 | 1235 | 1235 | 1235 | 1265 | 1265 | 1265 |



NOTE: ALL DIMENSIONS ARE OUT TO OUT. D= PIN DIAMETER

IOWADOT Highway Division

STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE

PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES

APRIL, 2012

DECK & ABUTMENT REINF. H30S1-14-12

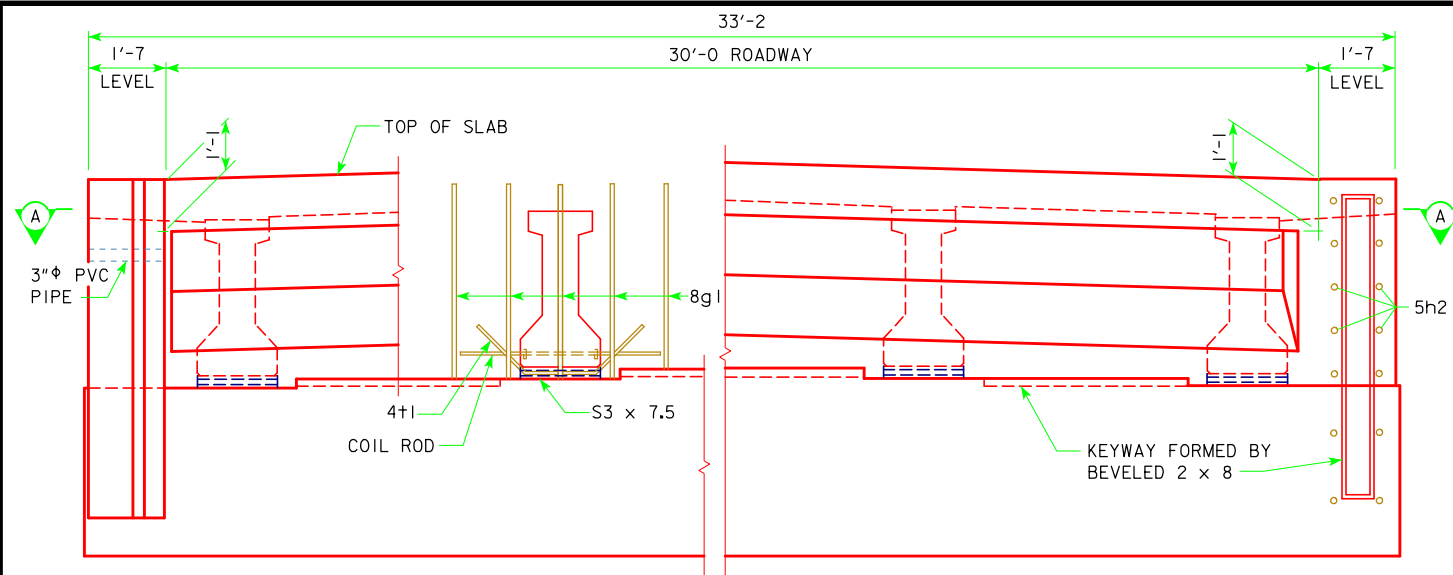
15° SKEW

LATEST REVISION DATE

11-15

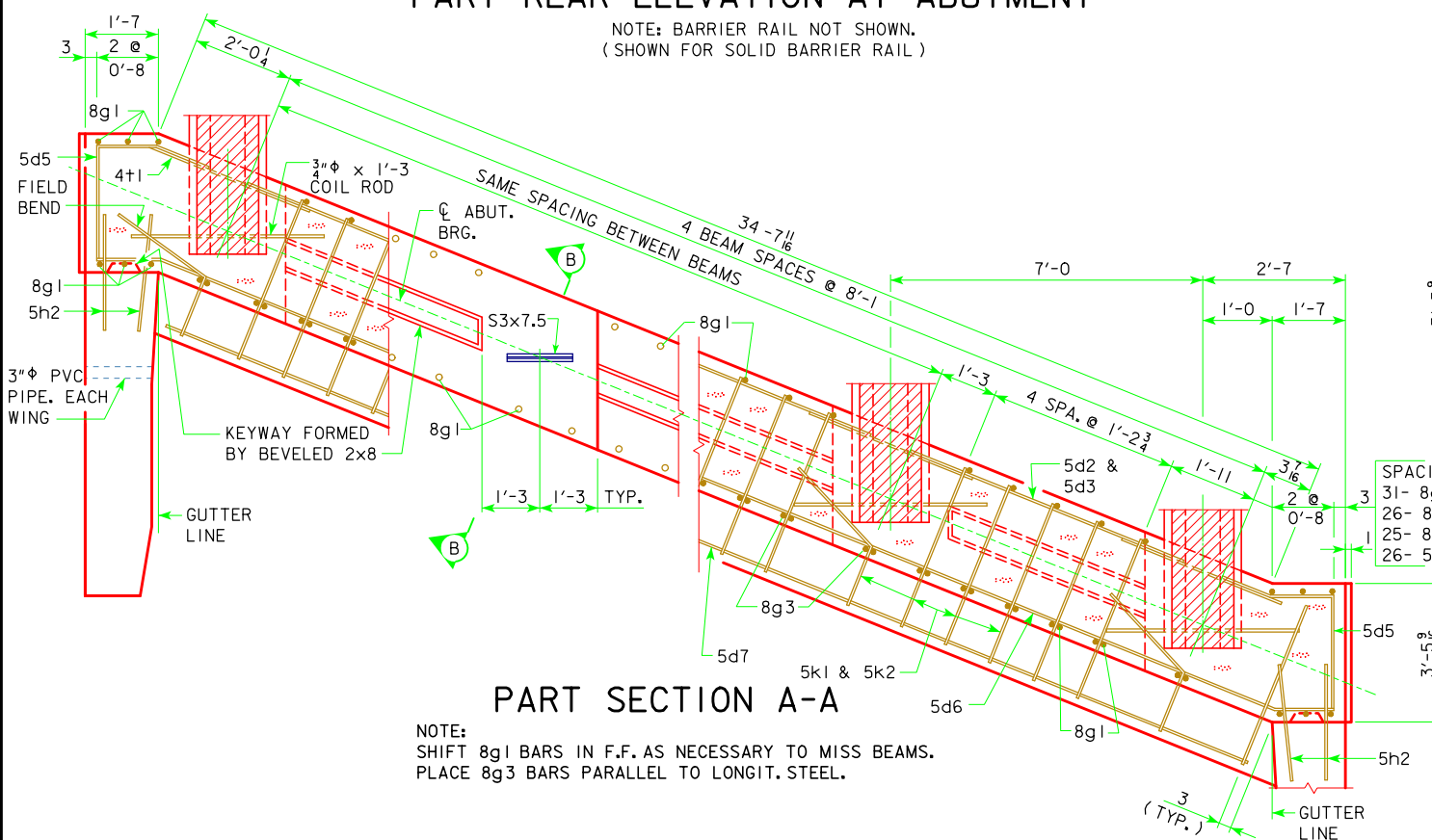
APPROVED BY BRIDGE ENGINEER

Thomas L. Mc Donald



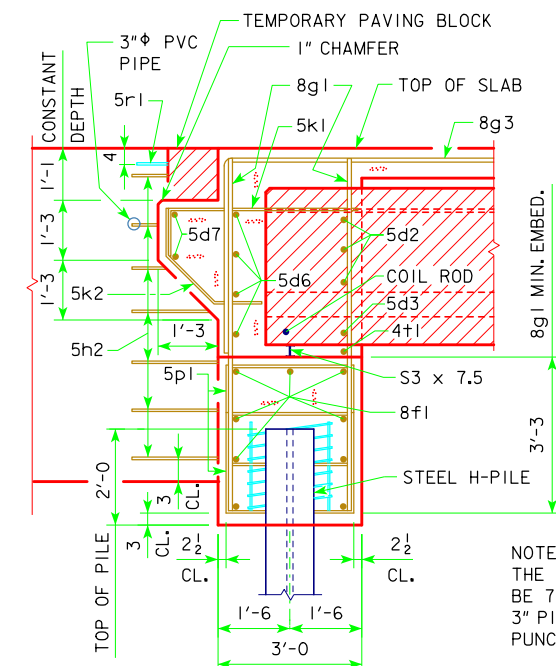
PART REAR ELEVATION AT ABUTMENT

NOTE: BARRIER RAIL NOT SHOWN.
(SHOWN FOR SOLID BARRIER RAIL)



PART SECTION A-A

NOTE:
SHIFT 8g1 BARS IN F.F. AS NECESSARY TO MISS BEAMS.
PLACE 8g3 BARS PARALLEL TO LONGIT. STEEL.



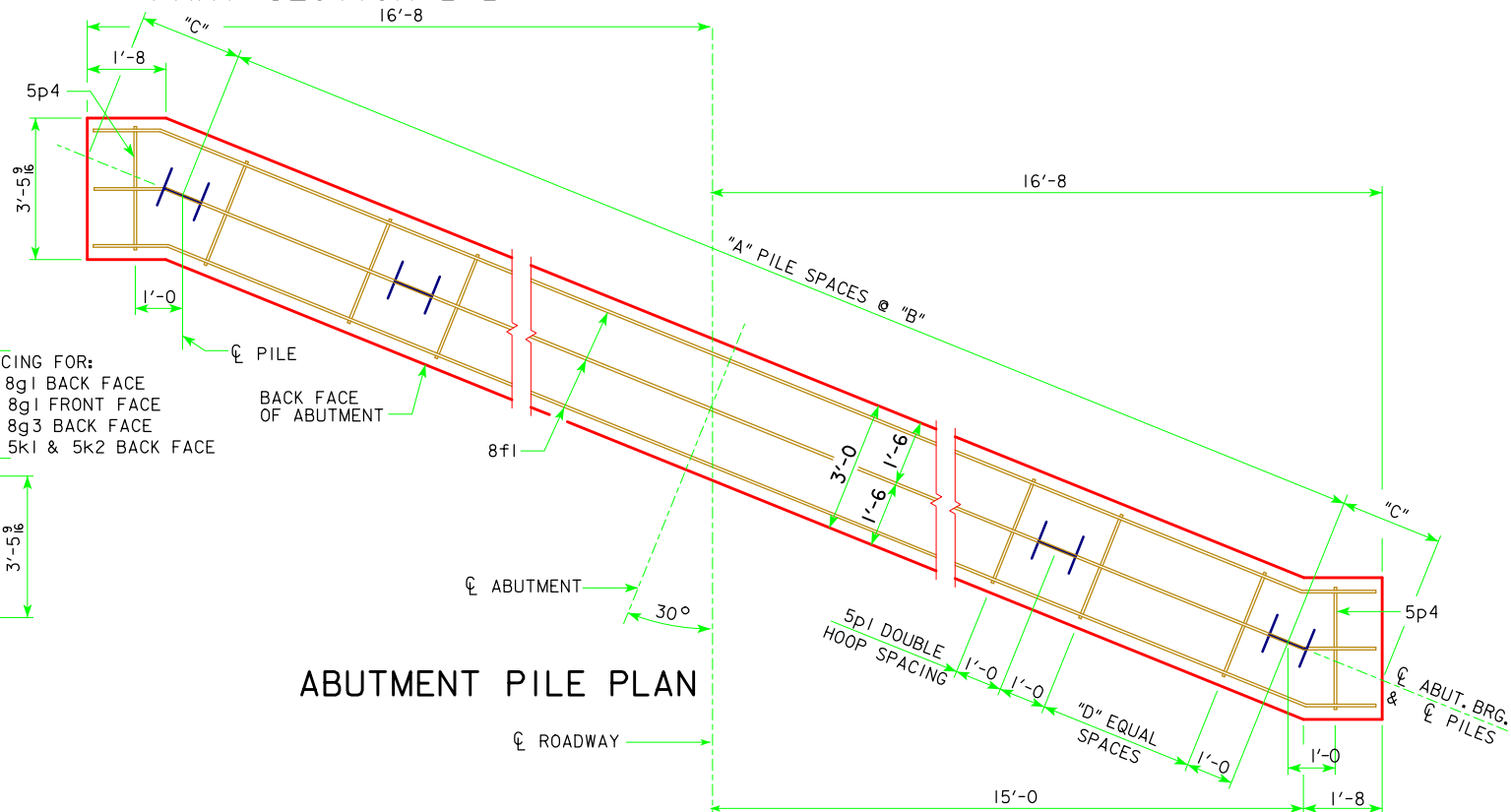
PART SECTION B-B

NOTE:
THE SPIRAL AT THE TOP OF EACH PILE TO
BE 7 TURNS OF NO. 2 BAR, 21\"/>

ABUTMENT PILE SPACING

| DIMENSION OR NO. | CL TO CL ABUTMENT BEARING | | |
|-------------------------------|---------------------------|------------|------------|
| | 46'-8 | 55'-0 | 67'-6 |
| "A" PILE SPACES | 5 | 5 | 5 |
| "B" (FT. - IN.) | 6'-11 | 6'-11 | 6'-11 |
| "C" (FT. - IN.) | 1'-11 7/16 | 1'-11 7/16 | 1'-11 7/16 |
| "D" EQUAL SPACES | 5 | 5 | 5 |
| NO. OF PILES PER ABUT. | 6 | 6 | 6 |
| STRENGTH I DESIGN LOAD (KIPS) | 111 | 119 | 137 |

NOTE: P_u, STRENGTH I DESIGN LOAD (KIPS)
IS NOT THE VALUE USED IN THE
FIELD FOR DRIVING PILES.



ABUTMENT PILE PLAN

ABUTMENT NOTES:

MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2\"/>

ABUTMENT PILES SHALL BE DRIVEN TO VALUES SHOWN IN DESIGN PLANS.

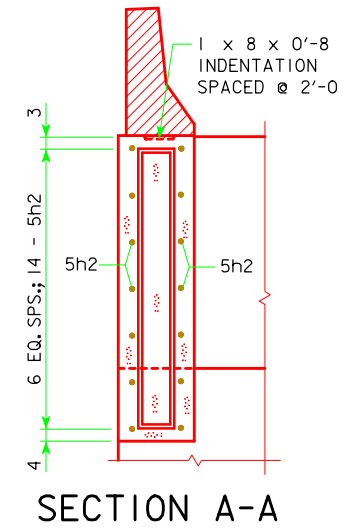
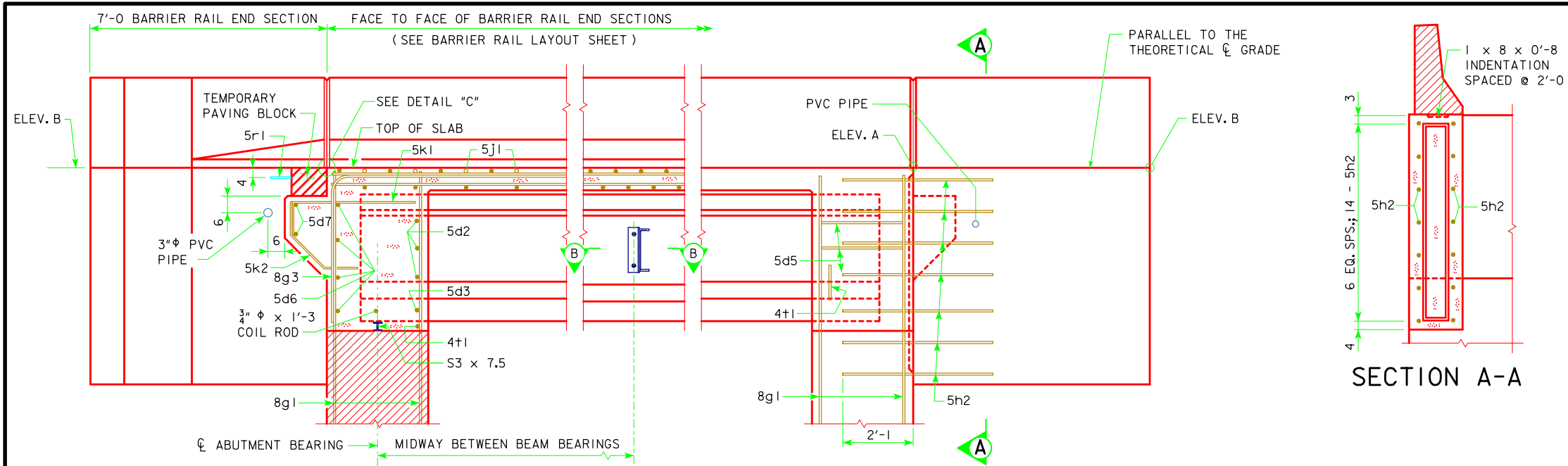
PLACE 5h2 BAR AT 1:6 SLOPE TO MATCH TRAFFIC SIDE OF ABUTMENT WING FACE. (BOTH SIDES TYPICAL)

BARRIER RAIL NOT SHOWN IN DETAILS.

IF ROCK IS CLOSER THAN 15' BELOW ABUTMENT FOOTING, SPECIAL ANALYSIS MAY BE REQUIRED.

| | | |
|--|---|--------------------|
| 05-13 LATEST REVISION DATE <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | | |
| | STANDARD DESIGN - 30° ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | ABUTMENT DETAILS 30° SKEW A & B BEAMS | H30SI-15-12 |

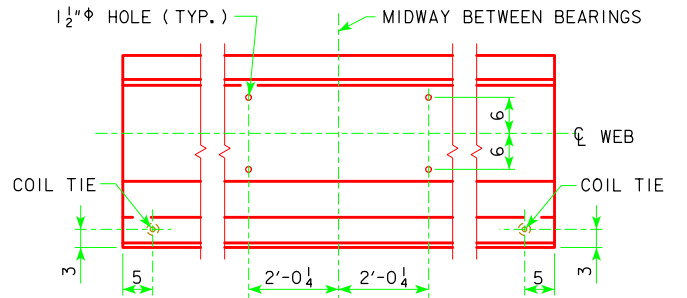
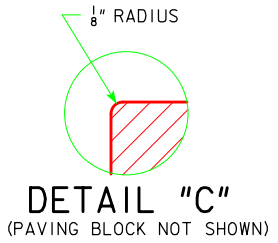
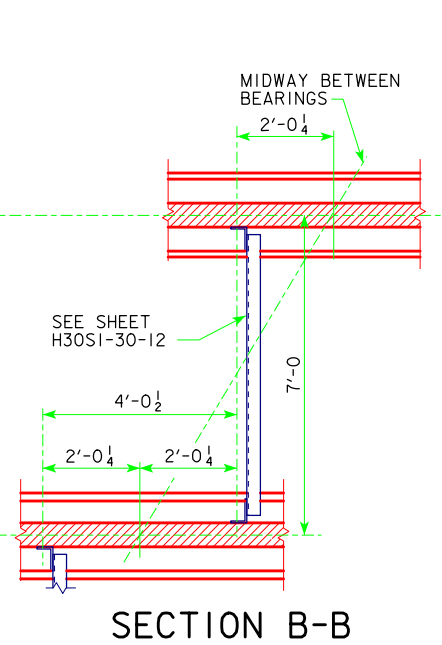
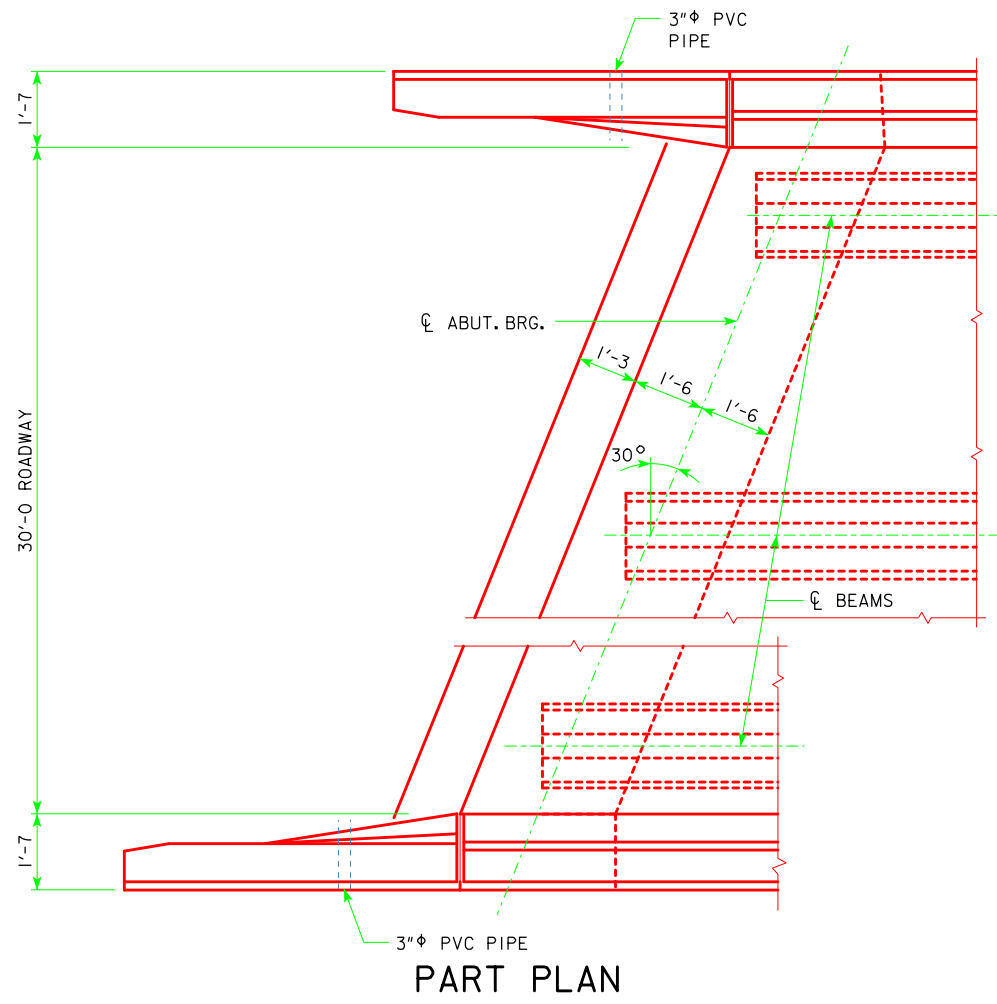
REVISED 05-13 - REVISION FOR LRFD PILE DESIGN.



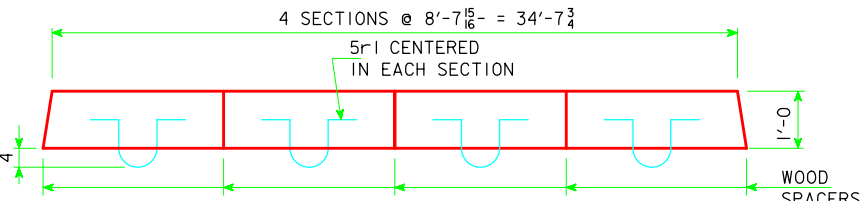
PART LONGITUDINAL SECTION NEAR GUTTER
(FOR DETAILS OF INTERMEDIATE DIAPHRAGM SEE SHEET H30SI-30-12)

PART END VIEW AT ABUTMENT
PROVIDE ELEVATIONS A AND B IN THE BRIDGE PLAN SHEETS.

NOTE: PLUG 3" ϕ PVC PIPE WITH EXPANDING FOAM PRIOR TO BACKFILLING BEHIND ABUTMENTS.

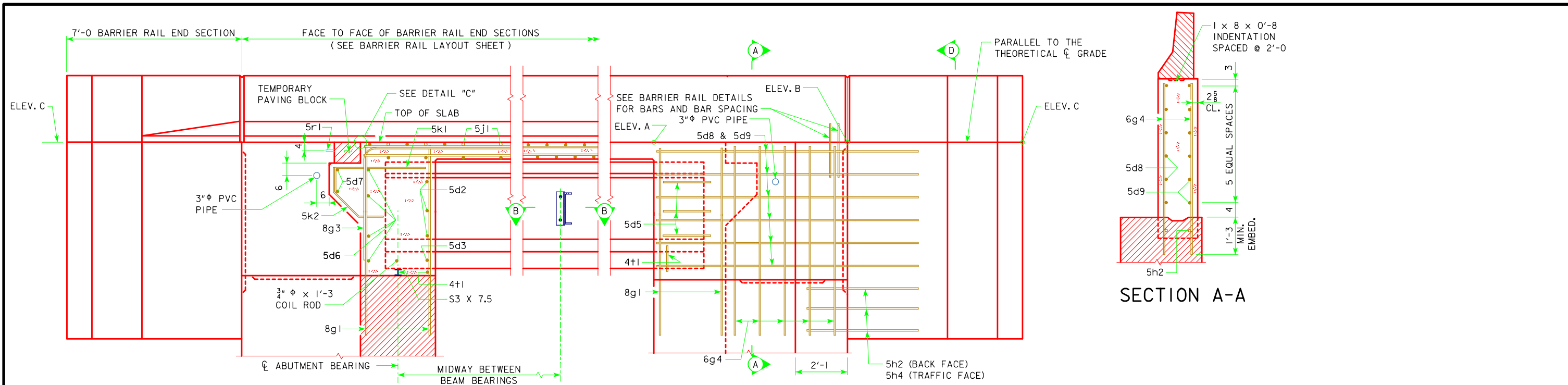


LOCATION OF BEAM COIL TIES AND STEEL DIAPHRAGM BOLT HOLES



PLAN OF TEMPORARY PAVING BLOCK
NOTE: LINE PAVING NOTCH WITH TAR PAPER BEFORE PLACING THE TEMPORARY PAVING BLOCK.

| | | | |
|----------------------|---------------------------------|---|--------------------|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | LONGITUDINAL SECTION 30° SKEW A & B BEAMS | H30SI-17-12 |
| | | | |



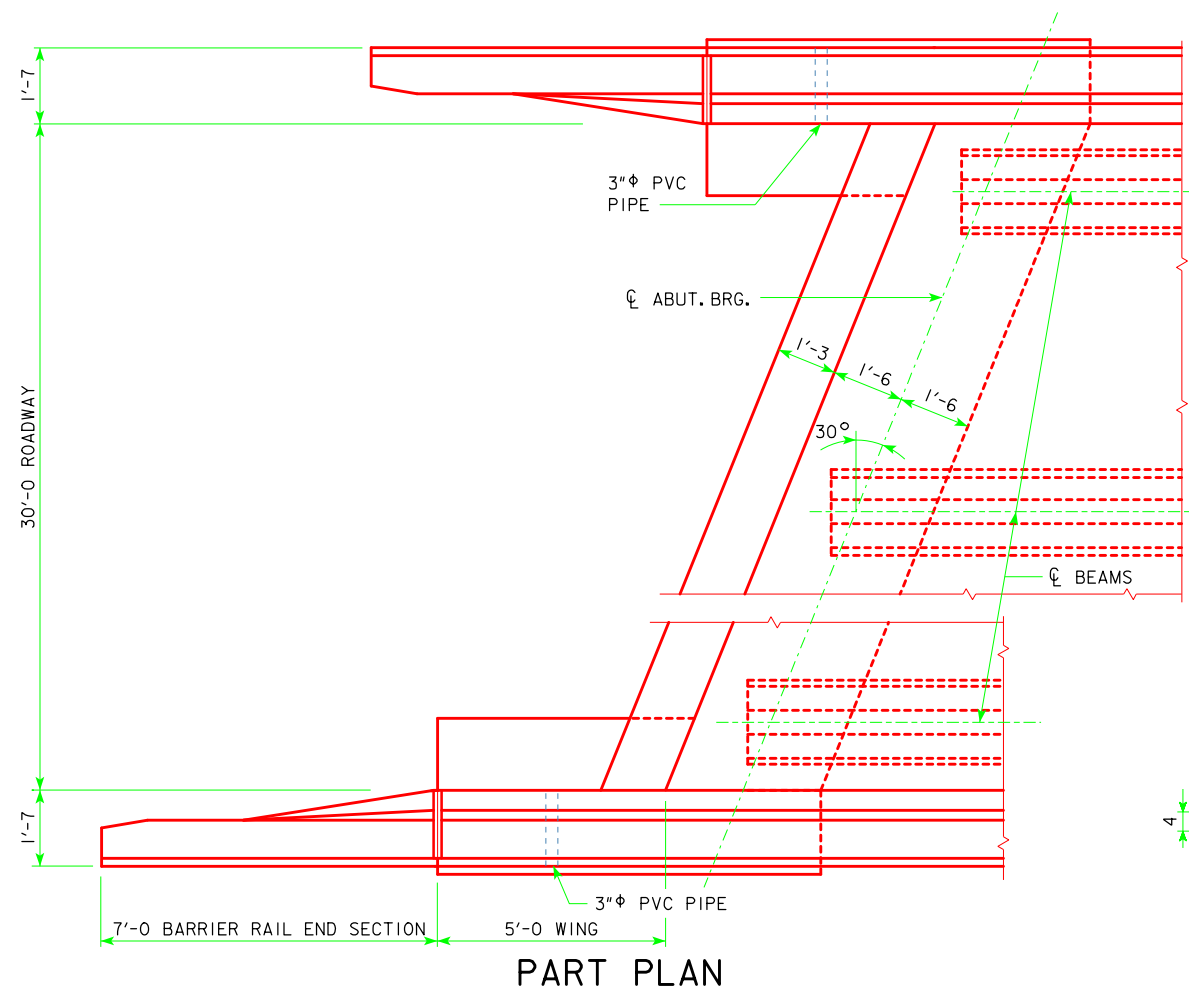
PART LONGITUDINAL SECTION NEAR GUTTER

(FOR DETAILS OF INTERMEDIATE DIAPHRAGM SEE SHEET H30SI-30-12)

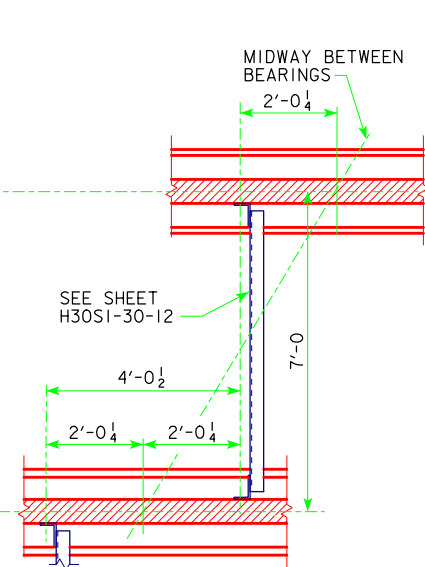
PART END VIEW AT ABUTMENT

PROVIDE ELEVATIONS A, B, AND C IN THE BRIDGE PLAN SHEETS.

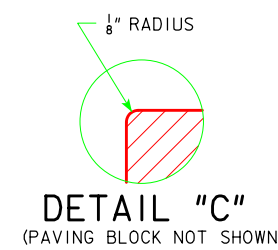
NOTE: PLUG 3" ϕ PVC PIPE WITH EXPANDING FOAM PRIOR TO BACKFILLING BEHIND ABUTMENTS.



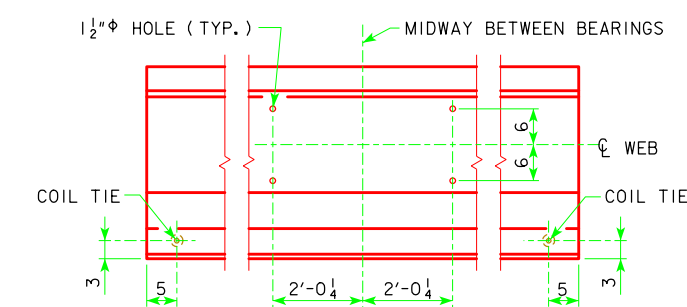
PART PLAN



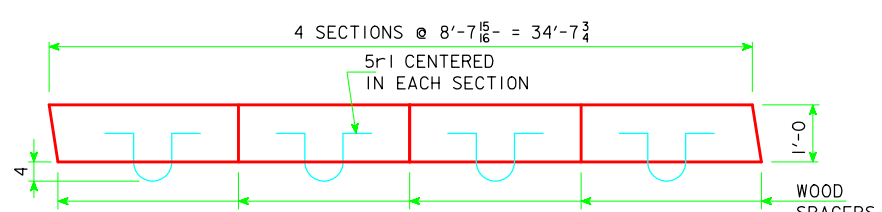
SECTION B-B



DETAIL "C"
(PAVING BLOCK NOT SHOWN)

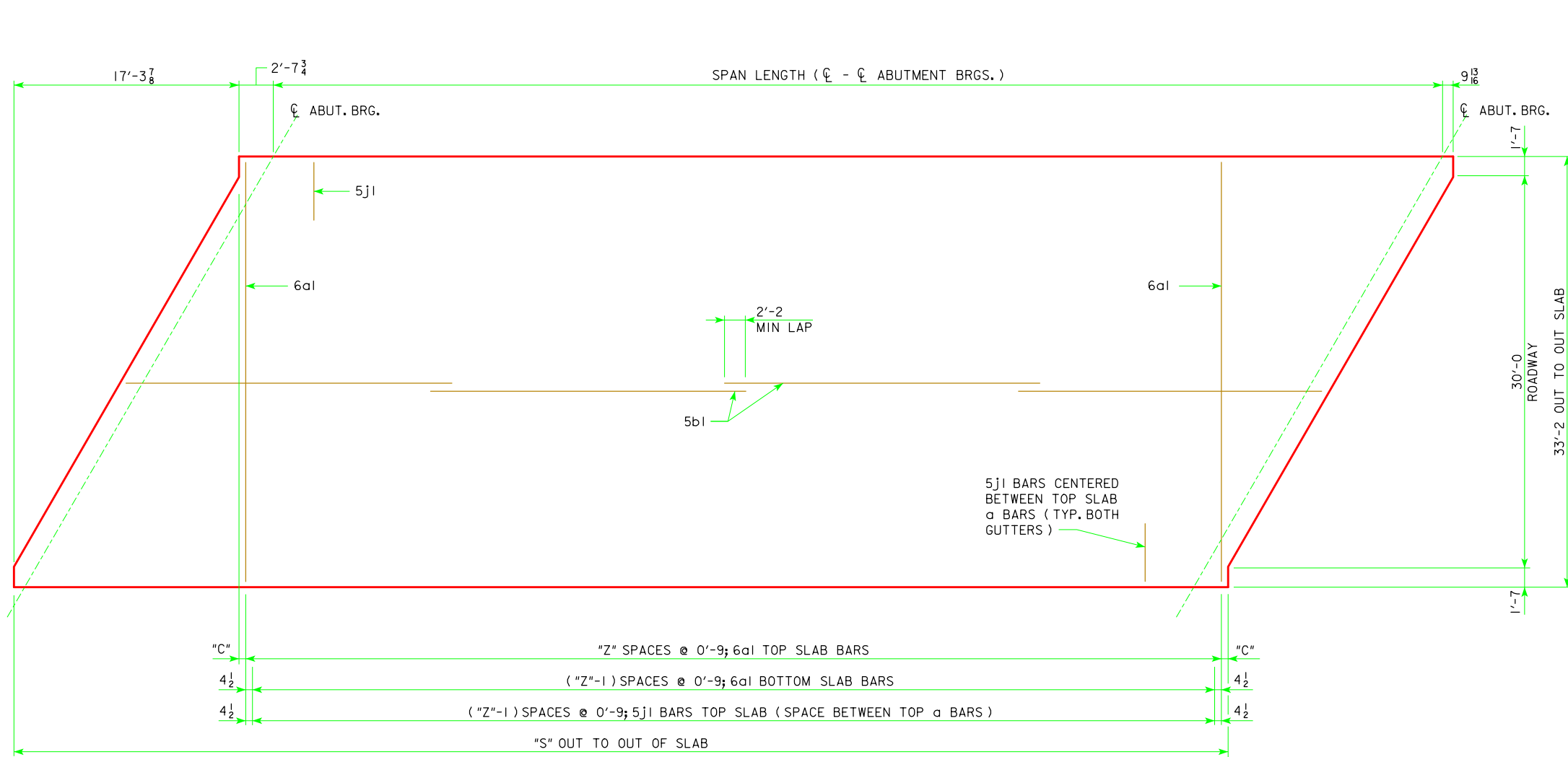


LOCATION OF BEAM COIL TIES AND STEEL DIAPHRAGM BOLT HOLES

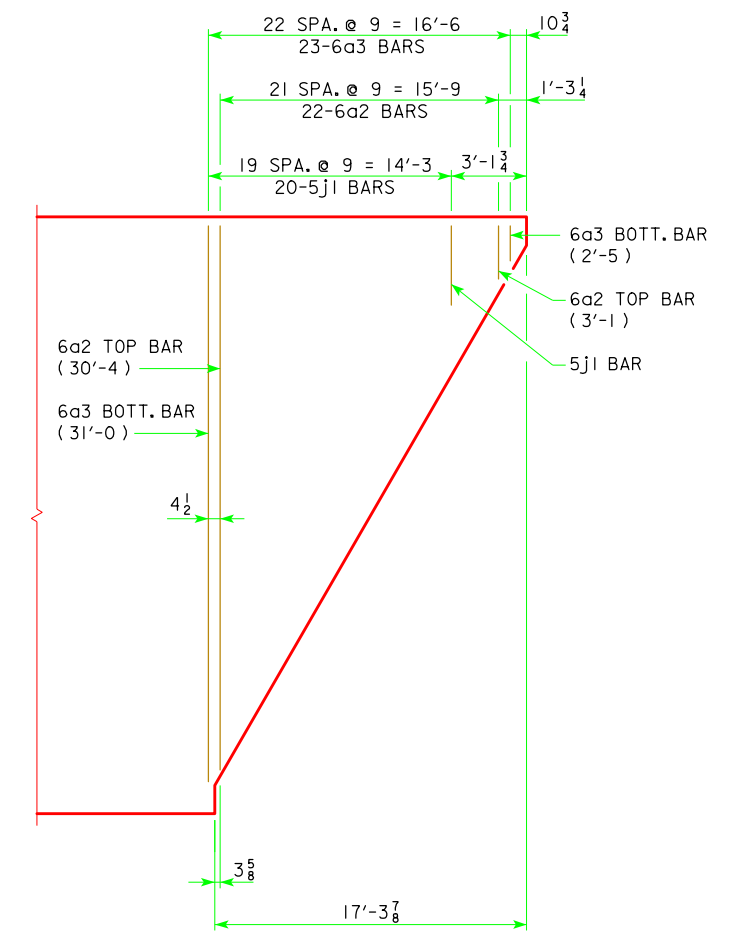


PLAN OF TEMPORARY PAVING BLOCK
NOTE: LINE PAVING NOTCH WITH TAR PAPER BEFORE PLACING THE TEMPORARY PAVING BLOCK.

| | |
|---|---|
| LATEST REVISION DATE APPROVED BY BRIDGE ENGINEER <i>Thomas L. Mc Donald</i> | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 |
| | LONGITUDINAL SECTION H30SI-18-12 30° SKEW C & D BEAMS |



SLAB LAYOUT
(LEFT AHEAD SKEW SHOWN, RIGHT AHEAD SKEW SIMILAR)



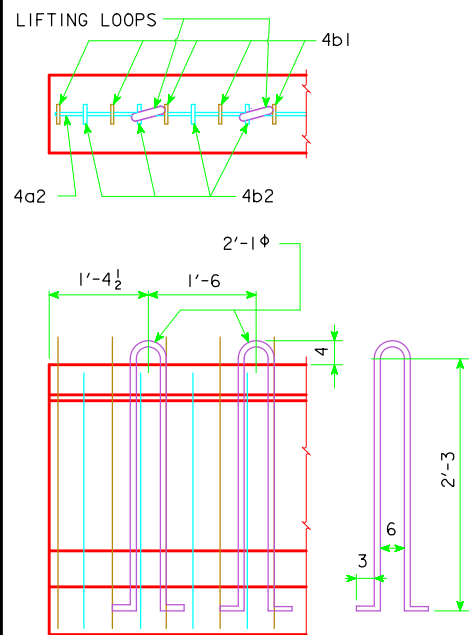
END OF SLAB REINFORCING
(TYPICAL EACH END OF DECK)

GENERAL DATA

| | | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 |
|--|---|------------------------------------|------------------------------------|-------------------------------------|------------------------------------|------------------------------------|-------------------------------------|-------------------------------------|
| SPAN LENGTH (C - C ABUTMENT BRGS.) | | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 |
| LOCATION OF EXTREME 6a1 TOP BAR FROM END OF SLAB | "C" | 3 ³ / ₈ | 3 ⁷ / ₈ | 2 ³ / ₈ | 5 ⁵ / ₈ | 2 ³ / ₈ | 3 ⁷ / ₈ | 5 ³ / ₈ |
| NO. OF SPACES FOR 6a1 TOP BARS | "Z" | 43 | 54 | 71 | 87 | 101 | 114 | 127 |
| OUT TO OUT OF SLAB | "S" | 50'-1 ⁹ / ₁₆ | 58'-5 ⁹ / ₁₆ | 70'-11 ⁹ / ₁₆ | 83'-5 ⁹ / ₁₆ | 93'-5 ⁹ / ₁₆ | 103'-5 ⁹ / ₁₆ | 113'-5 ⁹ / ₁₆ |
| VERTICAL CURVE | TOP OF SLAB TO ABUTMENT TOP AT C ABUTMENT BEARING | "U" | 3'-8 ¹ / ₈ | 3'-8 ¹ / ₂ | 4'-3 ⁷ / ₈ | 4'-9 ⁷ / ₁₆ | 5'-6 ³ / ₁₆ | 5'-6 ¹¹ / ₁₆ |
| STRAIGHT GRADE | TOP OF SLAB TO ABUTMENT TOP AT C ABUTMENT BEARING | "U" | 3'-8 ⁵ / ₁₆ | 3'-8 ³ / ₄ | 4'-4 ³ / ₁₆ | 4'-9 ⁵ / ₁₆ | 5'-6 ¹³ / ₁₆ | 5'-7 ⁷ / ₁₆ |
| SERVICE D.L. ABUTMENT REACTION (D.L. + F.W.S.) SERVICE LOADS | | KIPS | 314.7 | 341.8 | 406.9 | 521.6 | 595.9 | 637.0 |
| SERVICE L.L. ABUTMENT REACTION (HL-93) NO IMPACT SERVICE LOADS | | KIPS | 152.8 | 162.4 | 175.0 | 186.0 | 194.4 | 209.8 |

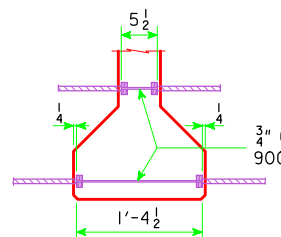
| | | | |
|-------------------------------|---|---|--------------------|
| 05-13 LATEST REVISION DATE | <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | SUPERSTRUCTURE 30° SKEW | H30S1-19-12 |

REVISED 05-13 - REVISION FOR LRFD PILE DESIGN.



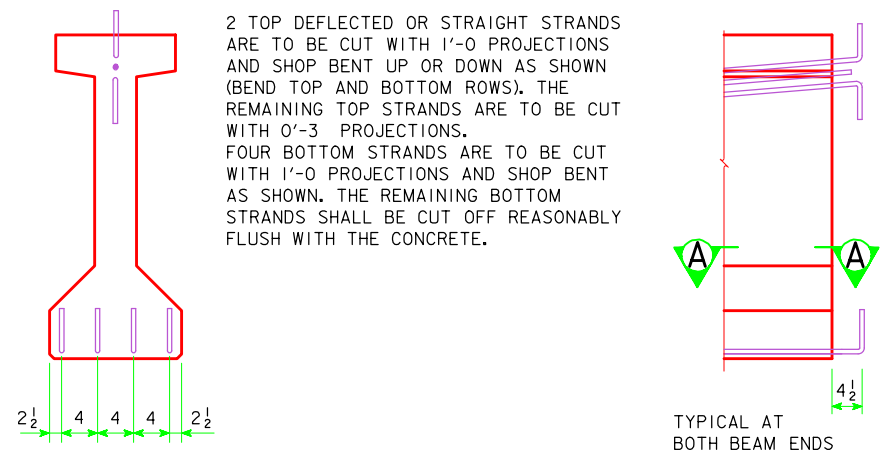
LIFTING LOOP DETAIL

ALTERNATE TYPES MAY BE SUBSTITUTED WITH THE APPROVAL OF THE ENGINEER. LIFTING LOOPS ARE TO BE STRUCTURAL GRADE.



COIL TIE DETAIL

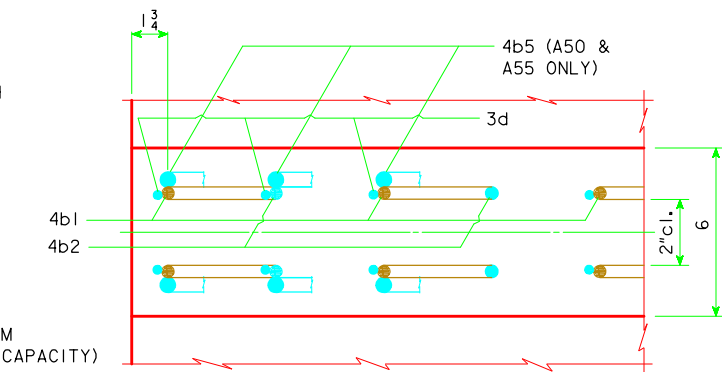
NUMBER AND EXACT LOCATION OF COIL TIES TO BE AS DETAILED ON SPECIFIC BRIDGE DESIGN.



STRAND PROJECTION AT BEAM ENDS WHEN EMBEDDED IN CONCRETE END DIAPHRAGMS

2 TOP DEFLECTED OR STRAIGHT STRANDS ARE TO BE CUT WITH 1'-0 PROJECTIONS AND SHOP BENT UP OR DOWN AS SHOWN (BEND TOP AND BOTTOM ROWS). THE REMAINING TOP STRANDS ARE TO BE CUT WITH 0'-3 PROJECTIONS. FOUR BOTTOM STRANDS ARE TO BE CUT WITH 1'-0 PROJECTIONS AND SHOP BENT AS SHOWN. THE REMAINING BOTTOM STRANDS SHALL BE CUT OFF REASONABLY FLUSH WITH THE CONCRETE.

TYPICAL AT BOTH BEAM ENDS



SECTION A-A SHOWING PLACEMENT OF STIRRUPS NEAR END OF BEAM

A BEAM DATA

| BEAM | SPAN LENGTH @ BEARING | OVERALL BEAM LENGTH (L) | STRAND SIZE DIA. (inches) | NO. OF STRANDS | | TOTAL INITIAL PRESTRESS KIPS | HOLD DOWN FORCE-KIPS | CAMBER (in.) | | DEFLECTION (in.) Δ _D | | WEIGHT (TONS) | CONCRETE (C. Y.) | REINFORCING STEEL-(lb) | | |
|------|-----------------------|-------------------------|---------------------------|----------------|-----------|------------------------------|----------------------|--------------|--------------|------------------------------------|--------------|---------------|------------------|------------------------|-------------------------------|--------------|
| | | | | STRAIGHT | DEFLECTED | | | AT RELEASE | AFTER LOSSES | IMMEDIATE (ELASTIC) Δ _E | | | | | TIME (PLASTIC) Δ _T | |
| | | | | | | | | | | STEEL DIAPH. | STEEL DIAPH. | | | | STEEL DIAPH. | STEEL DIAPH. |
| A46 | 46'-8 | 47'-8 | 0.60 | 8 | 2 | 426 | 8.5 | 0.76 | 1.35 | 0.44 | 0.11 | 7.7 | 3.82 | 488 | | |
| A55 | 55'-0 | 56'-0 | 0.60 | 10 | 3 | 553 | 10.8 | 1.29 | 2.30 | 0.82 | 0.21 | 9.1 | 4.49 | 547 | | |

- ① DEFLECTIONS AT MID-SPAN DUE TO WEIGHT OF SLAB AND DIAPHRAGM.
- ② DEFLECTIONS DUE TO THE COMBINED EFFECT OF CREEP DUE TO WEIGHT OF SLAB AND SHRINKAGE OF SLAB.
- ③ TOTAL INITIAL PRESTRESS IS BASED ON 72.6% f's, f's = 270 ksi AND A_s = 0.217 sq. in.

DESIGN STRESSES:

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE TO BE IN ACCORDANCE WITH A.A.S.H.T.O. LRFD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2007: REINFORCING STEEL IN ACCORDANCE WITH SECTION 5, GRADE 60. CONCRETE IN ACCORDANCE WITH SECTION 5. MINIMUM CONCRETE f'c (AT 28 DAYS) SHALL BE 7,000 psi. MINIMUM f'ci AT RELEASE SHALL BE 6,000 psi. PRESTRESSING STEEL IN ACCORDANCE WITH SECTION 5, f's = 270,000 psi.

SPECIFICATIONS:

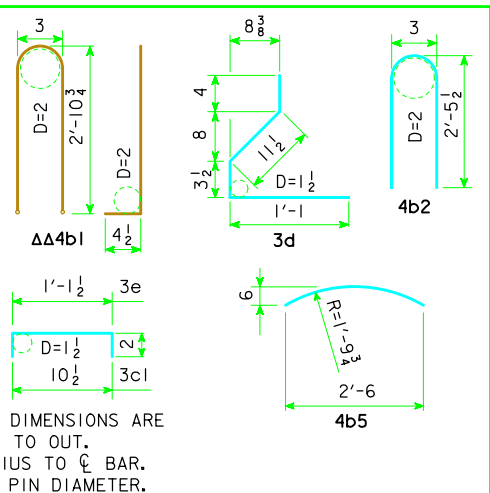
CONSTRUCTION: STANDARD SPECIFICATIONS OF THE IOWA DEPARTMENT OF TRANSPORTATION, CURRENT SERIES, WITH CURRENT APPLICABLE SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS. DESIGN: A.A.S.H.T.O. LRFD, SERIES OF 2007, WITH MINOR MODIFICATIONS.

BEAM NOTES:

- THESE BEAMS ARE DESIGNED FOR AASHTO HL-93 LIVE LOADS AS WITH AN ALLOWANCE OF 20 LB. PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE.
- ALL PPC BEAMS SHALL USE HIGH PERFORMANCE CONCRETE (HPC) IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. HOLD DOWN POINTS FOR DEFLECTED STRANDS MAY BE MOVED TOWARD ENDS OF BEAM A DISTANCE OF 0.05 L MAXIMUM AT PRODUCER'S OPTION.
- ALL PRESTRESSING STRANDS SHALL CONFORM TO ASTM A416 GRADE 270 LOW RELAXATION STRANDS.
- TOPS OF BEAMS ARE TO BE STRUCK OFF LEVEL AND FINISHED AS PER MATERIALS IM570.
- BEARINGS SHALL BE AS DETAILED ON OTHER DESIGN SHEETS.
- BEAMS SHALL BE AT LEAST 28 DAYS OLD BEFORE THE SLAB IS PLACED EXCEPT AS OTHERWISE APPROVED BY THE ENGINEER.
- THE PORTIONS OF THE PRESTRESS BEAMS THAT ARE TO BE EMBEDDED IN THE ABUTMENT SHALL BE ROUGHENED FOR A DISTANCE OF 10" FROM THE BEAM END BY SANDBLASTING OR OTHER APPROVED METHODS TO PROVIDE SUITABLE BOND BETWEEN THE BEAM AND THE DIAPHRAGM IN ACCORDANCE WITH ARTICLE 2403.03, I, OF THE STANDARD SPECIFICATIONS.
- ALL BEAMS ARE TO BE INCREASED IN LENGTH TO COMPENSATE FOR ELASTIC SHORTENING, CREEP AND SHRINKAGE.
- HOLES MUST BE CAST IN THE WEB TO ACCOMMODATE THE STEEL DIAPHRAGM ATTACHMENTS AS DETAILED ON THE STEEL DIAPHRAGM DETAIL SHEET.
- 0.6" DIAMETER STRANDS STRESSED TO NOT MORE THAN 5,000 LBS. EACH MAY BE USED IN LIEU OF THE α BARS WHICH RUN THE FULL LENGTH OF THE BEAM IN THE TOP FLANGE.

REINFORCING BAR LIST

| BEAM | SPAN | A46 | | A55 | |
|------|------|--------|--------|-------|--------|
| | | NO. | LENGTH | NO. | LENGTH |
| 5a1 | 4 | 24'-11 | 4 | 29'-1 | |
| 4a2 | 2 | 3'-3 | 2 | 3'-3 | |
| 4b1 | 44 | 6'-8 | 50 | 6'-8 | |
| 4b2 | 12 | 5'-0 | 8 | 5'-0 | |
| 4b5 | | | 12 | 2'-9 | |
| 3c1 | 44 | 1'-3 | 50 | 1'-3 | |
| 3d | 112 | 2'-8 | 116 | 2'-8 | |
| 3e | 20 | 1'-6 | 18 | 1'-6 | |

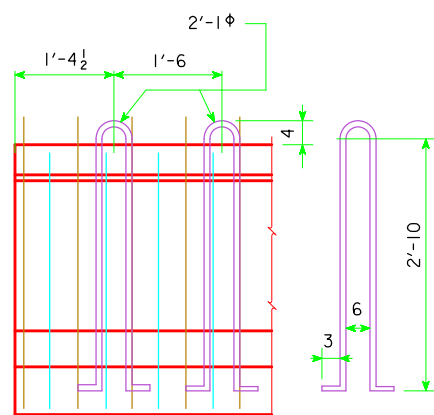
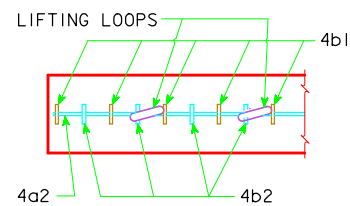


ALL DIMENSIONS ARE OUT TO OUT. RADIUS TO C. BAR. D = PIN DIAMETER.

ΔΔ 4b1 BARS TO BE EPOXY COATED.

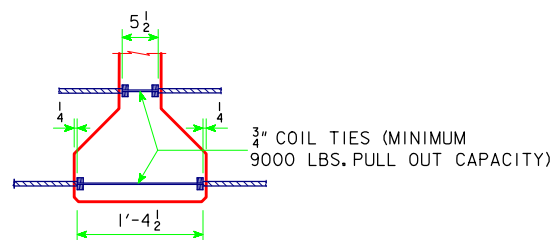
** WHERE DEFLECTING STRANDS INTERFERE WITH PLACEMENT, SOME IN-PLACE BENDING MAY BE NECESSARY.

| | | | |
|-----------------------|---|--|--|
| LATEST REVISION DATE | <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | IOWA DOT Highway Division | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE | |
| | | PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| A BEAM DETAILS | | H30SI-21-12 | |



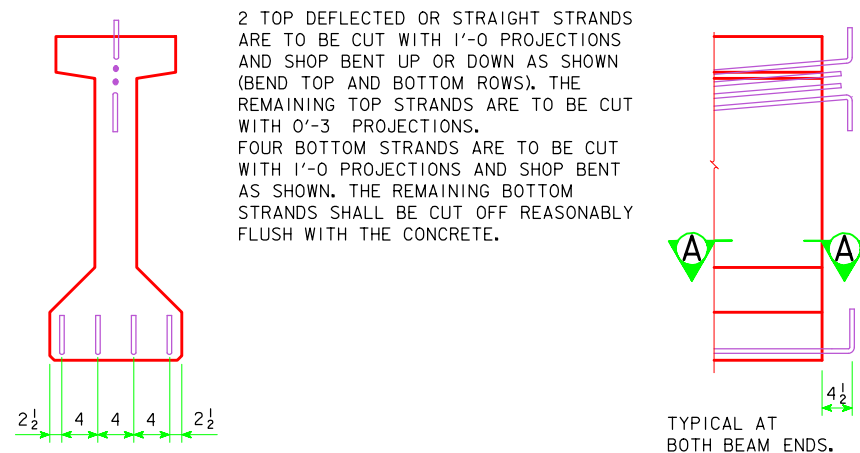
LIFTING LOOP DETAIL

ALTERNATE TYPES MAY BE SUBSTITUTED WITH THE APPROVAL OF THE ENGINEER. LIFTING LOOPS ARE TO BE STRUCTURAL GRADE.

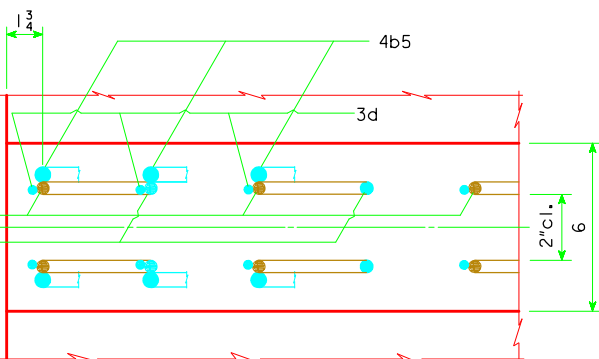


COIL TIE DETAIL

NUMBER AND EXACT LOCATION OF COIL TIES TO BE AS DETAILED ON SPECIFIC BRIDGE DESIGN.



STRAND PROJECTION AT BEAM ENDS WHEN EMBEDDED IN CONCRETE END DIAPHRAGMS



SECTION A-A SHOWING PLACEMENT OF STIRRUPS NEAR END OF BEAM

2 TOP DEFLECTED OR STRAIGHT STRANDS ARE TO BE CUT WITH 1'-0 PROJECTIONS AND SHOP BENT UP OR DOWN AS SHOWN (BEND TOP AND BOTTOM ROWS). THE REMAINING TOP STRANDS ARE TO BE CUT WITH 0'-3 PROJECTIONS. FOUR BOTTOM STRANDS ARE TO BE CUT WITH 1'-0 PROJECTIONS AND SHOP BENT AS SHOWN. THE REMAINING BOTTOM STRANDS SHALL BE CUT OFF REASONABLY FLUSH WITH THE CONCRETE.

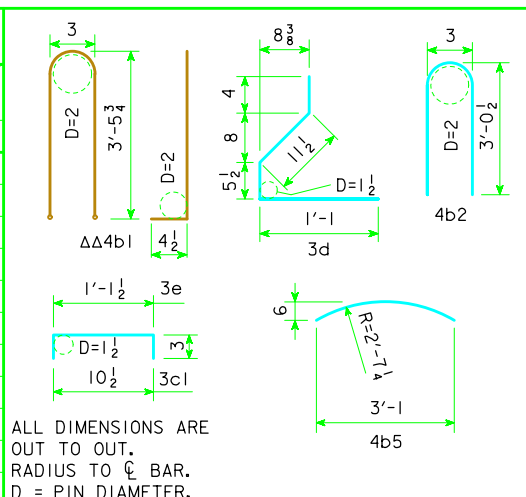
TYPICAL AT BOTH BEAM ENDS.

ΔΔ4b1 BARS TO BE EPOXY COATED.

** WHERE DEFLECTING STRANDS INTERFERE WITH PLACEMENT, SOME IN-PLACE BENDING MAY BE NECESSARY.

REINFORCING BAR LIST

| BEAM | SPAN | B67 | | |
|--------|-------|-------|--------|--|
| | | 67'-6 | | |
| BAR | SHAPE | NO. | LENGTH | |
| 6a1 | | 4 | 35'-7 | |
| 4a2 | | 2 | 4'-2 | |
| ΔΔ 4b1 | | 56 | 7'-10 | |
| 4b2 | | 12 | 6'-2 | |
| 4b5 | | 12 | 3'-3 | |
| 3c1 | | 56 | 1'-5 | |
| ** 3d | | 136 | 2'-10 | |
| 3e | | 24 | 1'-8 | |



ALL DIMENSIONS ARE OUT TO OUT. RADIUS TO C.C. BAR. D = PIN DIAMETER.

B BEAM DATA

| BEAM | SPAN LENGTH C-C BEARING | OVERALL BEAM LENGTH (L) | STRAND SIZE DIA. (inches) | NO. OF STRANDS | | TOTAL INITIAL PRESTRESS KIPS | HOLD DOWN FORCE-KIPS | CAMBER (in.) | | DEFLECTION (in.) Δ _D | | WEIGHT (TONS) | CONCRETE (C. Y.) | REINFORCING STEEL-(lb) |
|------|-------------------------|-------------------------|---------------------------|----------------|-----------|------------------------------|----------------------|--------------|--------------|---|--|---------------|------------------|------------------------|
| | | | | STRAIGHT | DEFLECTED | | | AT RELEASE | AFTER LOSSES | IMMEDIATE ^① (ELASTIC) Δ _T | TIME ^② (PLASTIC) Δ _T | | | |
| | | | | | | | | | | | | | | |
| B67 | 67'-6 | 68'-6 | 0.60 | 14 | 3 | 723 | 11.6 | 1.69 | 2.98 | 1.02 | 0.25 | 13.6 | 6.74 | 778 |

- ① DEFLECTIONS AT MID-SPAN DUE TO WEIGHT OF SLAB AND DIAPHRAGM.
- ② DEFLECTIONS DUE TO THE COMBINED EFFECT OF CREEP DUE TO WEIGHT OF SLAB AND SHRINKAGE OF SLAB.
- TOTAL BEAM DEFLECTIONS AT C/C OF SPAN, Δ_D, DUE TO WEIGHT OF SLAB AND DIAPHRAGMS FOR DETAILING PURPOSE: (A) Δ_D = Δ₁ + Δ_T FOR SIMPLE SPAN.
- ③ TOTAL INITIAL PRESTRESS IS BASED ON 72.6% f'_s, f'_s = 270 ksi AND A_s = 0.217 sq. in.

DESIGN STRESSES:

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE TO BE IN ACCORDANCE WITH A.A.S.H.T.O. LRFD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2007: REINFORCING STEEL IN ACCORDANCE WITH SECTION 5, GRADE 60. CONCRETE IN ACCORDANCE WITH SECTION 5. MINIMUM CONCRETE f'_c (AT 28 DAYS) SHALL BE 7,000 psi. MINIMUM f'_{ci} AT RELEASE SHALL BE 6,000 psi.

PRESTRESSING STEEL IN ACCORDANCE WITH SECTION 5, f'_s = 270,000 psi.

SPECIFICATIONS:

CONSTRUCTION: STANDARD SPECIFICATIONS OF THE IOWA DEPARTMENT OF TRANSPORTATION, CURRENT SERIES, WITH CURRENT APPLICABLE SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS. DESIGN: A.A.S.H.T.O. LRFD, SERIES OF 2007, WITH MINOR MODIFICATIONS.

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THE PORTIONS OF THE PRESTRESS BEAMS THAT ARE TO BE EMBEDDED IN THE ABUTMENT SHALL BE ROUGHENED FOR A DISTANCE OF 10" FROM THE BEAM END BY SANDBLASTING OR OTHER APPROVED METHODS TO PROVIDE SUITABLE BOND BETWEEN THE BEAM AND THE DIAPHRAGM IN ACCORDANCE WITH ARTICLE 2403.03, I, OF THE STANDARD SPECIFICATIONS.

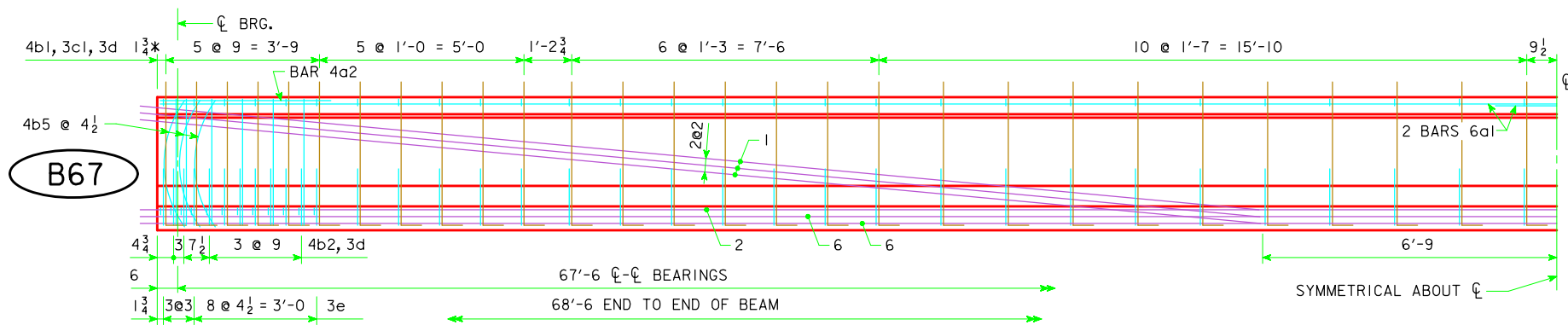
ALL BEAMS ARE TO BE INCREASED IN LENGTH TO COMPENSATE FOR ELASTIC SHORTENING, CREEP AND SHRINKAGE.

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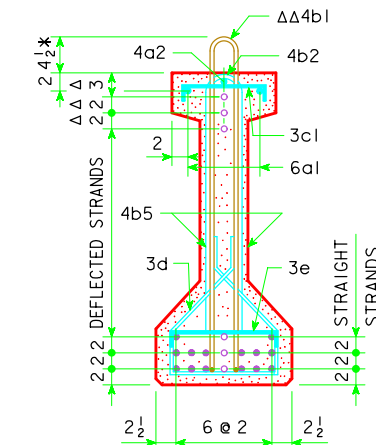
0.6" DIAMETER STRANDS STRESSED TO NOT MORE THAN 5,000 LBS. EACH MAY BE USED IN LIEU OF THE α BARS WHICH RUN THE FULL LENGTH OF THE BEAM IN THE TOP FLANGE.

| | | | |
|----------------------|--|---|--------------------|
| LATEST REVISION DATE | | IOWA DOT Highway Division | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE | |
| | | PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES | |
| | | APRIL, 2012 | |
| | | B BEAM DETAILS | H30SI-23-12 |

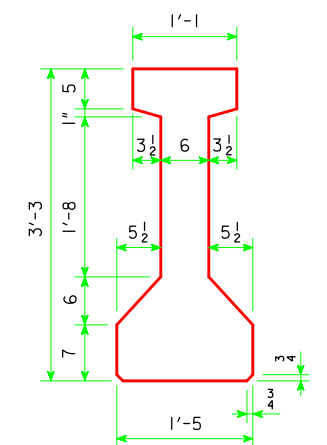
NOTE: DIMENSIONS FOR THE LOCATION OF THE DEFLECTED STRANDS ARE AT \bar{C} BEAM AND END OF BEAM.



B67



BEAM B67

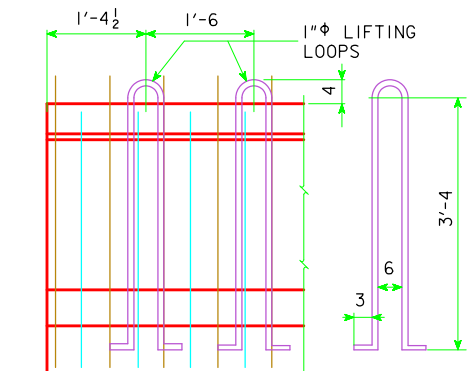
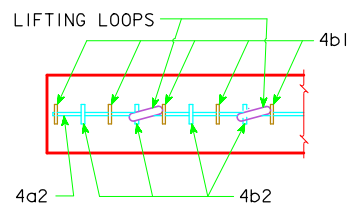


"B" BEAM CROSS SECTION

A = 382.5 in.²
 Y_b = 17.06 in.
 I = 62,000 in.⁴

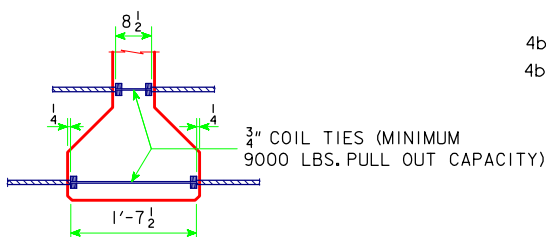
- NOTE: BARS 3d AND 4b5 ARE TO BE PLACED IN PAIRS.
- DEFLECTED STRANDS
 - * KEEP
 - △ DIMENSIONS AT END OF BEAM
 - △△ EPOXY COATED BARS

| | | | |
|----------------------|---------------------------------|---|-------------|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | B67 BEAM DETAILS | H30SI-24-12 |



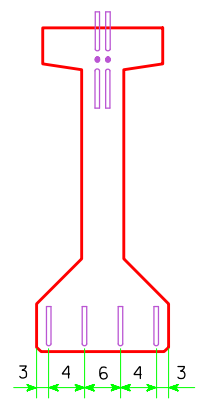
LIFTING LOOP DETAIL

ALTERNATE TYPES MAY BE SUBSTITUTED WITH THE APPROVAL OF THE ENGINEER. LIFTING LOOPS ARE TO BE STRUCTURAL GRADE.



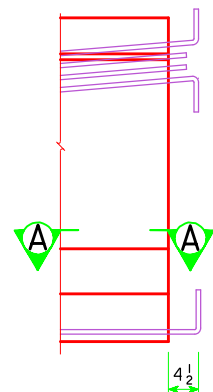
COIL TIE DETAIL

NUMBER AND EXACT LOCATION OF COIL TIES TO BE AS DETAILED ON SPECIFIC BRIDGE DESIGN.

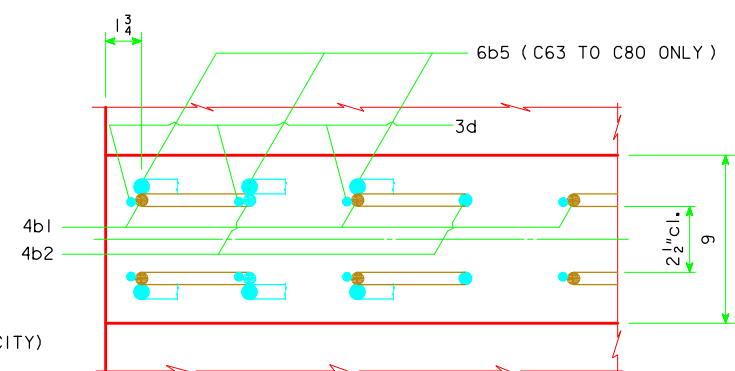


STRAND PROJECTION AT BEAM ENDS WHEN EMBEDDED IN CONCRETE END DIAPHRAGMS

THE TOP STRAIGHT STRANDS OF BEAMS C30 THROUGH C67 ARE TO BE CUT WITH 1'-0 PROJECTIONS AND SHOP BENT UP. THE TOP AND BOTTOM DEFLECTED STRANDS OF BEAMS C71 THROUGH C80 ARE TO BE CUT WITH 1'-0 PROJECTIONS AND SHOP BENT UP OR DOWN AS SHOWN. THE REMAINING TOP STRANDS ARE TO BE CUT WITH 0'-3 PROJECTIONS. FOUR BOTTOM STRANDS ARE TO BE CUT WITH 1'-0 PROJECTIONS AND SHOP BENT AS SHOWN. THE REMAINING BOTTOM STRANDS SHALL BE CUT OFF REASONABLY FLUSH WITH THE CONCRETE.



TYPICAL AT BOTH BEAM ENDS

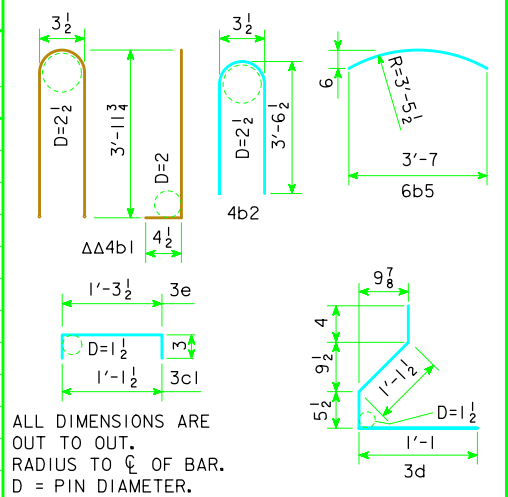


SECTION A-A SHOWING PLACEMENT OF STIRRUPS NEAR END OF BEAM

** WHERE DEFLECTING STRANDS INTERFERE WITH PLACEMENT, SOME IN-PLACE BENDING MAY BE NECESSARY.
 ΔΔ 4b1 BARS TO BE EPOXY COATED.

REINFORCING BAR LIST

| BEAM | SPAN | C80 |
|-----------|-------|--------|
| | 80'-0 | |
| BAR SHAPE | NO. | LENGTH |
| 6a1 | 4 | 41'-10 |
| 4a2 | 2 | 5'-0 |
| 8a3 | 2 | 40'-0 |
| ΔΔ 4b1 | 60 | 8'-10 |
| 4b2 | 12 | 7'-2 |
| 6b5 | 16 | 3'-9 |
| 3c1 | 60 | 1'-8 |
| ** 3d | 144 | 3'-0 |
| 3e | 26 | 1'-10 |



ALL DIMENSIONS ARE OUT TO OUT. RADIUS TO C OF BAR. D = PIN DIAMETER.

C BEAM DATA

| BEAM | SPAN LENGTH @ BEARING | OVERALL BEAM LENGTH (L) | STRAND SIZE DIA. (inches) | NO. OF STRANDS | | TOTAL INITIAL PRESTRESS KIPS | HOLD DOWN FORCE-KIPS | CAMBER (in.) | | DEFLECTION (in.) Δ _D | | WEIGHT (TONS) | CONCRETE (C. Y.) | REINFORCING STEEL-(lb) | | |
|------|-----------------------|-------------------------|---------------------------|----------------|-----------|------------------------------|----------------------|--------------|--------------|------------------------------------|--------------|---------------|------------------|------------------------|-------------------------------|--|
| | | | | STRAIGHT | DEFLECTED | | | AT RELEASE | AFTER LOSSES | IMMEDIATE (ELASTIC) Δ _E | | | | | TIME (PLASTIC) Δ _T | |
| | | | | | | | | | | STEEL DIAPH. | STEEL DIAPH. | | | | | |
| C80 | 80'-0 | 81'-0 | 0.60 | 16 | 6 | 936 | 21 | 1.64 | 2.90 | 1.16 | 0.29 | 23.8 | 11.76 | 1191 | | |

- ① DEFLECTIONS AT MID-SPAN DUE TO WEIGHT OF SLAB AND DIAPHRAGM.
- ② DEFLECTIONS DUE TO THE COMBINED EFFECT OF CREEP DUE TO WEIGHT OF SLAB AND SHRINKAGE OF SLAB.
 TOTAL BEAM DEFLECTIONS AT C OF SPAN, Δ_D, DUE TO WEIGHT OF SLAB AND DIAPHRAGMS FOR DETAILING PURPOSE:
 (A) Δ_D = Δ_E + Δ_T FOR SIMPLE SPAN.
- ③ TOTAL INITIAL PRESTRESS IS BASED ON 72.6% f'_s, f'_s = 270 ksi AND A_s = 0.217 sq. in.

DESIGN STRESSES:

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE TO BE IN ACCORDANCE WITH A.A.S.H.T.O. LRFD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2007:
 REINFORCING STEEL IN ACCORDANCE WITH SECTION 5, GRADE 60. CONCRETE IN ACCORDANCE WITH SECTION 5.
 MINIMUM CONCRETE f'_c (AT 28 DAYS) SHALL BE 6,000 psi. MINIMUM f'_{ci} AT RELEASE SHALL BE 5,000 psi.
 PRESTRESSING STEEL IN ACCORDANCE WITH SECTION 5, f'_s = 270,000 psi.

SPECIFICATIONS:

CONSTRUCTION: STANDARD SPECIFICATIONS OF THE IOWA DEPARTMENT OF TRANSPORTATION, CURRENT SERIES, WITH CURRENT APPLICABLE SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS.
 DESIGN: A.A.S.H.T.O. LRFD, SERIES OF 2007, WITH MINOR MODIFICATIONS.

BEAM NOTES:

THESE BEAMS ARE DESIGNED FOR AASHTO HL-93 LIVE LOADS AS WITH AN ALLOWANCE OF 20 LB. PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE.
 ALL PPC BEAMS SHALL USE HIGH PERFORMANCE CONCRETE (HPC) IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. HOLD DOWN POINTS FOR DEFLECTED STRANDS MAY BE MOVED TOWARD ENDS OF BEAM A DISTANCE OF 0.05 L MAXIMUM AT PRODUCER'S OPTION.
 ALL PRESTRESSING STRANDS SHALL CONFORM TO ASTM A416 GRADE 270 LOW RELAXATION STRANDS.
 TOPS OF BEAMS ARE TO BE STRUCK OFF LEVEL AND FINISHED AS PER MATERIALS IM570.
 BEARINGS SHALL BE AS DETAILED ON OTHER DESIGN SHEETS.
 BEAMS SHALL BE AT LEAST 28 DAYS OLD BEFORE THE SLAB IS PLACED EXCEPT AS OTHERWISE APPROVED BY THE ENGINEER.

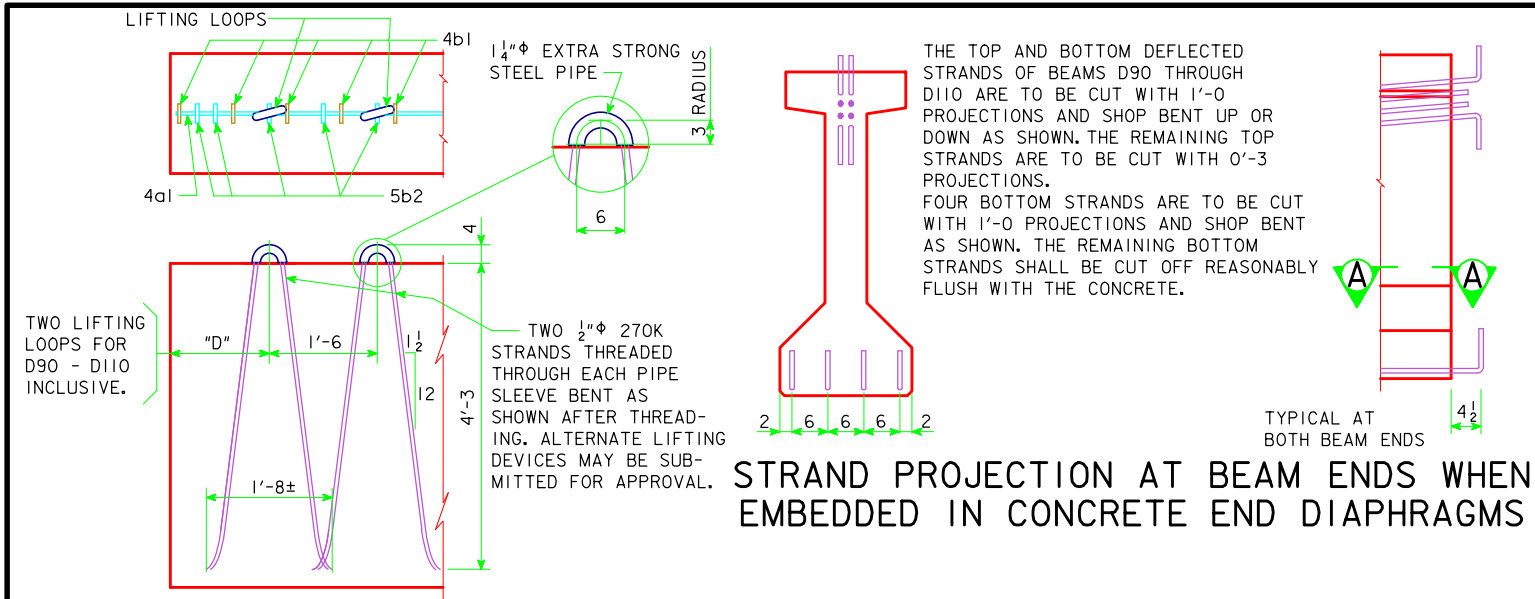
THE PORTIONS OF THE PRESTRESS BEAMS THAT ARE TO BE EMBEDDED IN THE ABUTMENT SHALL BE ROUGHENED FOR A DISTANCE OF 10" FROM THE BEAM END BY SANDBLASTING OR OTHER APPROVED METHODS TO PROVIDE SUITABLE BOND BETWEEN THE BEAM AND THE DIAPHRAGM IN ACCORDANCE WITH ARTICLE 2403.03, I, OF THE STANDARD SPECIFICATIONS.

ALL BEAMS ARE TO BE INCREASED IN LENGTH TO COMPENSATE FOR ELASTIC SHORTENING, CREEP AND SHRINKAGE.

HOLES MUST BE CAST IN THE WEB TO ACCOMMODATE THE STEEL DIAPHRAGM ATTACHMENTS AS DETAILED ON THE STEEL DIAPHRAGM DETAIL SHEET.

0.6" DIAMETER STRANDS STRESSED TO NOT MORE THAN 5,000 LBS. EACH MAY BE USED IN LIEU OF THE α BARS WHICH RUN THE FULL LENGTH OF THE BEAM IN THE TOP FLANGE.

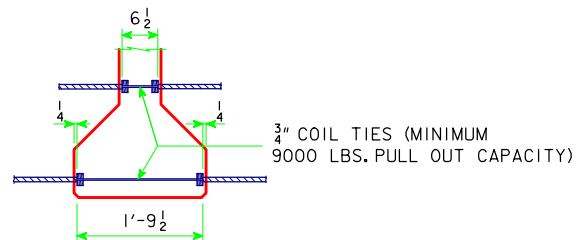
| | | | |
|-----------------------|---|--|--|
| LATEST REVISION DATE | <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | IOWA DOT Highway Division | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE | |
| | | PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| C BEAM DETAILS | | H30SI-25-12 | |



LIFTING LOOP DETAIL

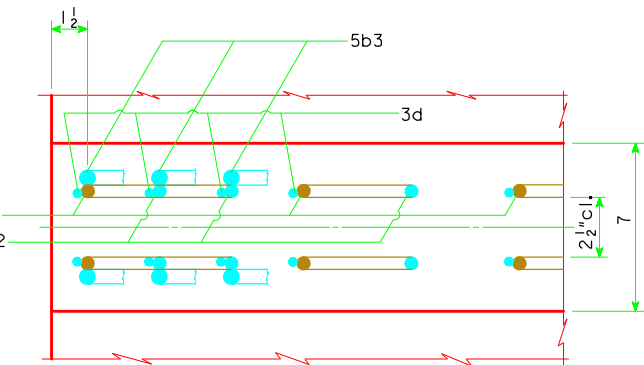
"D" = 1'-3 FOR D90
 "D" = 3'-9 FOR D100
 "D" = 8'-2 FOR D110

NUMBER AND EXACT LOCATION OF COIL TIES TO BE AS DETAILED ON SPECIFIC BRIDGE DESIGN.



COIL TIE DETAIL

STRAND PROJECTION AT BEAM ENDS WHEN EMBEDDED IN CONCRETE END DIAPHRAGMS



SECTION A-A SHOWING PLACEMENT OF STIRRUPS NEAR END OF BEAM

| BEAM | SPAN LENGTH ℓ - ℓ BEARING | OVERALL BEAM LENGTH (L) | STRAND SIZE DIA. (inches) | NO. OF STRANDS | | TOTAL INITIAL PRESTRESS KIPS $\text{\textcircled{C}}$ | HOLD DOWN FORCE-KIPS | CAMBER (in.) | | DEFLECTION (in.) Δ_D | | WEIGHT (TONS) | CONCRETE (C.Y.) | REINFORCING STEEL - (lbs.) |
|---------------|-------------------------------------|-------------------------|---------------------------|----------------|-----------|---|----------------------|--------------|--------------|--------------------------------|---------------------------|---------------|-----------------|----------------------------|
| | | | | STRAIGHT | DEFLECTED | | | AT RELEASE | AFTER LOSSES | IMMEDIATE Δ_i (ELASTIC) | TIME Δ_T (PLASTIC) | | | |
| | | | | | | | | | | | | | | |
| D90 | 90'-0 | 91'-0 | 0.60 | 16 | 6 | 936 | 25.8 | 1.40 | 2.46 | 0.93 | 0.23 | 30.4 | 15.0 | 1310 |
| *D100 | 100'-0 | 101'-0 | 0.60 | 22 | 6 | 1192 | 22.3 | 2.08 | 3.67 | 1.41 | 0.35 | 33.6 | 16.6 | 1521 |
| Δ D110 | 110'-0 | 111'-0 | 0.60 | 28 | 6 | 1446 | 21.2 | 2.83 | 4.83 | 1.69 | 0.42 | 36.9 | 18.2 | 1664 |

- $\text{\textcircled{1}}$ DEFLECTIONS AT MID-SPAN DUE TO WEIGHT OF SLAB AND DIAPHRAGM.
- $\text{\textcircled{2}}$ DEFLECTIONS DUE TO THE COMBINED EFFECT OF CREEP DUE TO WEIGHT OF SLAB AND SHRINKAGE OF SLAB.
- $\text{\textcircled{3}}$ TOTAL INITIAL PRESTRESS IS BASED ON 72.6% $f's$, $f's = 270$ ksi AND $A_s = 0.217$ sq. in.

BEAM NOTES:

THESE BEAMS ARE DESIGNED FOR AASHTO HL-93 LIVE LOADS AS WITH AN ALLOWANCE OF 20 LB. PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE.

ALL PPC BEAMS SHALL USE HIGH PERFORMANCE CONCRETE (HPC) IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. HOLD DOWN POINTS FOR DEFLECTED STRANDS MAY BE MOVED TOWARD ENDS OF BEAM A DISTANCE OF 0.05 L MAXIMUM AT PRODUCER'S OPTION.

ALL PRESTRESSING STRANDS SHALL CONFORM TO ASTM A416 GRADE 270 LOW RELAXATION STRANDS.

TOPS OF BEAMS ARE TO BE STRUCK OFF LEVEL AND FINISHED AS PER MATERIALS IM570.

BEARINGS SHALL BE AS DETAILED ON OTHER DESIGN SHEETS.

BEAMS SHALL BE AT LEAST 28 DAYS OLD BEFORE THE SLAB IS PLACED EXCEPT AS OTHERWISE APPROVED BY THE ENGINEER.

THE PORTIONS OF THE PRESTRESS BEAMS THAT ARE TO BE EMBEDDED IN THE ABUTMENT SHALL BE ROUGHENED FOR A DISTANCE OF 10" FROM THE BEAM END BY SANDBLASTING OR OTHER APPROVED METHODS TO PROVIDE SUITABLE BOND BETWEEN THE BEAM AND THE DIAPHRAGM IN ACCORDANCE WITH ARTICLE 2403.03, 1, OF THE STANDARD SPECIFICATIONS.

DESIGN STRESSES:

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE TO BE IN ACCORDANCE WITH A.A.S.H.T.O. LRFD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2007:

REINFORCING STEEL IN ACCORDANCE WITH SECTION 5, GRADE 60. CONCRETE IN ACCORDANCE WITH SECTION 5, $f'c = 5000$ psi (EXCEPT AS NOTED)

* MINIMUM CONCRETE $f'c$ (AT 28 DAYS) SHALL BE 7500 psi. MINIMUM $f'ci$ AT RELEASE SHALL BE 6000 psi.

Δ MINIMUM CONCRETE $f'c$ (AT 28 DAYS) SHALL BE 7500 psi. MINIMUM $f'ci$ AT RELEASE SHALL BE 6500 psi.

PRESTRESSING STEEL IN ACCORDANCE WITH SECTION 5, $f's = 270,000$ psi.

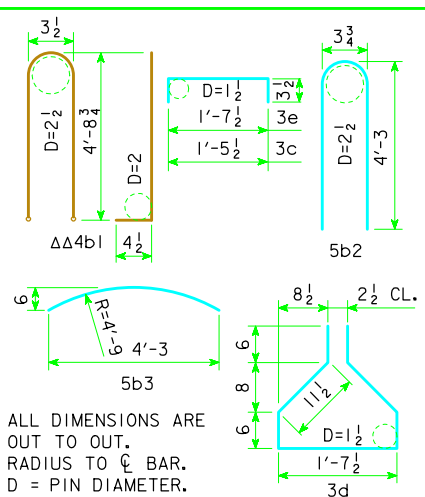
SPECIFICATIONS:

CONSTRUCTION: STANDARD SPECIFICATIONS OF THE IOWA DEPARTMENT OF TRANSPORTATION, CURRENT SERIES, WITH CURRENT APPLICABLE SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS.

DESIGN: A.A.S.H.T.O. LRFD, SERIES OF 2007, WITH MINOR MODIFICATIONS.

A=SIZE B=NO. $\Delta\Delta$ 4b1 BARS TO BE EPOXY COATED

| BEAM | | D90 | | D100 | | D110 | |
|--------------------|-----|--------|--------|-------|--------|-------|--------|
| SPAN | | NO. | LENGTH | NO. | LENGTH | NO. | LENGTH |
| 4a1 | 2 | 18'-0 | 2 | 22'-0 | 2 | 26'-6 | |
| a2 | 5/4 | 30'-10 | 6/4 | 35'-4 | 6/4 | 38'-4 | |
| a3 | 7/2 | 34'-0 | 8/2 | 36'-0 | 8/2 | 40'-0 | |
| $\Delta\Delta$ 4b1 | 74 | 10'-4 | 81 | 10'-4 | 91 | 10'-4 | |
| 5b2 | 16 | 8'-8 | 16 | 8'-8 | 16 | 8'-8 | |
| 5b3 | 20 | 4'-4 | 20 | 4'-4 | 20 | 4'-4 | |
| 3c | 74 | 2'-1 | 81 | 2'-1 | 91 | 2'-1 | |
| 3d | 90 | 5'-7 | 97 | 5'-7 | 107 | 5'-7 | |
| 3e | 30 | 2'-3 | 30 | 2'-3 | 30 | 2'-3 | |



STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE

PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES

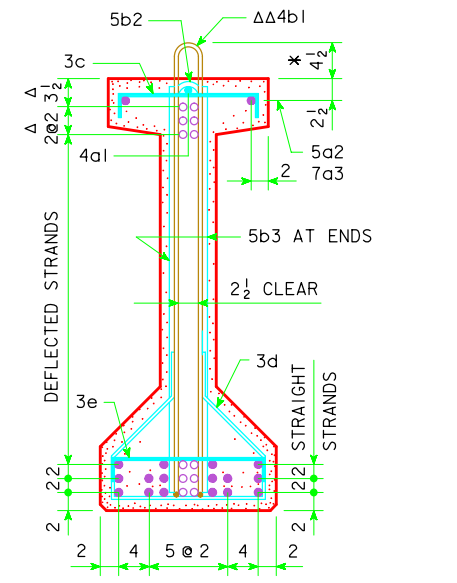
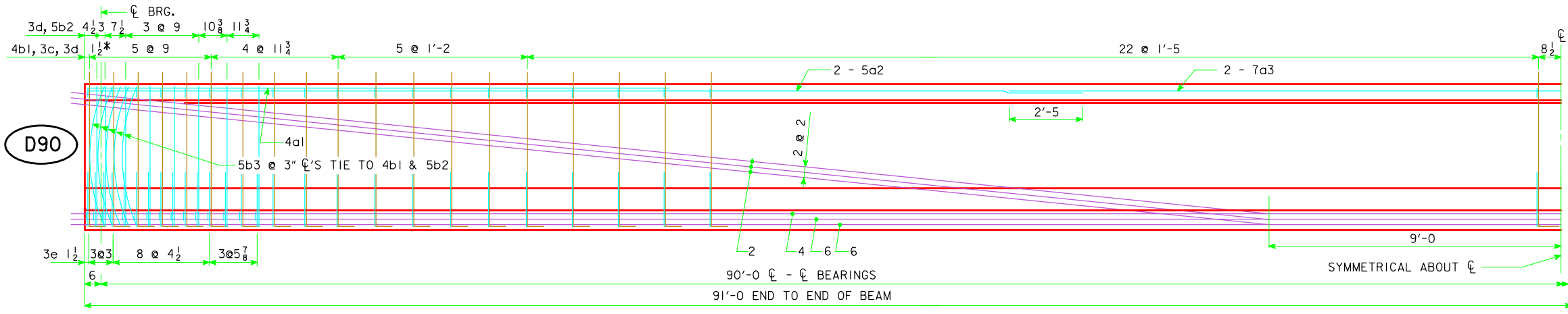
APRIL, 2012

LATEST REVISION DATE

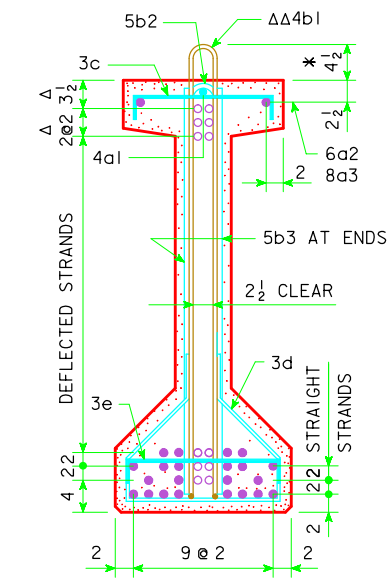
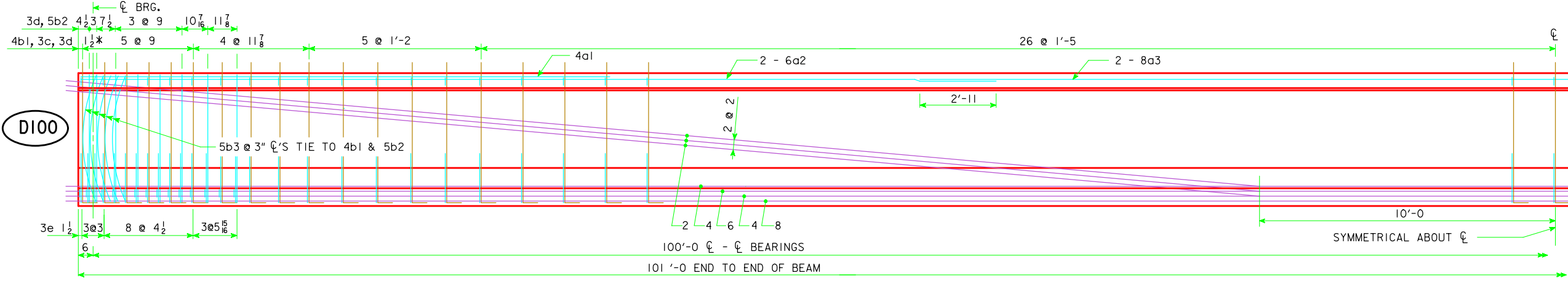
APPROVED BY BRIDGE ENGINEER

H30S1-27-12

NOTE: DIMENSIONS FOR THE LOCATION OF THE DEFLECTED STRANDS ARE AT \bar{C} BEAM AND END OF BEAM.

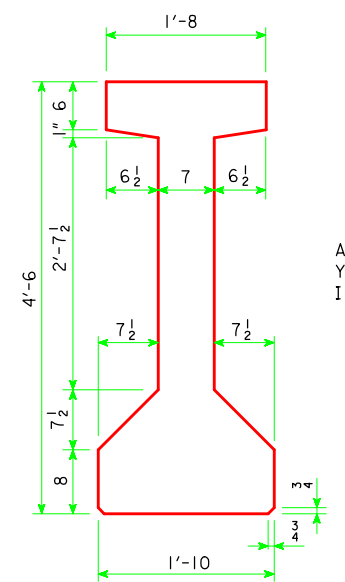


BEAM D90



BEAM D100

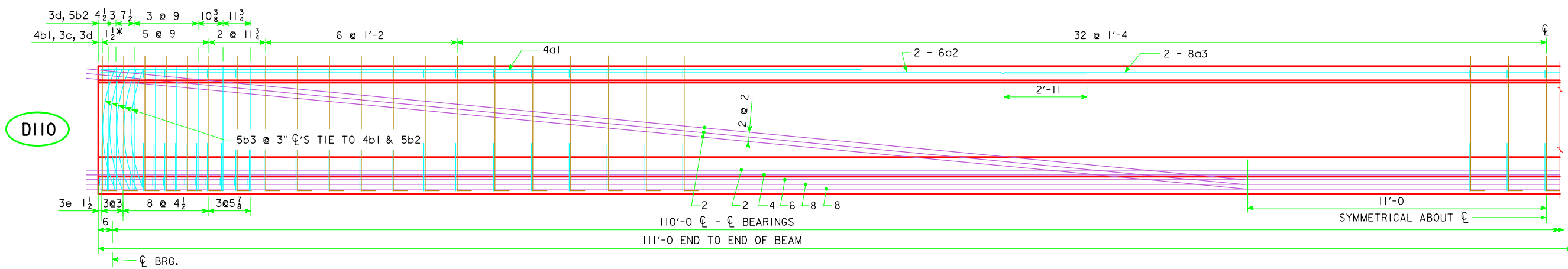
- DEFLECTED STRANDS
- * KEEP
- △ EPOXY COATED BARS
- △ DIMENSIONS AT END OF BEAM



AREA = 638.75 in²
 Y_b = 24.37 in
 I = 214,974 in⁴

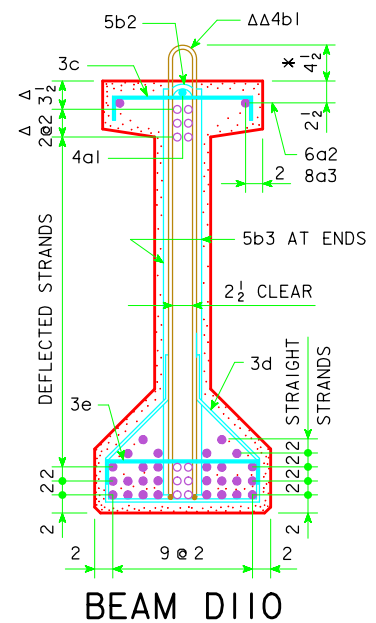
"D" BEAM CROSS SECTION

| | | | | |
|----------------------|---------------------------------|-----------------------|---|-------------|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | D90-D100 BEAM DETAILS | | H30SI-28-12 |
| | | | | |

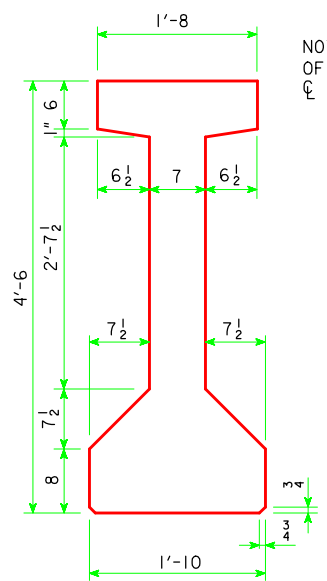


D110

ELEVATION VIEW



BEAM D110



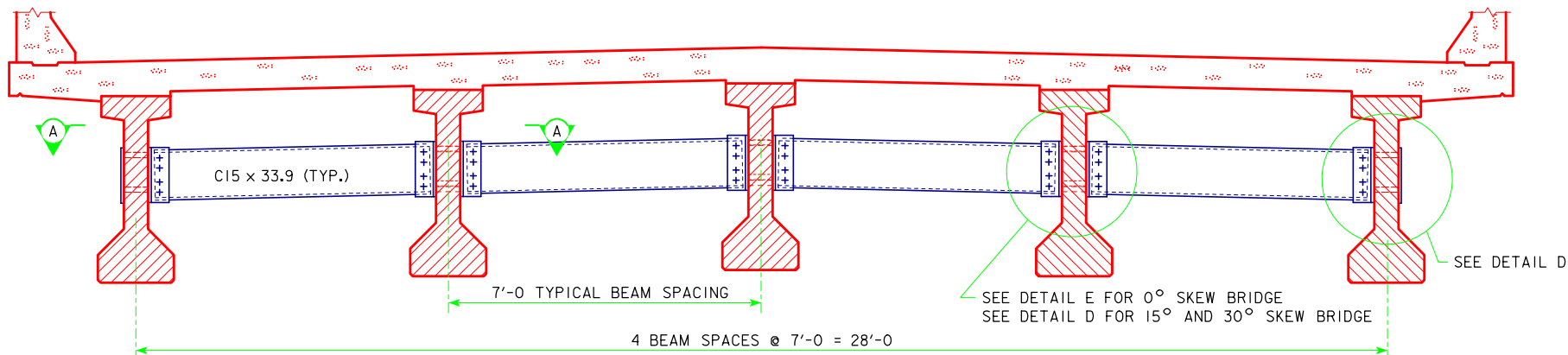
D110 CROSS SECTION

NOTE: DIMENSIONS FOR THE LOCATION OF THE DEFLECTED STRANDS ARE AT CL BEAM AND END OF BEAM.

- DEFLECTED STRANDS
- * KEEP
- △△ EPOXY COATED BARS
- △ DIMENSIONS AT END OF BEAM

AREA = 638.75 in²
 Y_b = 24.37 in
 I = 214,974 in⁴

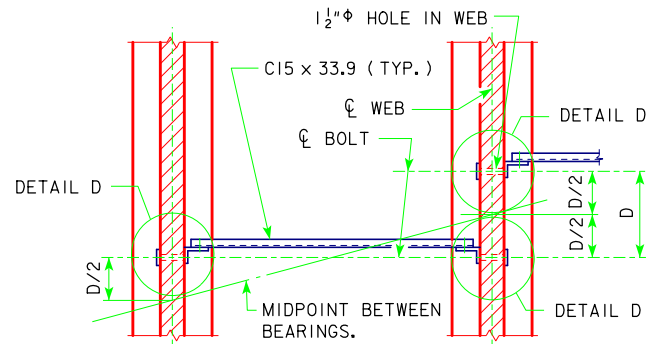
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|----------------------|---------------------------------|--|-------------|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | D110 BEAM DETAILS | H30S1-29-12 |



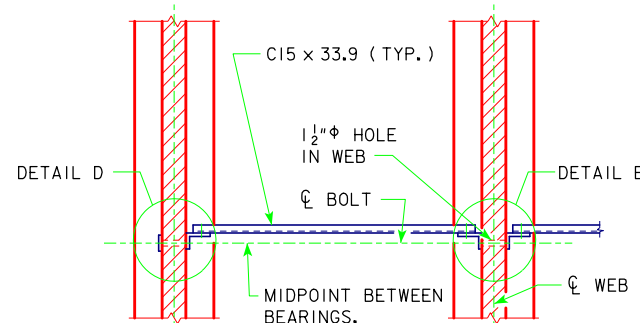
SECTION SHOWING INTERMEDIATE DIAPHRAGM

NOTES:

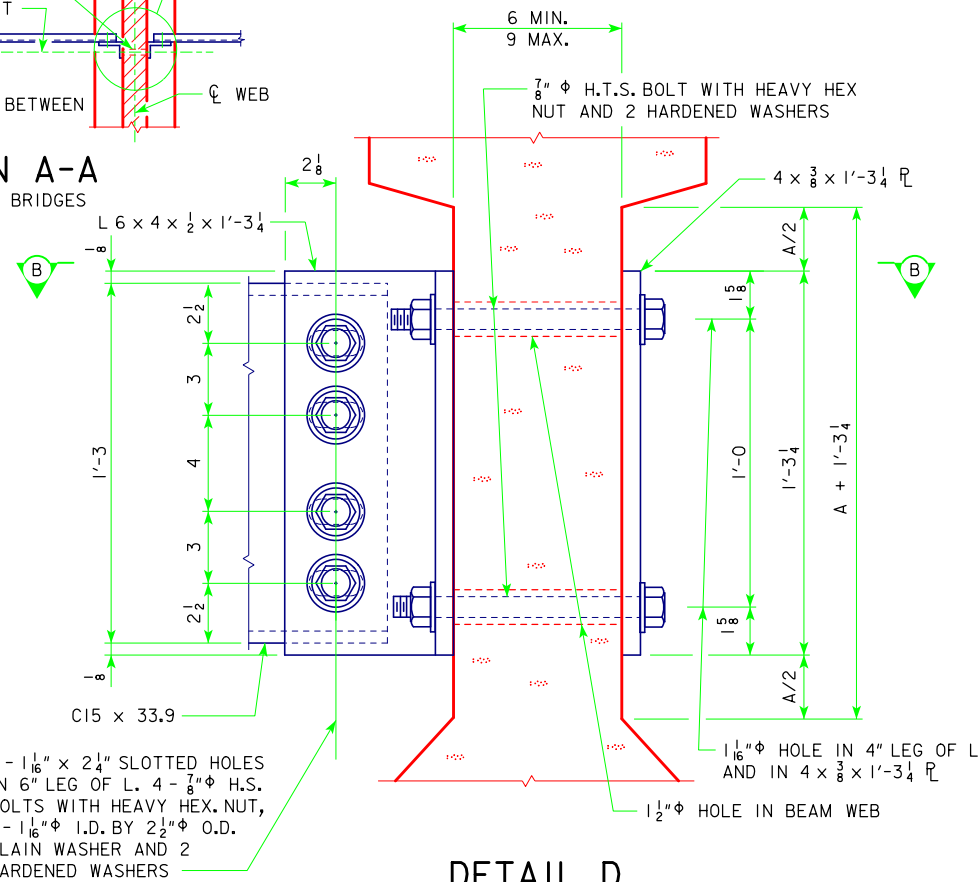
- ALL DIAPHRAGM MATERIALS, INCLUDING BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED.
- SHOP DRAWINGS OF THE STEEL DIAPHRAGMS SHOWING LAYOUT AND DETAILS OF THE DIAPHRAGMS SHALL BE SUBMITTED FOR APPROVAL.
- ALL COSTS FOR FURNISHING AND INSTALLING STEEL INTERMEDIATE DIAPHRAGMS SHALL BE INCLUDED IN THE PRICE BID FOR STRUCTURAL STEEL.
- THE 1 1/2" φ HOLES FOR THE 7/8" φ H.S. BOLTS SHALL BE CAST INTO THE WEB. DRILLING IS NOT ALLOWED.
- THE 7/8" φ H.S. BOLTS THROUGH THE WEB SHALL HAVE A THREAD LENGTH OF 3" MIN. AND 4" MAX. AND SHALL MEET THE REQUIREMENTS OF ASTM A449.
- ALL BOLTS ARE TO BE TIGHTENED PRIOR TO PLACING BRIDGE FLOOR CONCRETE.



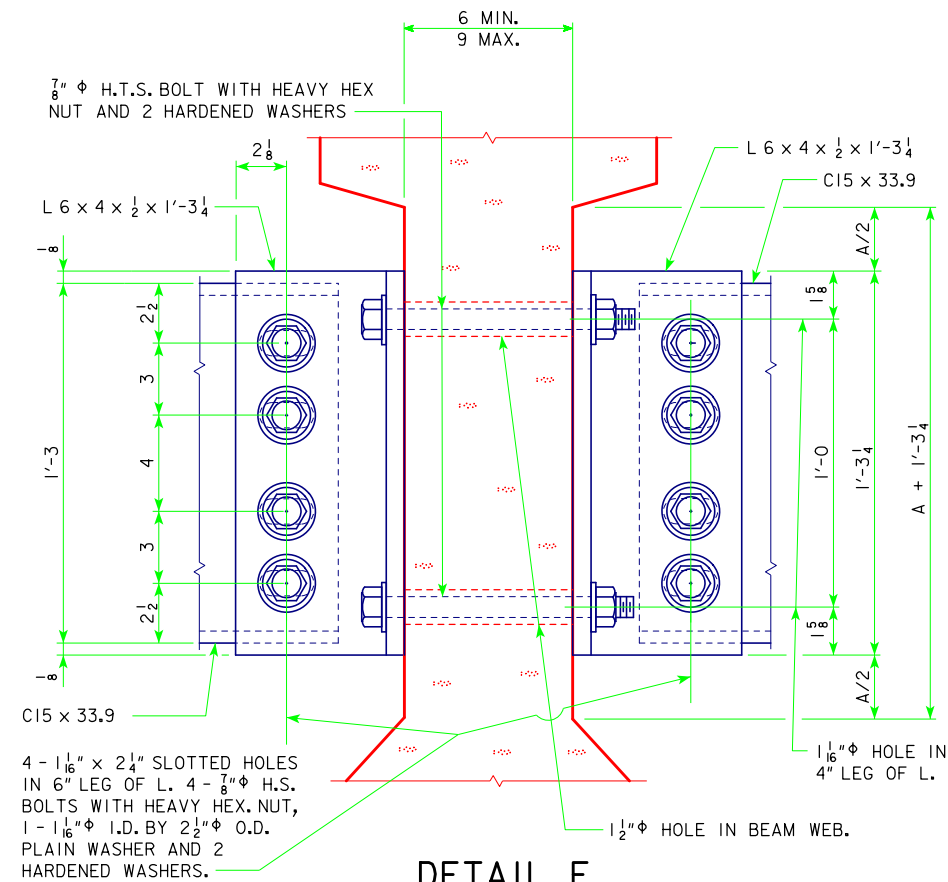
SECTION A-A FOR 15° AND 30° SKEW BRIDGES



SECTION A-A FOR 0° SKEW BRIDGES



DETAIL D



DETAIL E

INTERMEDIATE DIAPHRAGM STRUCTURAL STEEL

ONE CONNECTION DETAIL "E"

| 2 - 7/8" φ x LENGTH H.S. BOLTS WITH NUTS AND WASHERS | | |
|--|----------------------|-----------------------|
| WEB THICKNESS | LENGTH OF H.S. BOLTS | WEIGHT PER DETAIL "E" |
| 6" | 9" | 4.30 LBS. |
| 7" | 10" | 4.66 LBS. |
| 9" | 12" | 5.34 LBS. |
| 2 - L 6 x 4 x 1/2 x 1'-3 1/4 = 41.2 LBS. | | |

ONE CONNECTION DETAIL "D"

| 2 - 7/8" φ x LENGTH H.T.S. BOLTS WITH NUTS AND WASHERS | | |
|--|----------------------|-----------------------|
| WEB THICKNESS | LENGTH OF H.S. BOLTS | WEIGHT PER DETAIL "D" |
| 6" | 9" | 4.30 LBS. |
| 7" | 10" | 4.66 LBS. |
| 9" | 12" | 5.34 LBS. |
| 1 - BACKING L 4 x 3/8 x 1'-3 1/4 = 6.5 LBS. | | |
| 1 - L 6 x 4 x 1/2 x 1'-3 1/4 = 20.6 LBS. | | |

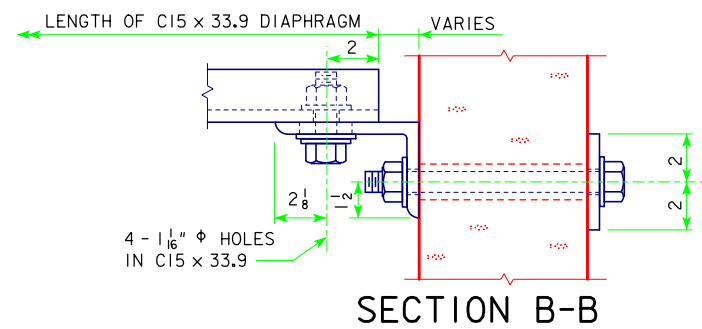
ONE C15 x 33.9 DIAPHRAGM

| WEB THICKNESS | 7'-0" | |
|---------------|--------------|--------------------|
| | BEAM SPACING | UNIT WEIGHT (LBS.) |
| 6" | 6'-2 5/8" | 210.8 |
| 7" | 6'-1 5/8" | 208.0 |
| 9" | 5'-11 5/8" | 202.4 |

DIAPHRAGM CONNECTION BOLTS

| | |
|---|-----------|
| 8 - 7/8" φ x 0'-2 3/4" H.S. BOLTS WITH NUTS AND WASHERS, PER UNIT DIAPHRAGM | 10.3 LBS. |
|---|-----------|

* THE LENGTH OF THE C15 x 33.9 SHOWN IN THE TABLE IS BASED ON A VARIABLE CLEARANCE OF 1/16" TO 2/16" BETWEEN THE FACE OF BEAM WEB AND END OF C15 x 33.9.



SECTION B-B

| BEAM SERIES | DIMENSIONS | |
|-------------|---------------|--------|
| | A + 1'-3 1/4" | A/2 |
| A | 1'-4" | 8 3/8" |
| B | 1'-8" | 2 3/8" |
| C | 2'-1" | 4 7/8" |
| D | 2'-7 1/2" | 8 1/8" |

LATEST REVISION DATE

Harmon L. Mc Donald

APPROVED BY BRIDGE ENGINEER

IOWADOT Highway Division

STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE

PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES

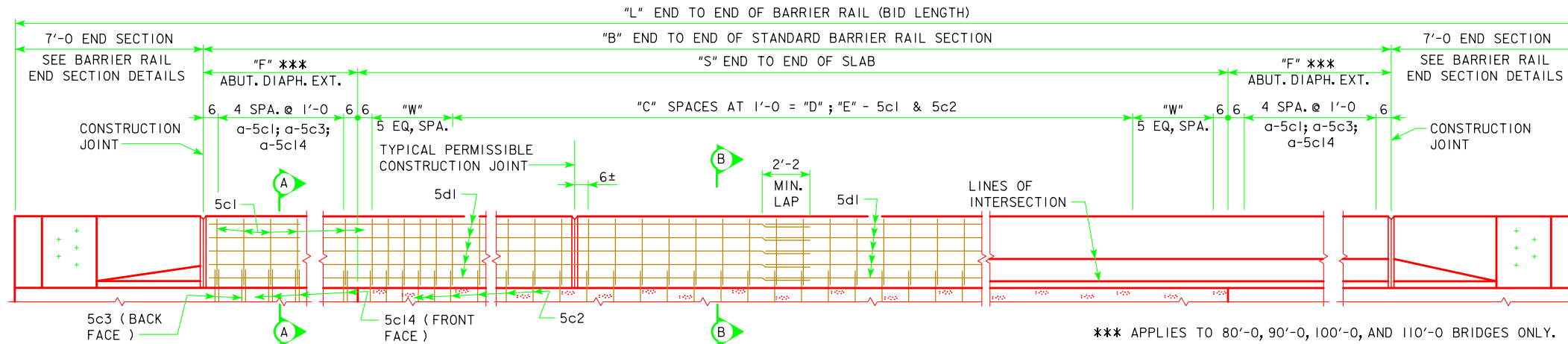
APRIL, 2012

INTERMEDIATE STEEL DIAPHRAGMS

H30SI-30-12

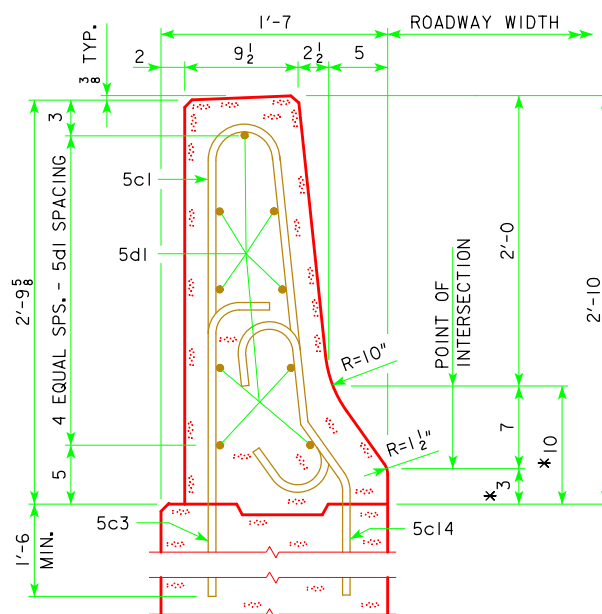
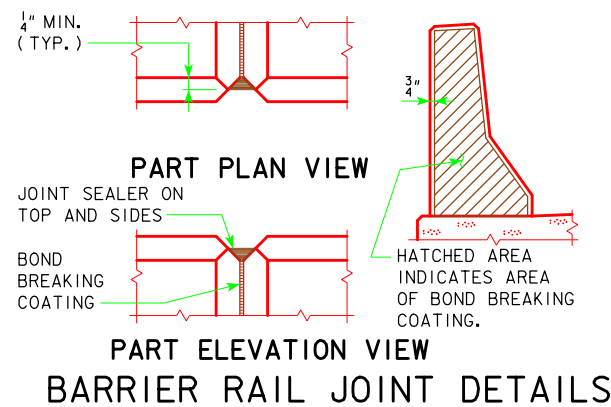
TABLE OF BARRIER RAIL DIMENSIONS AND NUMBERS

| CL-CL ABUT. BRG | 46'-8 | | | 55'-0 | | | 67'-6 | | | 80'-0 | | | 90'-0 | | | 100'-0 | | | 110'-0 | | | |
|---------------------|---------------|-----------------------------------|------------------------------------|------------------------------------|----------------------------------|------------------------------------|------------------------------------|----------------------------------|-------------------------------------|-------------------------------------|----------------------------------|------------------------------------|-------------------------------------|----------------------------------|------------------------------------|-------------------------------------|----------------------------------|------------------------------------|-------------------------------------|----------------------------------|------------------------------------|-------------------------------------|
| | SKEW | 0° | 15° | 30° | 0° | 15° | 30° | 0° | 15° | 30° | 0° | 15° | 30° | 0° | 15° | 30° | 0° | 15° | 30° | 0° | 15° | 30° |
| DIMENSION OR NUMBER | L (FT.-IN.) | 63'-8 | 63'-9 ¹ / ₄ | 64'-1 ⁹ / ₁₆ | 72'-0 | 72'-1 ¹ / ₄ | 72'-5 ⁹ / ₁₆ | 84'-6 | 84'-7 ¹ / ₄ | 84'-11 ⁹ / ₁₆ | 107'-0 | 107'-1 ¹ / ₄ | 107'-5 ⁹ / ₁₆ | 117'-0 | 117'-1 ¹ / ₄ | 117'-5 ⁹ / ₁₆ | 127'-0 | 127'-1 ¹ / ₄ | 127'-5 ⁹ / ₁₆ | 137'-0 | 137'-1 ¹ / ₄ | 137'-5 ⁹ / ₁₆ |
| | B (FT.-IN.) | 49'-8 | 49'-9 ¹ / ₄ | 50'-1 ⁹ / ₁₆ | 58'-0 | 58'-1 ¹ / ₄ | 58'-5 ⁹ / ₁₆ | 70'-6 | 70'-7 ¹ / ₄ | 70'-11 ⁹ / ₁₆ | 93'-0 | 93'-1 ¹ / ₄ | 93'-5 ⁹ / ₁₆ | 103'-0 | 103'-1 ¹ / ₄ | 103'-5 ⁹ / ₁₆ | 113'-0 | 113'-1 ¹ / ₄ | 113'-5 ⁹ / ₁₆ | 123'-0 | 123'-1 ¹ / ₄ | 123'-5 ⁹ / ₁₆ |
| | S (FT.-IN.) | 49'-8 | 49'-9 ¹ / ₄ | 50'-1 ⁹ / ₁₆ | 58'-0 | 58'-1 ¹ / ₄ | 58'-5 ⁹ / ₁₆ | 70'-6 | 70'-7 ¹ / ₄ | 70'-11 ⁹ / ₁₆ | 83'-0 | 83'-1 ¹ / ₄ | 83'-5 ⁹ / ₁₆ | 93'-0 | 93'-1 ¹ / ₄ | 93'-5 ⁹ / ₁₆ | 103'-0 | 103'-1 ¹ / ₄ | 103'-5 ⁹ / ₁₆ | 113'-0 | 113'-1 ¹ / ₄ | 113'-5 ⁹ / ₁₆ |
| | C | 39 | 39 | 39 | 47 | 47 | 47 | 60 | 60 | 60 | 72 | 72 | 72 | 82 | 82 | 82 | 92 | 92 | 92 | 102 | 102 | 102 |
| | D (FT.-IN.) | 39'-0 | 39'-0 | 39'-0 | 47'-0 | 47'-0 | 47'-0 | 60'-0 | 60'-0 | 60'-0 | 72'-0 | 72'-0 | 72'-0 | 82'-0 | 82'-0 | 82'-0 | 92'-0 | 92'-0 | 92'-0 | 102'-0 | 102'-0 | 102'-0 |
| | E | 40 | 40 | 40 | 48 | 48 | 48 | 61 | 61 | 61 | 73 | 73 | 73 | 83 | 83 | 83 | 93 | 93 | 93 | 103 | 103 | 103 |
| | F (FT.-IN.) | 0'-0 | 0'-0 | 0'-0 | 0'-0 | 0'-0 | 0'-0 | 0'-0 | 0'-0 | 0'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 |
| W (FT.-IN.) | 4'-10 | 4'-10 ⁵ / ₈ | 5'-0 ¹³ / ₁₆ | 5'-0 | 5'-0 ⁵ / ₈ | 5'-2 ¹³ / ₁₆ | 4'-9 | 4'-9 ⁵ / ₈ | 4'-11 ¹³ / ₁₆ | 5'-0 | 5'-0 ⁵ / ₈ | 5'-2 ¹³ / ₁₆ | 5'-0 | 5'-0 ⁵ / ₈ | 5'-2 ¹³ / ₁₆ | 5'-0 | 5'-0 ⁵ / ₈ | 5'-2 ¹³ / ₁₆ | 5'-0 | 5'-0 ⁵ / ₈ | 5'-2 ¹³ / ₁₆ | |
| a | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | 5 | |

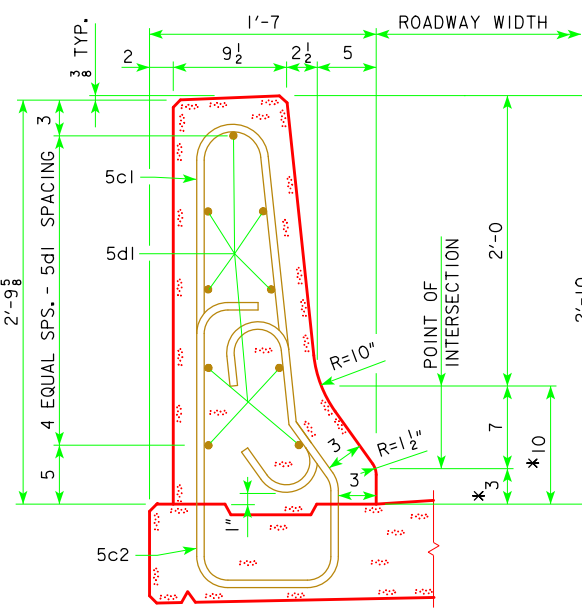


ELEVATION OF BARRIER RAIL LAYOUT

*** APPLIES TO 80'-0, 90'-0, 100'-0, AND 110'-0 BRIDGES ONLY.



PART SECTION A-A



PART SECTION B-B

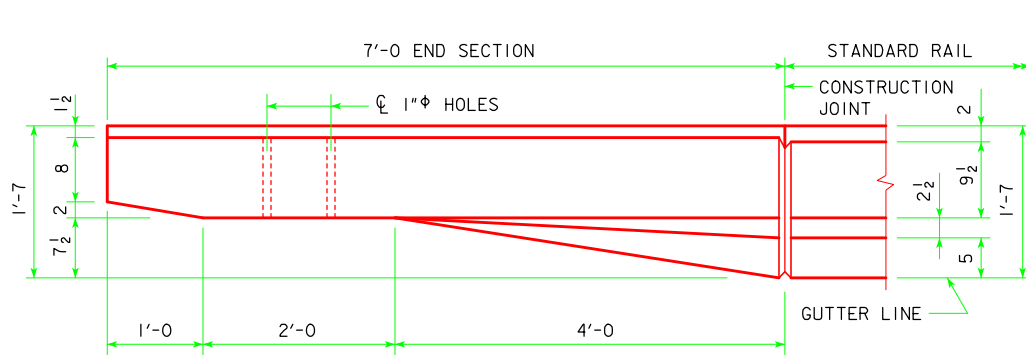
* DENOTES THE MAXIMUM VALUE FOR THIS DIMENSION. THIS DIMENSION MAY VARY DUE TO CONSTRUCTION INACCURACIES.

BARRIER RAIL NOTES:

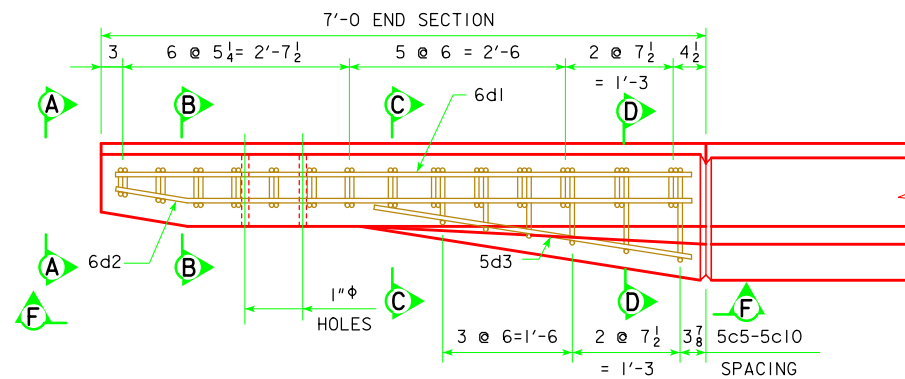
- MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.
- THE PERMISSIBLE CONSTRUCTION JOINTS ARE TO BE PLACED BETWEEN VERTICAL BARS AT A MINIMUM SPACING OF 20 FEET. CONSTRUCTION JOINT CONTACT SURFACES ARE TO BE COATED WITH AN APPROVED BOND BREAKER.
- COST OF THE JOINT SEALER AND BOND BREAKER SHALL BE CONSIDERED INCIDENTAL TO OTHER CONSTRUCTION.
- THE CONCRETE BARRIER RAIL IS TO BE BID ON A LINEAL FOOT BASIS. THE NUMBER OF LINEAL FEET OF BARRIER RAIL INSTALLED WILL BE PAID FOR AT THE CONTRACT PRICE PER LINEAL FOOT BASED ON PLAN QUANTITIES. PRICE BID FOR CONCRETE BARRIER RAILING SHALL BE FULL COMPENSATION FOR FURNISHING ALL MATERIAL, EXCLUDING REINFORCING STEEL, AND ALL OF THE EQUIPMENT AND LABOR REQUIRED TO ERECT THE RAIL IN ACCORDANCE WITH THESE PLANS AND CURRENT SPECIFICATIONS. IF CONDUIT IS REQUIRED IN THIS PLAN THE RIGID STEEL CONDUIT, JUNCTION BOXES AND FITTINGS INCLUDING LABOR AND ANY ADDITIONAL WORK TO DO THE INSTALLATION IS CONSIDERED INCIDENTAL TO THE COST OF THE RAILING.

- ALL BARRIER RAIL REINFORCING STEEL IS TO BE INCLUDED WITH THE SUPERSTRUCTURE REINFORCING STEEL.
- THE JOINT SEALER SHALL BE LIGHT GRAY NONSAG LATEX CAULKING SEALER MARKETED FOR OUTDOOR USE. NO TESTING OR CERTIFICATION IS REQUIRED.
- TOP OF THE BARRIER RAIL IS TO BE PARALLEL TO THE THEORETICAL CL GRADE.
- CROSS SECTIONAL AREA OF THE STANDARD SECTION OF THE BARRIER RAIL = 2.84 SQUARE FEET.
- CONCRETE BARRIER RAILS PLACED USING THE SLIPFORM METHOD WILL REQUIRE THE USE OF A CLASS BR CONCRETE IN ACCORDANCE WITH ARTICLE 2513.03B OF THE STANDARD SPECIFICATION. CLASS D CONCRETE IS NOT PERMITTED FOR CONCRETE BARRIER RAILS PLACED USING THE SLIPFORM METHOD.

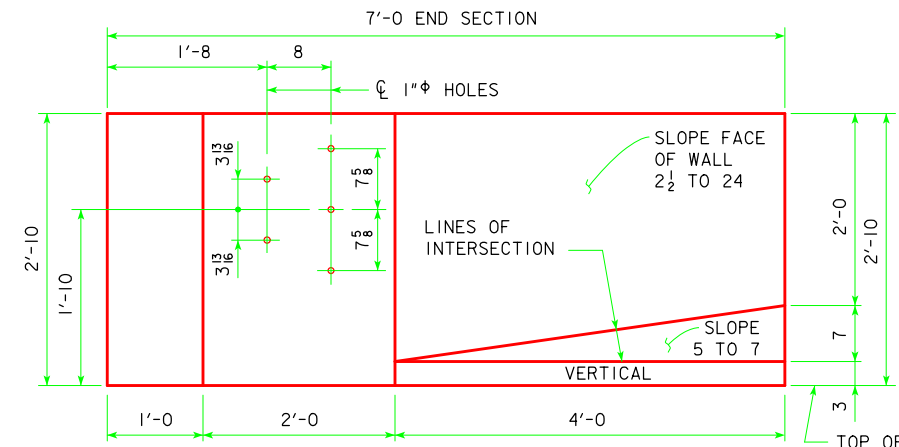
| | | | |
|----------------------|---------------------------------|---|--------------------|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | BARRIER RAIL DETAILS SHEET 1 OF 3 | H30S1-31-12 |



PART PLAN VIEW

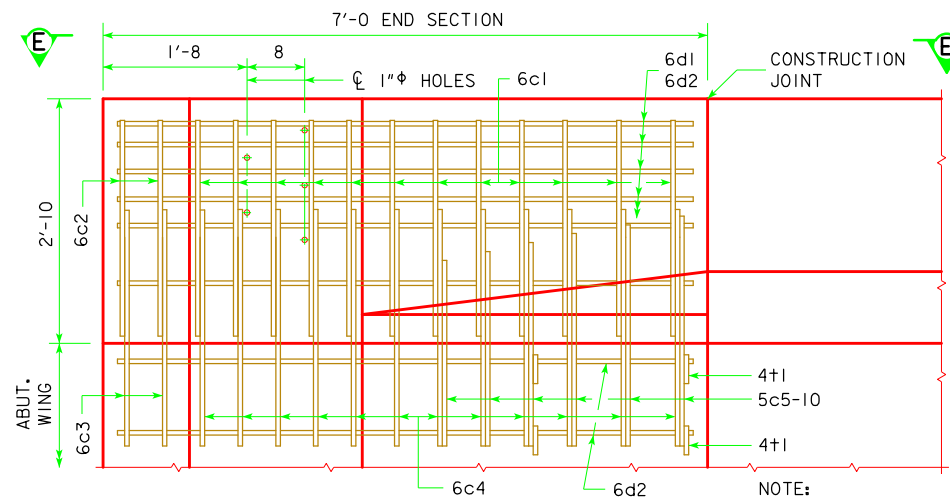


PART VIEW E-E



PART ELEVATION VIEW

PROVIDE 5 HOLES FORMED WITH 1"ϕ PLASTIC CONDUIT. COST TO BE INCLUDED IN PRICE BID FOR CONCRETE BARRIER RAILING.



PART VIEW F-F

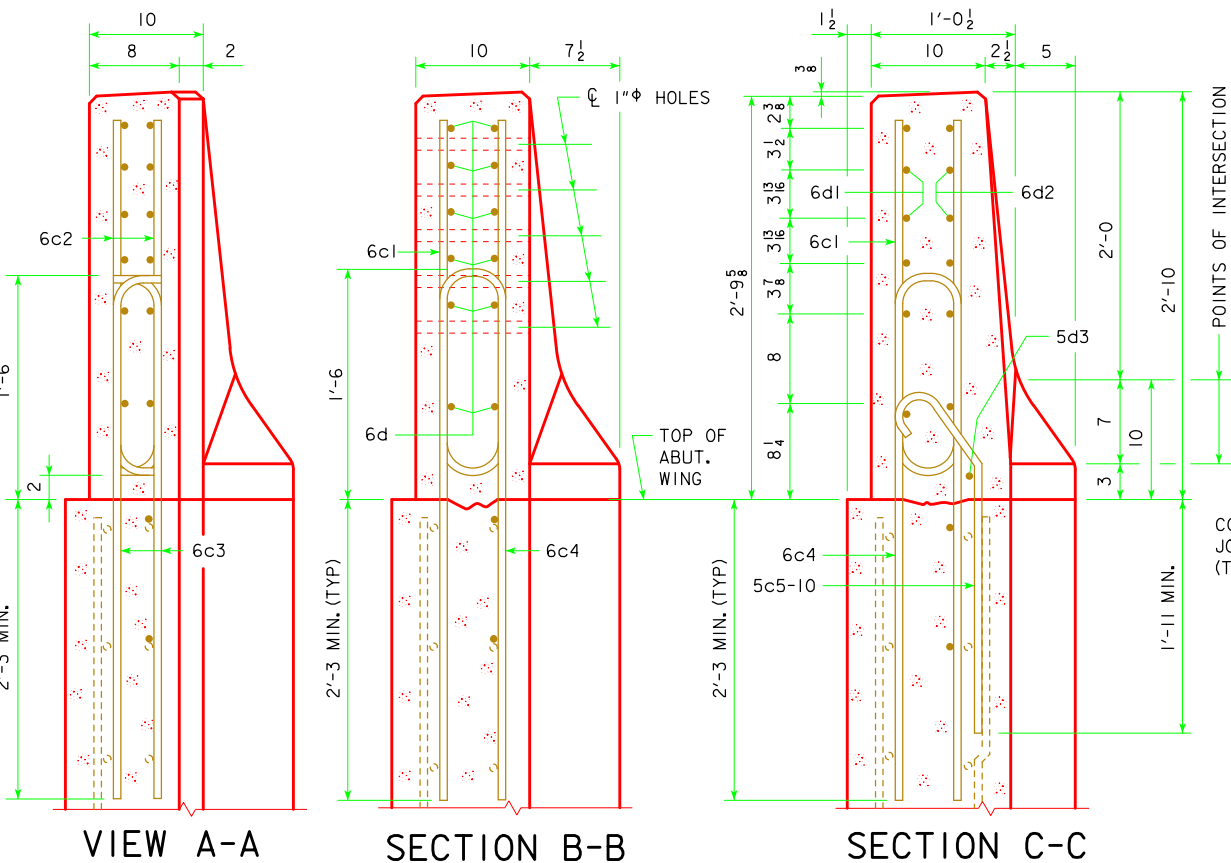
NOTE: 4+1 PLACEMENT- 2 BARS EACH LEVEL OF 6d2 IN WING FOOTING.

NOTE: CONSTRUCTION JOINT BETWEEN TOP OF WING AND BARRIER RAIL IS ROUGHENED CONCRETE.

NOTE: THE 10" RADIUS AND 1 1/2" RADIUS ARE TYPICAL AND SHALL BE USED WHEN CONSTRUCTING THE CORNERS FOR VIEW A-A, SECTION B-B, SECTION C-C AND SECTION D-D.

NOTE: THE 6c4, 6c3, 5c5-10, 2 - 6d2 AND 4+1 BARS ARE TO BE PLACED WITH THE ABUTMENT WING. THE DETAILS FOR PLACEMENT ARE SHOWN ON THE WING ABUTMENT SHEET.

NOTE: DASHED LINES BELOW THE TOP OF WING ARE THE ABUTMENT WING REINFORCING STEEL. SEE WING ABUTMENT SHEET FOR PLACEMENT.



VIEW A-A

SECTION B-B

SECTION C-C

SECTION D-D

REINFORCING STEEL - ONE END SECTION

| BAR | LOCATION | SHAPE | NO. | LENGTH | WEIGHT |
|--------|------------------------|-------|-----|---------------------|--------|
| 6c1 | VERTICAL | ┌ | 12 | 5'-6 | 99 |
| 6c2 | VERTICAL | ┌ | 4 | 2'-10 | 17 |
| 6c3 | VERTICAL | ┌ | 4 | 4'-1 | 25 |
| 6c4 | VERTICAL | ┌ | 12 | 8'-0 | 144 |
| 5c5-10 | VERTICAL | ┌ | 6 | VARIES | 23 |
| 6d1 | HORIZONTAL | — | 6 | 6'-8 | 60 |
| 6d2 | HORIZONTAL | — | 8 | 6'-9 | 81 |
| 5d3 | HORIZONTAL | — | 1 | 3'-9 | 4 |
| 4+1 | ABUTMENT WING TIE BARS | ┌ | 4 | VARIES | 5 |
| | | | | TOTAL WEIGHT (LBS.) | 458 |

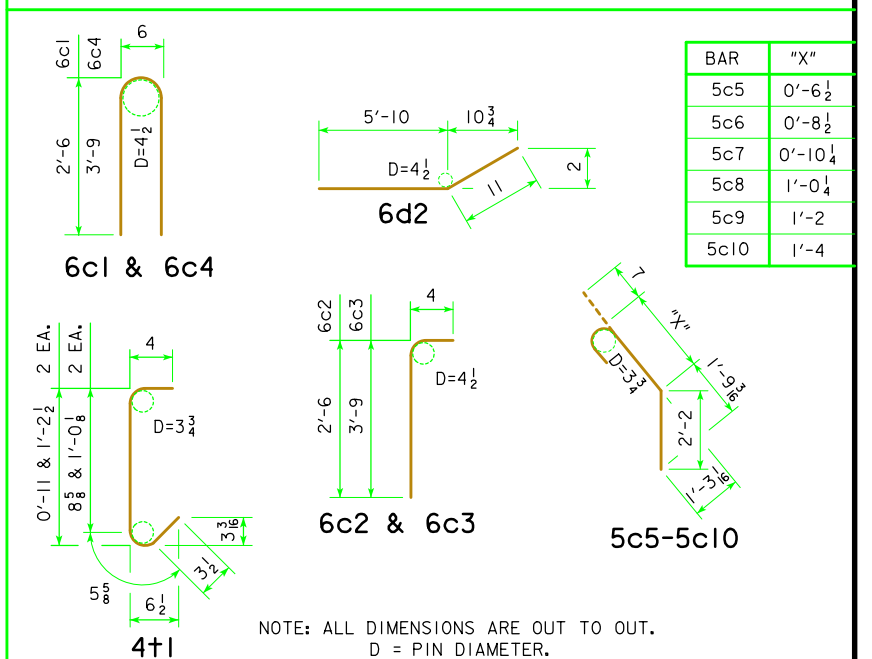
(INCLUDE WITH BARRIER RAIL REINFORCING)

TOTAL WEIGHT (LBS.) 458

CONCRETE PLACEMENT SUMMARY

| SECTION | TOTAL |
|------------------------------|--------------|
| BARRIER RAIL ONE END SECTION | 0.65 CU. YD. |

BENT BAR DETAILS



NOTE: ALL DIMENSIONS ARE OUT TO OUT. D = PIN DIAMETER.

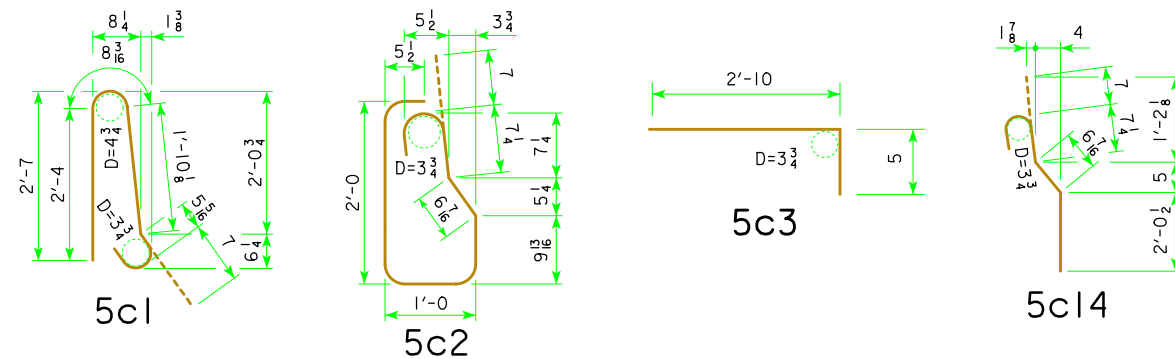
| | | | |
|----------------------|---------------------------------|---|--|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | BARRIER RAIL DETAILS | |
| | | H20SI-32-12 SHEET 2 OF 3 | |

REINFORCING STEEL-TWO BARRIER RAILS

(NOTE: THESE REINFORCING BARS TO BE USED ON ALL SKEWS)

| BRIDGE LENGTH | | | | 46'-8 | | | 55'-0 | | | 67'-6 | | | 80'-0 | | | 90'-0 | | | 100'-0 | | | 110'-0 | | | | | | | | | | | | | | | | | | | | |
|--|------|--------------|-------|-------|--------|--------|-------|--------|--------|-------|--------|--------|-------|--------|--------|-------|--------|--------|--------|--------|--------|--------|--------|--------|------|--|--|--|--|--|------|--|--|--|--|--|------|--|--|--|--|--|
| SECTION | BAR | LOCATION | SHAPE | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | | | | | | | | | | | | | | | | | | |
| STANDARD SECTION | 5c1 | VERTICAL | | 100 | 5'-11 | 617 | 116 | 5'-11 | 716 | 142 | 5'-11 | 876 | 186 | 5'-11 | 1148 | 206 | 5'-11 | 1271 | 226 | 5'-11 | 1395 | 246 | 5'-11 | 1518 | | | | | | | | | | | | | | | | | | |
| | 5c2 | VERTICAL | | 100 | 6'-0 | 626 | 116 | 6'-0 | 726 | 142 | 6'-0 | 889 | 166 | 6'-0 | 1039 | 186 | 6'-0 | 1164 | 206 | 6'-0 | 1289 | 226 | 6'-0 | 1414 | | | | | | | | | | | | | | | | | | |
| | 5c3 | VERTICAL | | - | - | - | - | - | - | - | - | - | 20 | 3'-3 | 65 | 20 | 3'-3 | 65 | 20 | 3'-3 | 65 | 20 | 3'-3 | 65 | | | | | | | | | | | | | | | | | | |
| | 5c14 | VERTICAL | | - | - | - | - | - | - | - | - | - | 20 | 3'-10 | 80 | 20 | 3'-10 | 80 | 20 | 3'-10 | 80 | 20 | 3'-10 | 80 | | | | | | | | | | | | | | | | | | |
| | 5d1 | LONGITUDINAL | | 36 | 26'-1 | 979 | 36 | 30'-3 | 1136 | 36 | 36'-6 | 1371 | 54 | 32'-7 | 1835 | 54 | 35'-11 | 2023 | 54 | 39'-3 | 2211 | 72 | 32'-6 | 2441 | | | | | | | | | | | | | | | | | | |
| 4 END SECTIONS @ 458 LBS. | | | | | | 1832 | 1832 | | | | | | 1832 | | | | | | 1832 | | | | | | | | | | | | | | | | | | | | | | | |
| TOTAL LBS. (INCLUDE WITH SUPERSTRUCTURE REINFORCING) | | | | | | 4054 | 4410 | | | | | | 4968 | | | | | | 6002 | | | | | | 6438 | | | | | | 6875 | | | | | | 7353 | | | | | |

BENT BAR DETAILS



NOTE:
ALL DIMENSIONS ARE OUT TO OUT.
D = PIN DIAMETER.

CONCRETE PLACEMENT SUMMARY

| BRIDGE LENGTH | | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 |
|--------------------|-------------------------------|-------|-------|-------|-------|-------|--------|--------|
| STANDARD SECTION * | 2 X B @ 0.1052 CU.YD. PER FT. | 10.5 | 12.3 | 14.9 | 19.7 | 21.8 | 23.9 | 26.0 |
| END SECTION | 4 @ 0.65 CU.YD. | 2.6 | 2.6 | 2.6 | 2.6 | 2.6 | 2.6 | 2.6 |
| TOTAL (CU. YD.) | | 13.1 | 14.9 | 17.5 | 22.3 | 24.4 | 26.5 | 28.6 |

* SEE SHEET H30SI-34-12 FOR DIMENSION "B".
CONCRETE QUANTITIES SHOWN ARE BASED
ON 30° SKEW BID LENGTHS.

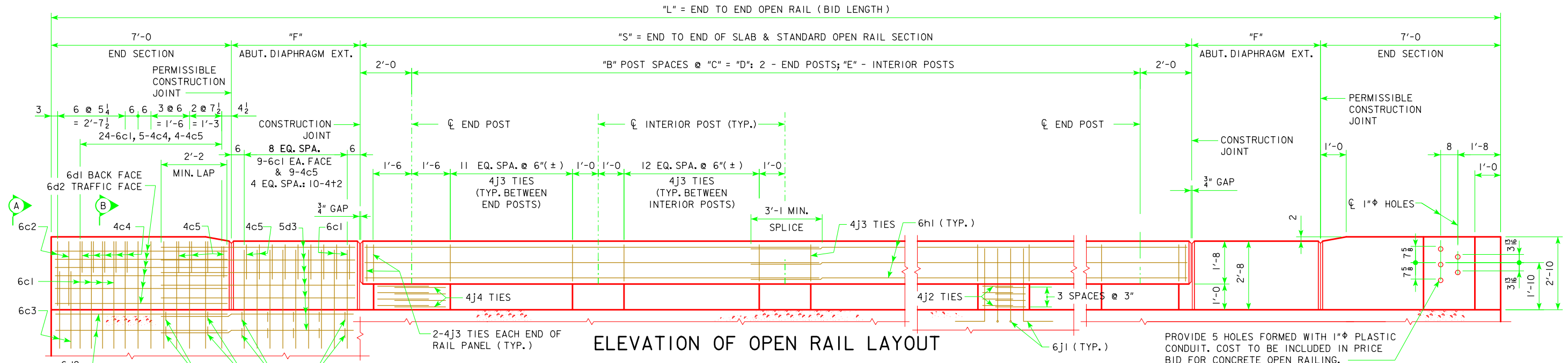
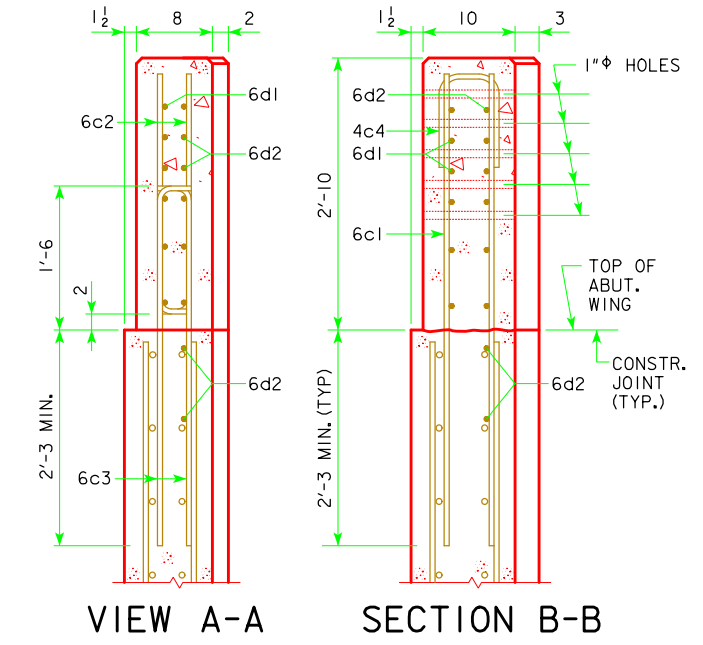
CONCRETE BARRIER RAIL QUANTITIES

| BRIDGE LENGTH | | UNIT | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 |
|--------------------------|----------|------|-------|-------|-------|-------|-------|--------|--------|
| CONCRETE BARRIER RAILING | 0° SKEW | L.F. | 127.3 | 144.0 | 169.0 | 214.0 | 234.0 | 254.0 | 274.0 |
| CONCRETE BARRIER RAILING | 15° SKEW | L.F. | 127.5 | 144.2 | 169.2 | 214.2 | 234.2 | 254.2 | 274.2 |
| CONCRETE BARRIER RAILING | 30° SKEW | L.F. | 128.3 | 144.9 | 169.9 | 214.9 | 234.9 | 254.9 | 274.9 |

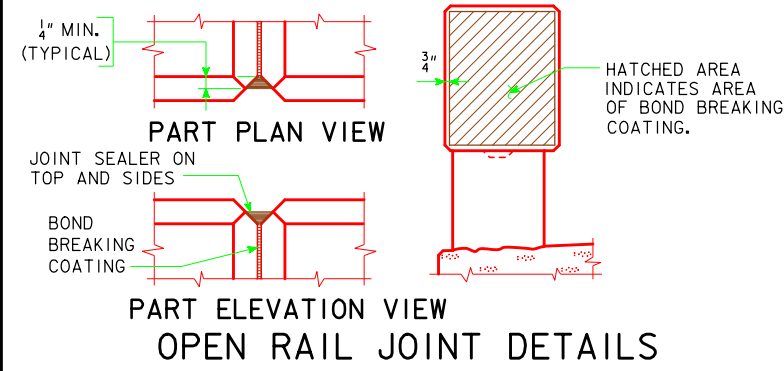
| | | |
|----------------------|---------------------------------|---|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 |
| | | BARRIER RAIL DETAILS H30SI-33-12 SHEET 3 OF 3 |

TABLE OF OPEN RAIL DIMENSIONS AND NUMBERS

| DIMENSION OR NUMBER | 46'-8 | | | 55'-0 | | | 67'-6 | | | 80'-0 | | | 90'-0 | | | 100'-0 | | | 110'-0 | | | DIMENSION OR NUMBER |
|---------------------|----------------------------------|-----------------------------------|-----------------------------------|----------------------------------|-----------------------------------|-----------------------------------|----------------------------------|-----------------------------------|------------------------------------|-----------------------------------|------------------------------------|------------------------------------|--------|------------------------------------|------------------------------------|----------------------------------|------------------------------------|------------------------------------|----------------------------------|------------------------------------|------------------------------------|---------------------|
| | 0° | 15° | 30° | 0° | 15° | 30° | 0° | 15° | 30° | 0° | 15° | 30° | 0° | 15° | 30° | 0° | 15° | 30° | 0° | 15° | 30° | |
| L (FT.-IN.) | 63'-8 | 63'-9 ¹ / ₄ | 64'-1 ⁵ / ₈ | 72'-0 | 72'-1 ¹ / ₄ | 72'-5 ⁵ / ₈ | 84'-6 | 84'-7 ¹ / ₄ | 84'-11 ⁵ / ₈ | 107'-0 | 107'-1 ¹ / ₄ | 107'-5 ⁵ / ₈ | 117'-0 | 117'-1 ¹ / ₄ | 117'-5 ⁵ / ₈ | 127'-0 | 127'-1 ¹ / ₄ | 127'-5 ⁵ / ₈ | 137'-0 | 137'-1 ¹ / ₄ | 137'-5 ⁵ / ₈ | L (FT.-IN.) |
| S (FT.-IN.) | 49'-8 | 49'-9 ¹ / ₄ | 50'-1 ⁵ / ₈ | 58'-0 | 58'-1 ¹ / ₄ | 58'-5 ⁵ / ₈ | 70'-6 | 70'-7 ¹ / ₄ | 70'-11 ⁵ / ₈ | 83'-0 | 83'-1 ¹ / ₄ | 83'-5 ⁵ / ₈ | 93'-0 | 93'-1 ¹ / ₄ | 93'-5 ⁵ / ₈ | 103'-0 | 103'-1 ¹ / ₄ | 103'-5 ⁵ / ₈ | 113'-0 | 113'-1 ¹ / ₄ | 113'-5 ⁵ / ₈ | S (FT.-IN.) |
| B | 6 | 6 | 6 | 7 | 7 | 7 | 9 | 9 | 9 | 10 | 10 | 10 | 12 | 12 | 12 | 13 | 13 | 13 | 14 | 14 | 14 | B |
| C (FT.-IN.) | 7'-7 ³ / ₈ | 7'-7 ¹ / ₂ | 7'-8 ¹ / ₄ | 7'-8 ⁵ / ₈ | 7'-8 ³ / ₄ | 7'-9 ³ / ₈ | 7'-4 ⁵ / ₈ | 7'-4 ³ / ₄ | 7'-5 ¹ / ₄ | 7'-10 ³ / ₄ | 7'-10 ¹ / ₈ | 7'-11 ³ / ₈ | 7'-5 | 7'-5 ¹ / ₈ | 7'-5 ¹ / ₂ | 7'-7 ³ / ₈ | 7'-7 ¹ / ₂ | 7'-7 ³ / ₄ | 7'-9 ³ / ₈ | 7'-9 ¹ / ₂ | 7'-9 ⁷ / ₈ | C (FT.-IN.) |
| D (FT.-IN.) | 45'-8 | 45'-9 ¹ / ₄ | 46'-1 ⁵ / ₈ | 54'-0 | 54'-1 ¹ / ₄ | 54'-5 ⁵ / ₈ | 66'-6 | 66'-7 ¹ / ₄ | 66'-11 ⁵ / ₈ | 79'-0 | 79'-1 ¹ / ₄ | 79'-5 ⁵ / ₈ | 89'-0 | 89'-1 ¹ / ₄ | 89'-5 ⁵ / ₈ | 99'-0 | 99'-1 ¹ / ₄ | 99'-5 ⁵ / ₈ | 109'-0 | 109'-1 ¹ / ₄ | 109'-5 ⁵ / ₈ | D (FT.-IN.) |
| E | 5 | 5 | 5 | 6 | 6 | 6 | 8 | 8 | 8 | 9 | 9 | 9 | 11 | 11 | 11 | 12 | 12 | 12 | 13 | 13 | 13 | E |
| F (FT.-IN.) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | 5'-0 | F (FT.-IN.) |



OPEN RAIL NOTES:
 CONSTRUCTION JOINT BETWEEN TOP OF WING AND RAIL IS ROUGHENED CONCRETE.
 MINIMUM CLEAR DISTANCE FROM FACE OF CONCRETE TO NEAR REINFORCING BAR IS TO BE 2" UNLESS OTHERWISE NOTED OR SHOWN.
 COST OF THE JOINT SEALER AND BOND BREAKER SHALL BE CONSIDERED INCIDENTAL TO OTHER CONSTRUCTION.
 ALL OPEN RAIL REINFORCING STEEL IS TO BE INCLUDED WITH THE SUPERSTRUCTURE REINFORCING STEEL.
 THE CAST-IN-PLACE OPEN RAIL SHALL USE CLASS C MIX. CLASS D CONCRETE IS NOT PERMITTED.
 TOP OF THE OPEN RAIL IS TO BE PARALLEL TO THEORETICAL ϕ GRADE.



| | | | |
|----------------------|---|---|------------------------------------|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER <i>Thomas L. Mc Donald</i> | | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | OPEN RAIL, TL-4 DETAILS | H30SI-34-12 SHEET 1 OF 2 |

REINFORCING STEEL-TWO OPEN RAILS

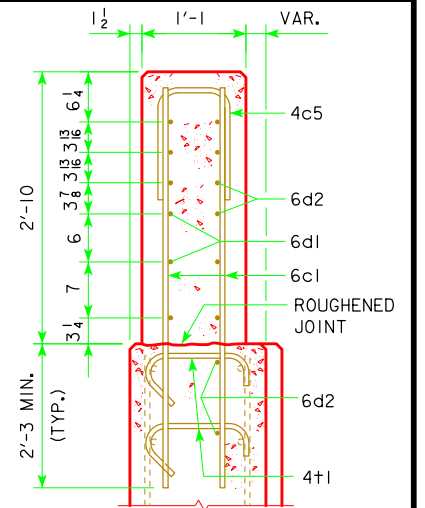
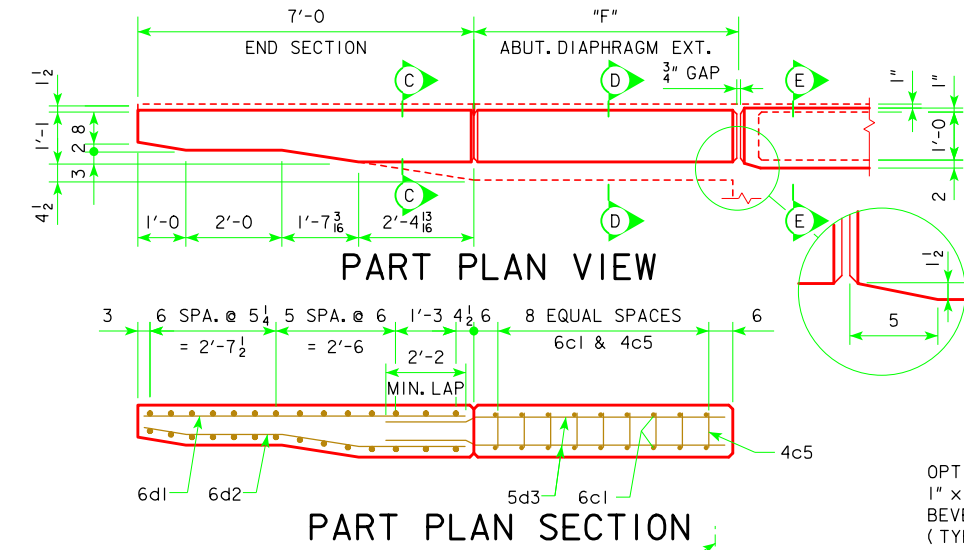
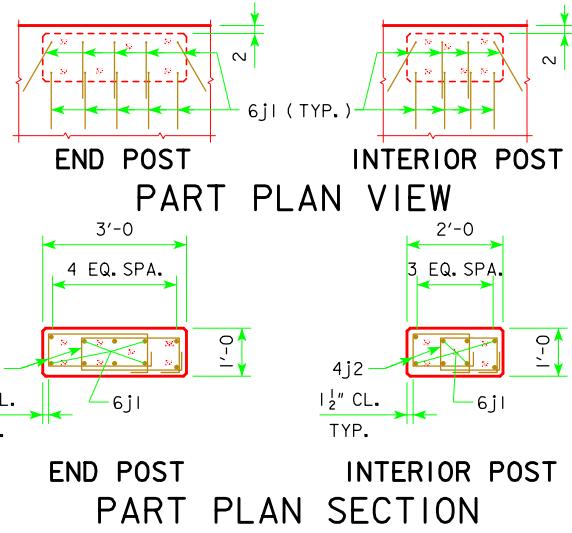
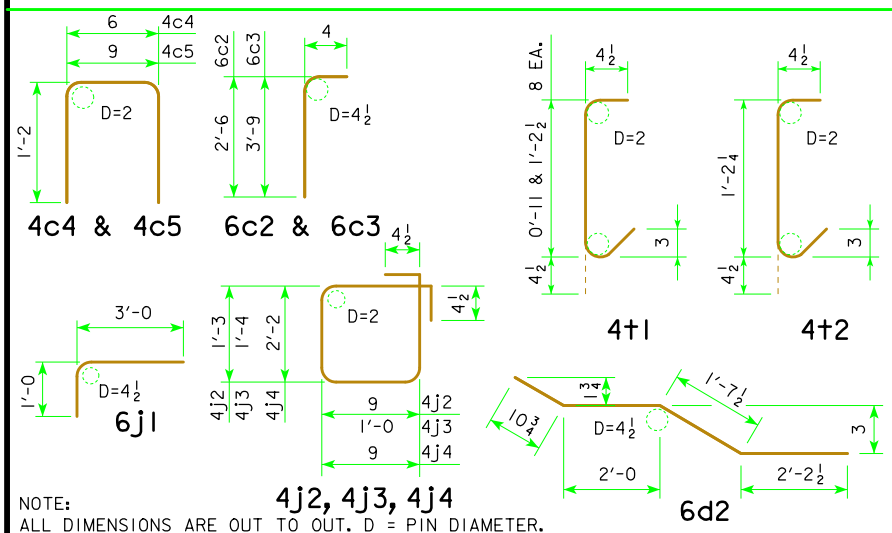
(NOTE: THESE REINFORCING BARS TO BE USED ON ALL SKEWS)

| BRIDGE LENGTH | | | 46'-8 | | 55'-0 | | 67'-6 | | 80'-0 | | 90'-0 | | 100'-0 | | 110'-0 | | | | | | | | | |
|--|---|-------|-------|--------|--------|-----|--------|--------|-------|--------|--------|-----|--------|--------|--------|--------|--------|-----|--------|-------|-----|--------|-------|--|
| BAR | LOCATION | SHAPE | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | | | | | | | |
| 6c1 | VERTICAL, END SECTION & ABUT. DIAPH. EXT. | — | 96 | 4'-11 | 709 | 96 | 4'-11 | 709 | 96 | 4'-11 | 709 | 168 | 4'-11 | 1,241 | 168 | 4'-11 | 1,241 | 168 | 4'-11 | 1,241 | | | | |
| 6c2 | VERTICAL, END SECTION | — | 16 | 2'-10 | 68 | 16 | 2'-10 | 68 | 16 | 2'-10 | 68 | 16 | 2'-10 | 68 | 16 | 2'-10 | 68 | 16 | 2'-10 | 68 | | | | |
| 6c3 | VERTICAL, END SECTION | — | 16 | 4'-1 | 98 | 16 | 4'-1 | 98 | 16 | 4'-1 | 98 | 16 | 4'-1 | 98 | 16 | 4'-1 | 98 | 16 | 4'-1 | 98 | | | | |
| 4c4 | VERTICAL HOOPS, END SECTION | □ | 20 | 2'-10 | 38 | 20 | 2'-10 | 38 | 20 | 2'-10 | 38 | 20 | 2'-10 | 38 | 20 | 2'-10 | 38 | 20 | 2'-10 | 38 | | | | |
| 4c5 | VERT. HOOPS, END SEC. & ABUT. DIAPH. EXT. | □ | 16 | 3'-1 | 33 | 16 | 3'-1 | 33 | 16 | 3'-1 | 33 | 52 | 3'-1 | 107 | 52 | 3'-1 | 107 | 52 | 3'-1 | 107 | | | | |
| 6d1 | HORIZONTAL, END SECTION-BACK FACE | — | 24 | 6'-8 | 240 | 24 | 6'-8 | 240 | 24 | 6'-8 | 240 | 24 | 6'-8 | 240 | 24 | 6'-8 | 240 | 24 | 6'-8 | 240 | | | | |
| 6d2 | HORIZONTAL, END SECTION-TRAFFIC FACE | — | 32 | 6'-9 | 324 | 32 | 6'-9 | 324 | 32 | 6'-9 | 324 | 32 | 6'-9 | 324 | 32 | 6'-9 | 324 | 32 | 6'-9 | 324 | | | | |
| 5d3* | HORIZONTAL, ABUT. DIAPH. EXT.-BOTH FACES | — | — | — | — | — | — | — | — | — | — | 48 | 7'-2 | 359 | 48 | 7'-2 | 359 | 48 | 7'-2 | 359 | | | | |
| 6h1 | LONGITUDINAL, OPEN RAIL | — | 24 | 26'-6 | 955 | 24 | 30'-8 | 1,106 | 24 | 36'-11 | 1,331 | 36 | 29'-10 | 1,613 | 36 | 33'-2 | 1,793 | 36 | 36'-6 | 1,974 | 36 | 39'-10 | 2,154 | |
| 6j1 | VERTICAL DOWELS, OPEN RAIL | ⌋ | 120 | 4'-0 | 721 | 136 | 4'-0 | 818 | 168 | 4'-0 | 1,009 | 184 | 4'-0 | 1,105 | 216 | 4'-0 | 1,298 | 232 | 4'-0 | 1,394 | 248 | 4'-0 | 1,490 | |
| 4j2 | HOOP, INTERIOR POST | □ | 80 | 4'-9 | 254 | 96 | 4'-9 | 305 | 128 | 4'-9 | 406 | 144 | 4'-9 | 457 | 176 | 4'-9 | 558 | 192 | 4'-9 | 609 | 208 | 4'-9 | 660 | |
| 4j3 | HOOP, OPEN RAIL | □ | 160 | 5'-5 | 579 | 186 | 5'-5 | 673 | 238 | 5'-5 | 861 | 264 | 5'-5 | 955 | 316 | 5'-5 | 1,143 | 342 | 5'-5 | 1,237 | 368 | 5'-5 | 1,332 | |
| 4j4 | HOOP, END POST | □ | 32 | 6'-7 | 141 | 32 | 6'-7 | 141 | 32 | 6'-7 | 141 | 32 | 6'-7 | 141 | 32 | 6'-7 | 141 | 32 | 6'-7 | 141 | 32 | 6'-7 | 141 | |
| 4t1 | WING FOOTING TIE BARS | — | 16 | VARIES | 19 | 16 | VARIES | 19 | 16 | VARIES | 19 | 16 | VARIES | 19 | 16 | VARIES | 19 | 16 | VARIES | 19 | 16 | VARIES | 19 | |
| 4t2 | WING FOOTING TIE BARS | — | — | — | — | — | — | — | — | — | — | 40 | 1'-11 | 51 | 40 | 1'-11 | 51 | 40 | 1'-11 | 51 | 40 | 1'-11 | 51 | |
| TOTAL LBS. (INCLUDE WITH SUPERSTRUCTURE REINFORCING) | | | 4179 | | 4572 | | 5277 | | 6816 | | 7478 | | 7900 | | 8322 | | | | | | | | | |

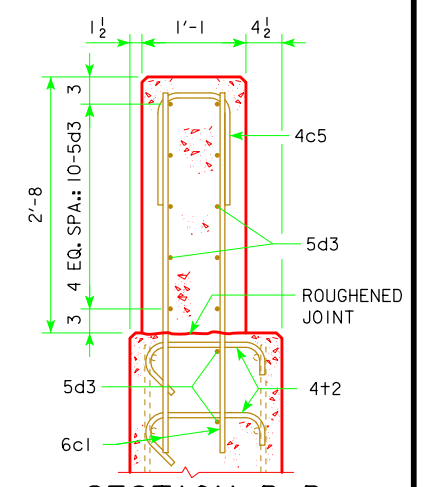
NOTE: ALL BARRIER RAIL REINFORCEMENT TO BE EPOXY COATED IF EPOXY COATING OPTION IS USED.

* TRAFFIC FACE 5d3 BARS MAY REQUIRE FIELD CUTTING OR BENDING FOR HIGHER SKEW BRIDGES.

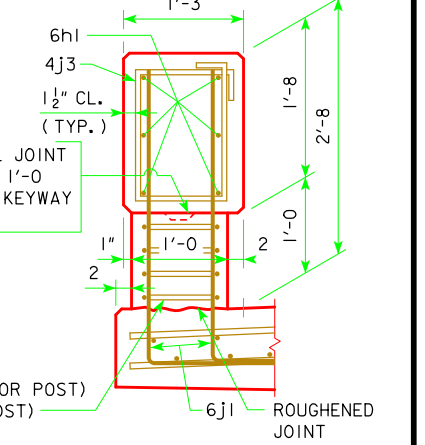
BENT BAR DETAILS



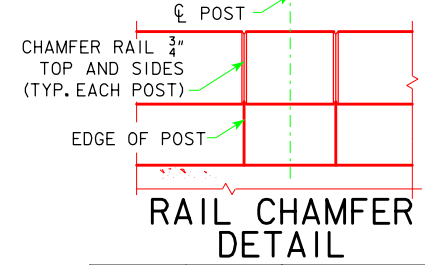
SECTION C-C



SECTION D-D



PART SECTION E-E



RAIL CHAMFER DETAIL

CONCRETE PLACEMENT SUMMARY - C.Y.

| BRIDGE LENGTH | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 |
|--------------------------------|----------------------------|-------|-------|-------|-------|--------|--------|
| OPEN RAIL SECTION | 2 @ 0.077 CU. YDS. PER FT. | 7.8 | 9.0 | 10.9 | 12.9 | 14.4 | 15.9 |
| OPEN RAIL-END SECTION | 4 @ 0.687 CU. YDS. | 2.7 | 2.7 | 2.7 | 2.7 | 2.7 | 2.7 |
| OPEN RAIL-ABUT. DIAPH. SECTION | 4 @ 0.107 CU. YDS. PER FT. | — | — | — | 1.9 | 1.9 | 1.9 |
| OPEN RAIL-END POSTS | 4 @ 0.11 CU. YDS. | 0.4 | 0.4 | 0.4 | 0.4 | 0.4 | 0.4 |
| OPEN RAIL-INTERIOR POSTS | 2 x "E" @ 0.07 CU. YDS. | 0.7 | 0.8 | 1.1 | 1.3 | 1.5 | 1.8 |
| TOTAL (C.Y.) | | 11.6 | 12.9 | 15.1 | 19.2 | 20.9 | 24.3 |

CONCRETE QUANTITIES SHOWN ARE BASED ON 30° SKEW.

OPEN CONCRETE RAIL, TL-4 QUANTITIES - L.F.

| BRIDGE LENGTH | 46'-8 | 55'-0 | 67'-6 | 80'-0 | 90'-0 | 100'-0 | 110'-0 |
|--------------------------------------|-------|-------|-------|-------|-------|--------|--------|
| OPEN CONCRETE RAILING, TL-4 0° SKEW | 127.3 | 144.0 | 169.0 | 214.0 | 234.0 | 254.0 | 274.0 |
| OPEN CONCRETE RAILING, TL-4 15° SKEW | 127.5 | 144.2 | 169.2 | 214.2 | 234.2 | 254.2 | 274.2 |
| OPEN CONCRETE RAILING, TL-4 30° SKEW | 128.3 | 144.9 | 169.9 | 214.9 | 234.9 | 254.9 | 274.9 |

LATEST REVISION DATE

Thomas L. Mc Donald

APPROVED BY BRIDGE ENGINEER

STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE

PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES

APRIL, 2012

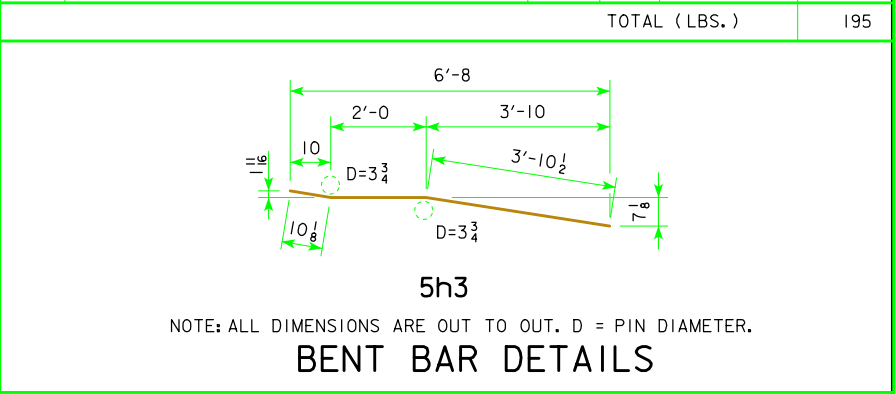
H30SI-35-12

OPEN RAIL, TL-4 DETAILS

SHEET 2 OF 2

REINFORCING BAR LIST - ONE ABUT. WING

| BAR | LOCATION | SHAPE | NO. | LENGTH | WEIGHT |
|--------------|-------------------------|-------|-----|--------|--------|
| 5h1 | HORIZONTAL BACK FACE | | 7 | 6'-8 | 49 |
| 5h3 | HORIZONTAL TRAFFIC FACE | | 7 | 6'-9 | 49 |
| 5s1 | VERTICAL BOTH FACES | | 16 | 5'-10 | 97 |
| TOTAL (LBS.) | | | | | 195 |

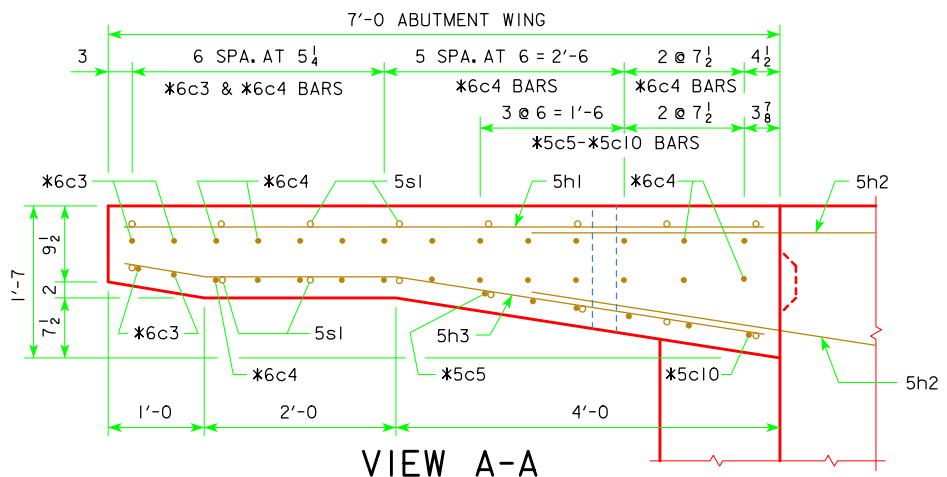


CONCRETE PLACEMENT SUMMARY

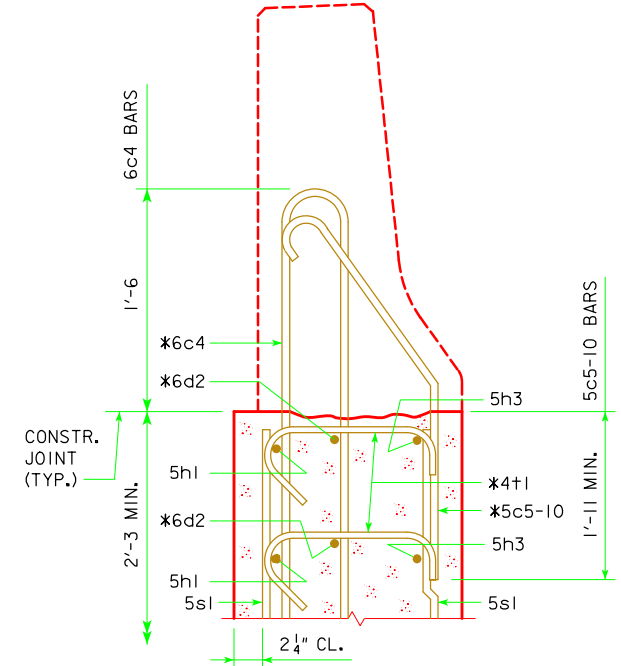
| CONCRETE | TOTAL |
|-------------------|-------|
| ONE ABUTMENT WING | 1.8 |
| TOTAL (CU. YDS.) | 1.8 |

NOTE:
REINFORCING STEEL QUANTITY AND CONCRETE QUANTITY ARE TO BE ADDED TO THE SUPERSTRUCTURE QUANTITIES SHOWN ELSEWHERE IN THESE PLANS.

| | | | |
|----------------------|---------------------------------|---|-------------|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | WING DETAILS | H30SI-36-12 |

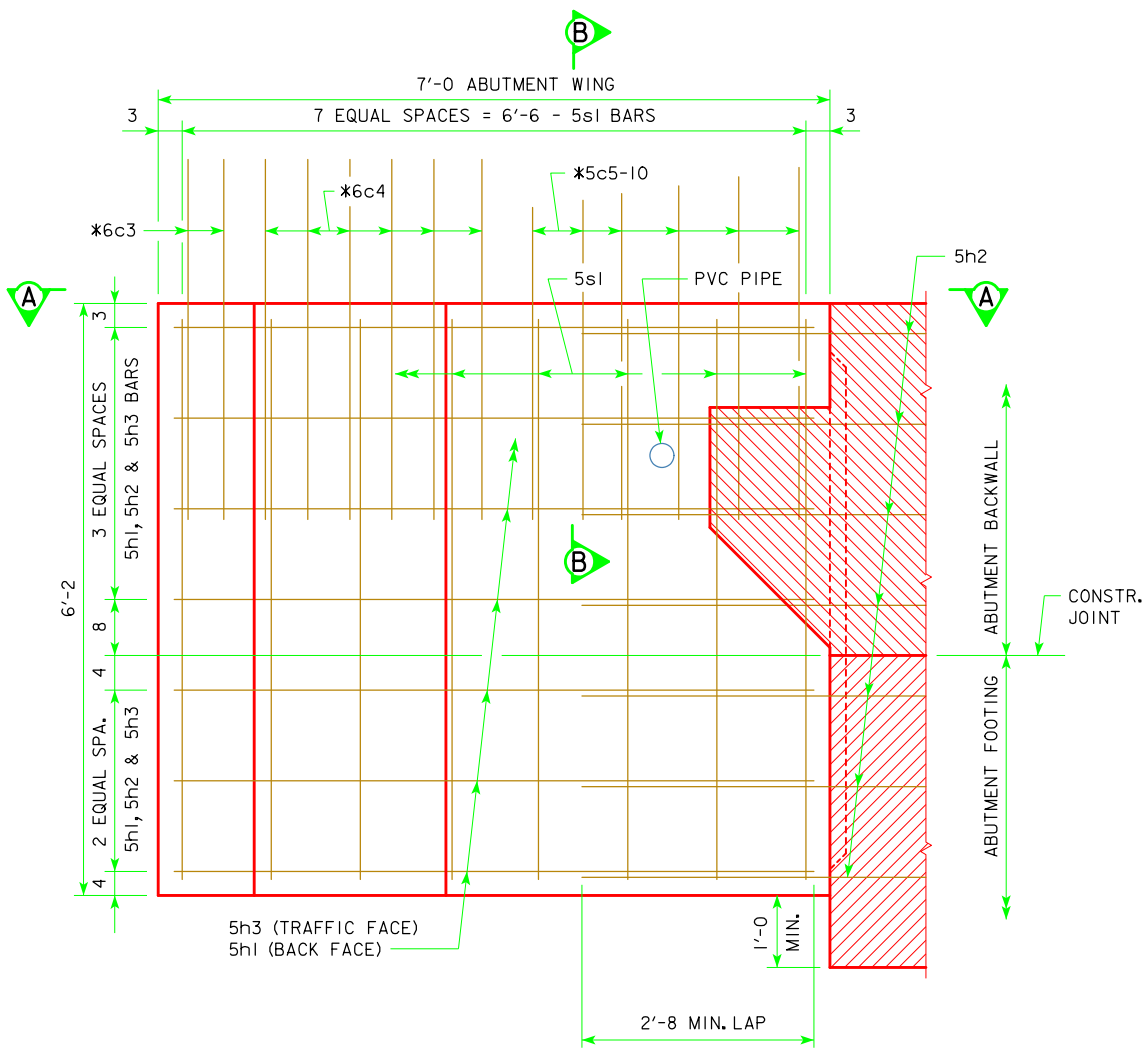
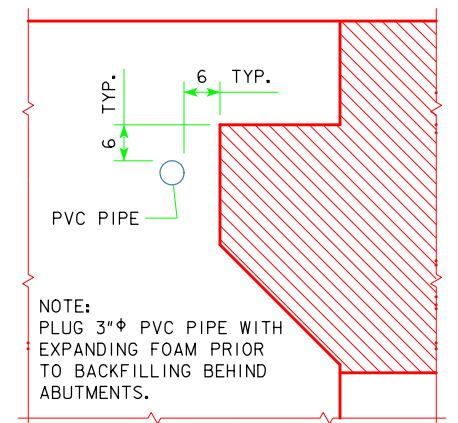


NOTE:
PLUG 3"φ PVC PIPE WITH EXPANDING FOAM PRIOR TO BACKFILLING BEHIND ABUTMENTS.



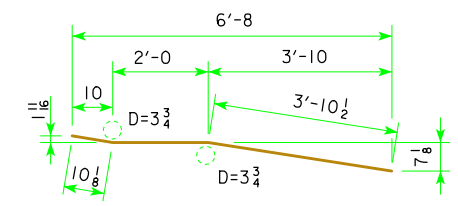
* BARRIER RAIL END SECTION BARS TO BE PLACED WITH ABUTMENT WING.

SEE END SECTION DETAILS IN THESE PLANS FOR DETAILS OF BARRIER RAIL END SECTION. REINFORCING BARS 6c3, 6c4, 5c5-10, 6d2 & 4+1 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.



REINFORCING BAR LIST - ONE ABUT. WING

| BAR | LOCATION | SHAPE | NO. | LENGTH | WEIGHT |
|--------------|-------------------------|-------|-----|--------|--------|
| 5h1 | HORIZONTAL BACK FACE | | 7 | 6'-8" | 49 |
| 5h3 | HORIZONTAL TRAFFIC FACE | | 7 | 6'-9" | 49 |
| 5s1 | VERTICAL BOTH FACES | | 16 | 6'-2" | 103 |
| TOTAL (LBS.) | | | | | 201 |

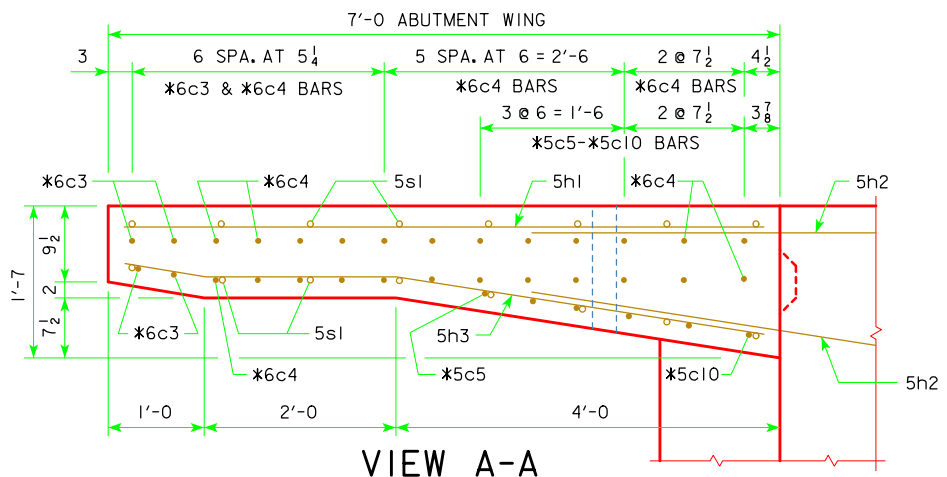


5h3
NOTE: ALL DIMENSIONS ARE OUT TO OUT. D = PIN DIAMETER.
BENT BAR DETAILS

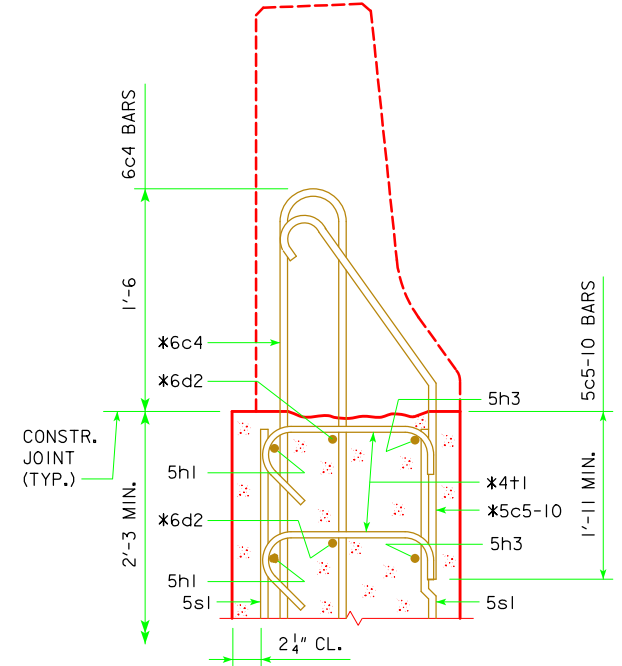
CONCRETE PLACEMENT SUMMARY

| CONCRETE | TOTAL |
|-------------------|-------|
| ONE ABUTMENT WING | 1.9 |
| TOTAL (CU. YDS.) | 1.9 |

NOTE:
REINFORCING STEEL QUANTITY AND CONCRETE QUANTITY ARE TO BE ADDED TO THE SUPERSTRUCTURE QUANTITIES SHOWN ELSEWHERE IN THESE PLANS.

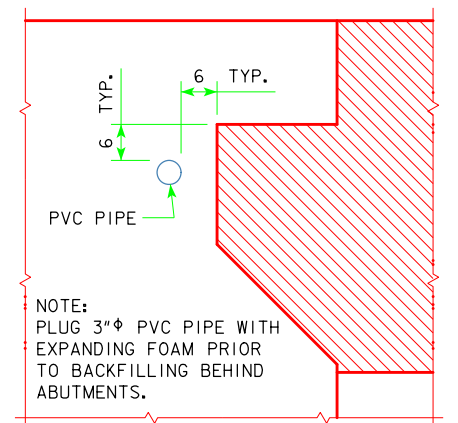


NOTE:
PLUG 3"φ PVC PIPE WITH EXPANDING FOAM PRIOR TO BACKFILLING BEHIND ABUTMENTS.



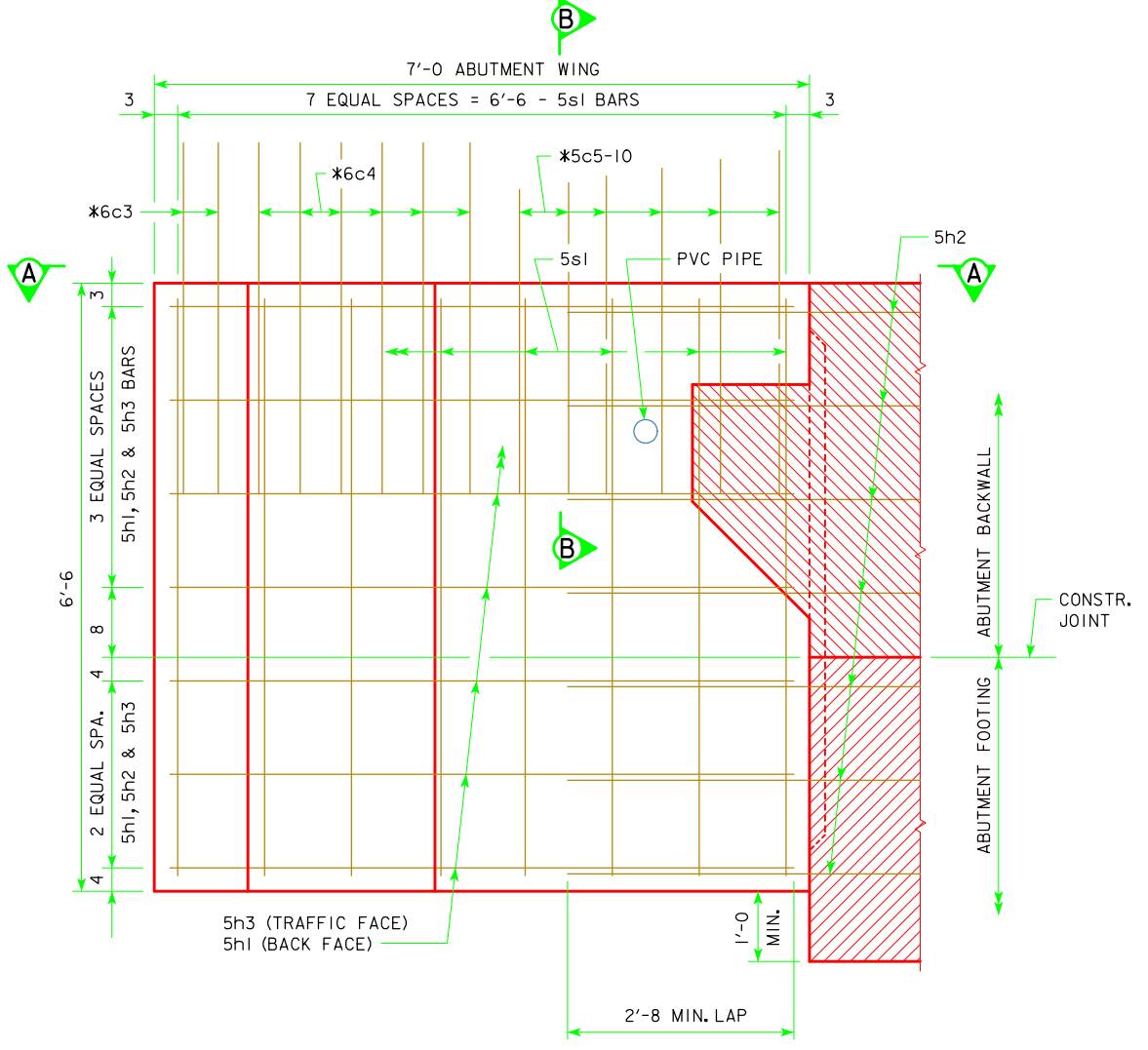
SECTION B-B

* BARRIER RAIL END SECTION BARS TO BE PLACED WITH ABUTMENT WING.
SEE END SECTION DETAILS IN THESE PLANS FOR DETAILS OF BARRIER RAIL END SECTION. REINFORCING BARS 6c3, 6c4, 5c5-10, 6d2 & 4t1 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.



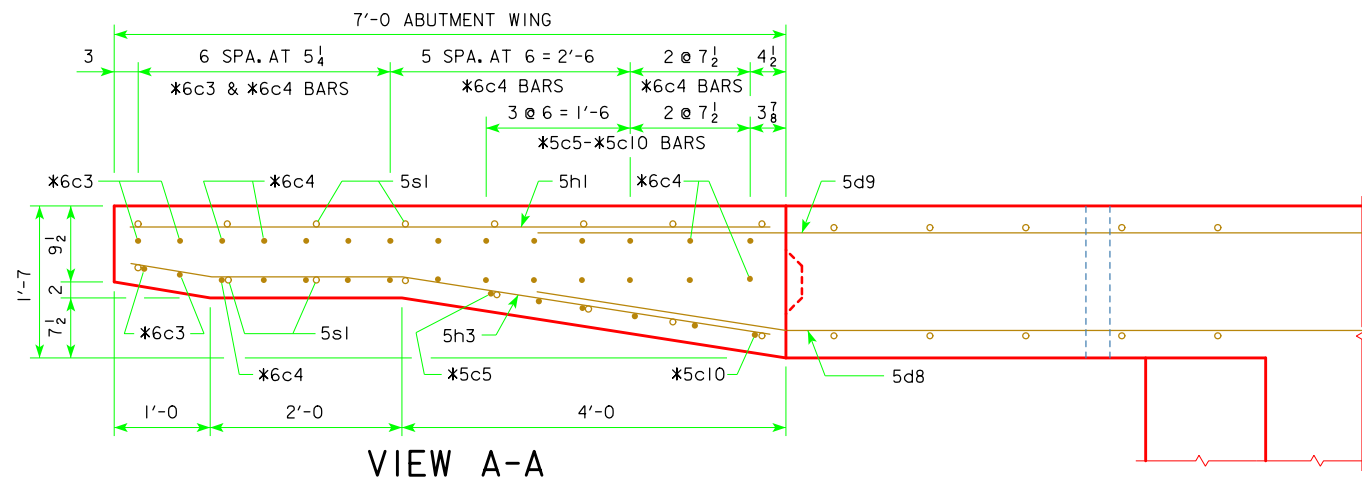
PVC PIPE LOCATION

NOTE:
PLUG 3"φ PVC PIPE WITH EXPANDING FOAM PRIOR TO BACKFILLING BEHIND ABUTMENTS.

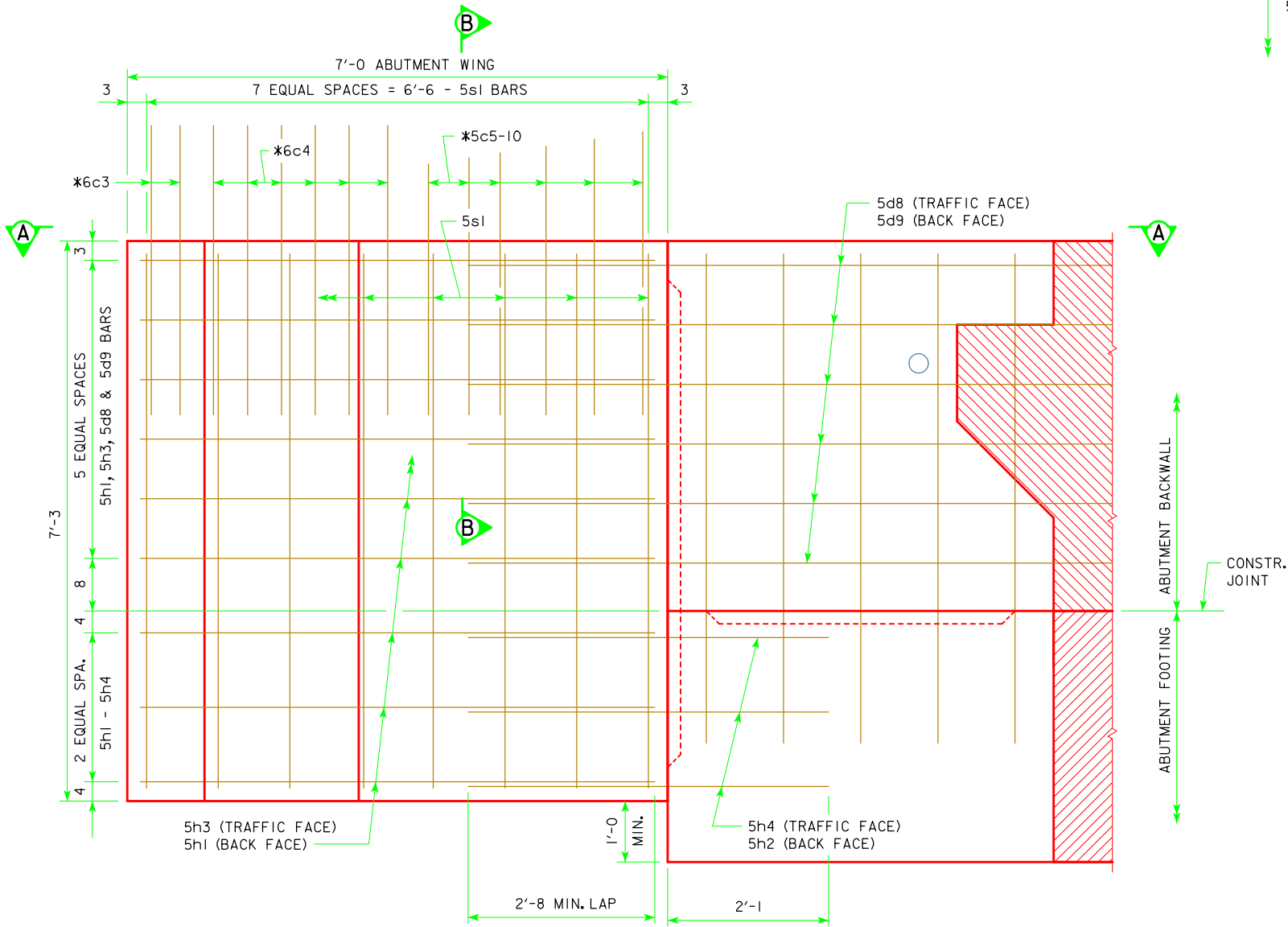


ABUTMENT WING - ELEVATION VIEW

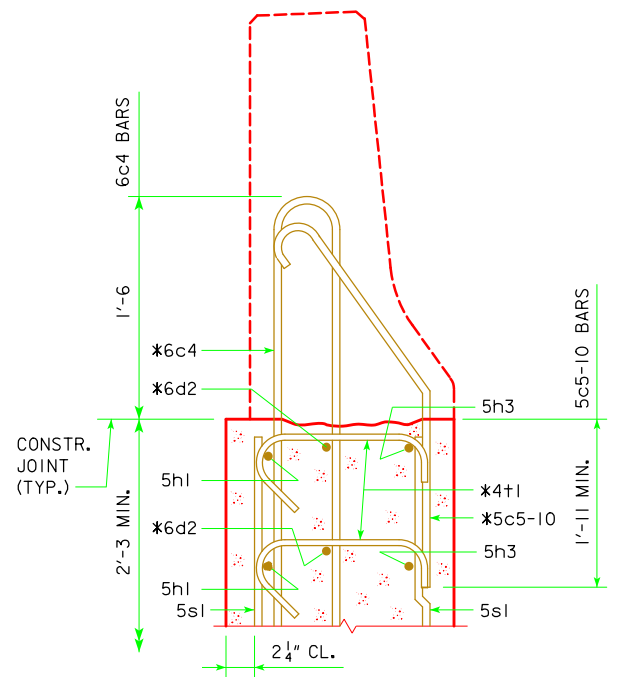
| | | | |
|----------------------|---------------------------------|---|-------------|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | WING DETAILS | H30SI-37-12 |
| | | | |



VIEW A-A



ABUTMENT WING - ELEVATION VIEW



SECTION B-B

* BARRIER RAIL END SECTION BARS TO BE PLACED WITH ABUTMENT WING.
 SEE END SECTION DETAILS IN THESE PLANS FOR DETAILS OF BARRIER RAIL END SECTION. REINFORCING BARS 6c3, 6c4, 5c5-10, 6d2 & 4+1 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.

| REINFORCING BAR LIST - ONE ABUT. WING | | | | | |
|---------------------------------------|-------------------------|-------|-----|--------|--------|
| BAR | LOCATION | SHAPE | NO. | LENGTH | WEIGHT |
| 5h1 | HORIZONTAL BACK FACE | | 9 | 6'-8 | 63 |
| 5h3 | HORIZONTAL TRAFFIC FACE | | 9 | 6'-9 | 63 |
| 5s1 | VERTICAL BOTH FACES | | 16 | 6'-11 | 115 |
| TOTAL (LBS.) | | | | | 241 |

5h3
 NOTE: ALL DIMENSIONS ARE OUT TO OUT. D = PIN DIAMETER.

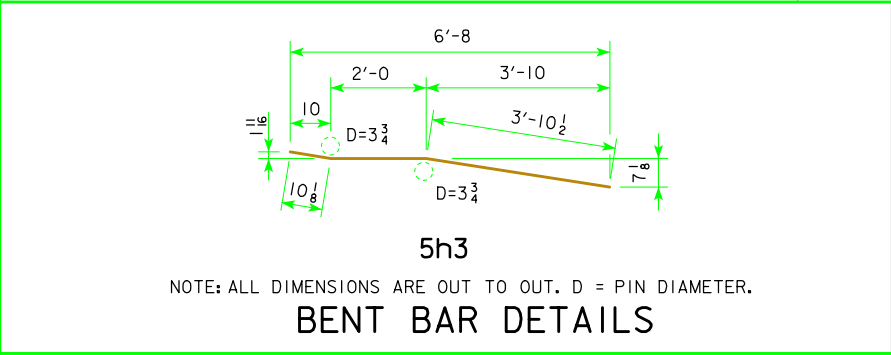
| CONCRETE PLACEMENT SUMMARY | |
|----------------------------|-------|
| CONCRETE | TOTAL |
| ONE ABUTMENT WING | 2.1 |
| TOTAL (CU. YDS.) | 2.1 |

NOTE:
 REINFORCING STEEL QUANTITY AND CONCRETE QUANTITY ARE TO BE ADDED TO THE SUPERSTRUCTURE QUANTITIES SHOWN ELSEWHERE IN THESE PLANS.

| | | | |
|----------------------|---|---|-------------|
| LATEST REVISION DATE | <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | WING DETAILS | H30SI-38-12 |

REINFORCING BAR LIST - ONE ABUT. WING

| BAR | LOCATION | SHAPE | NO. | LENGTH | WEIGHT |
|--------------|-------------------------|-------|-----|--------|--------|
| 5h1 | HORIZONTAL BACK FACE | | 9 | 6'-8" | 63 |
| 5h3 | HORIZONTAL TRAFFIC FACE | | 9 | 6'-9" | 63 |
| 5s1 | VERTICAL BOTH FACES | | 16 | 7'-8" | 128 |
| TOTAL (LBS.) | | | | | 254 |

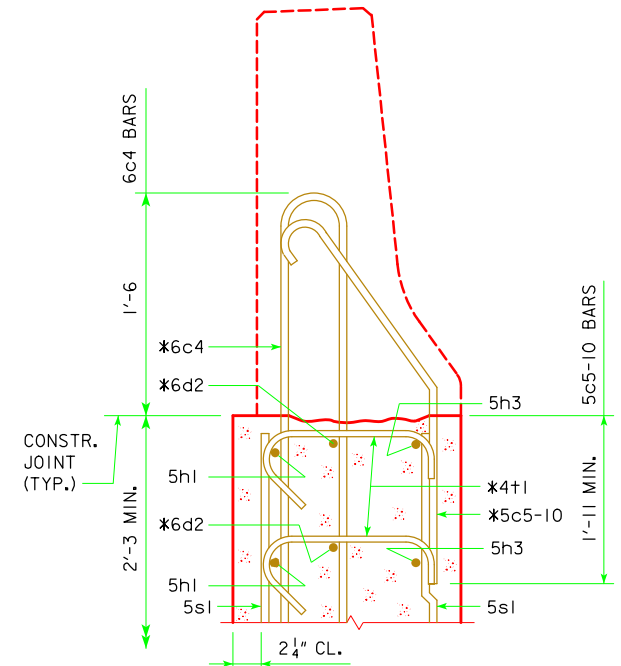
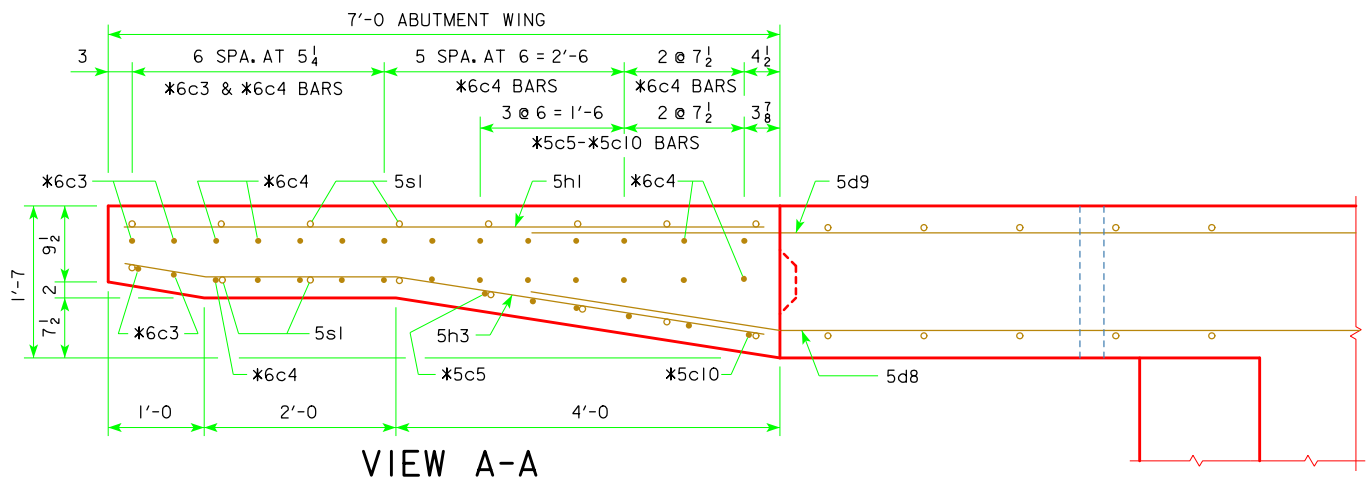


CONCRETE PLACEMENT SUMMARY

| CONCRETE | TOTAL |
|-------------------|-------|
| ONE ABUTMENT WING | 2.3 |
| TOTAL (CU. YDS.) | 2.3 |

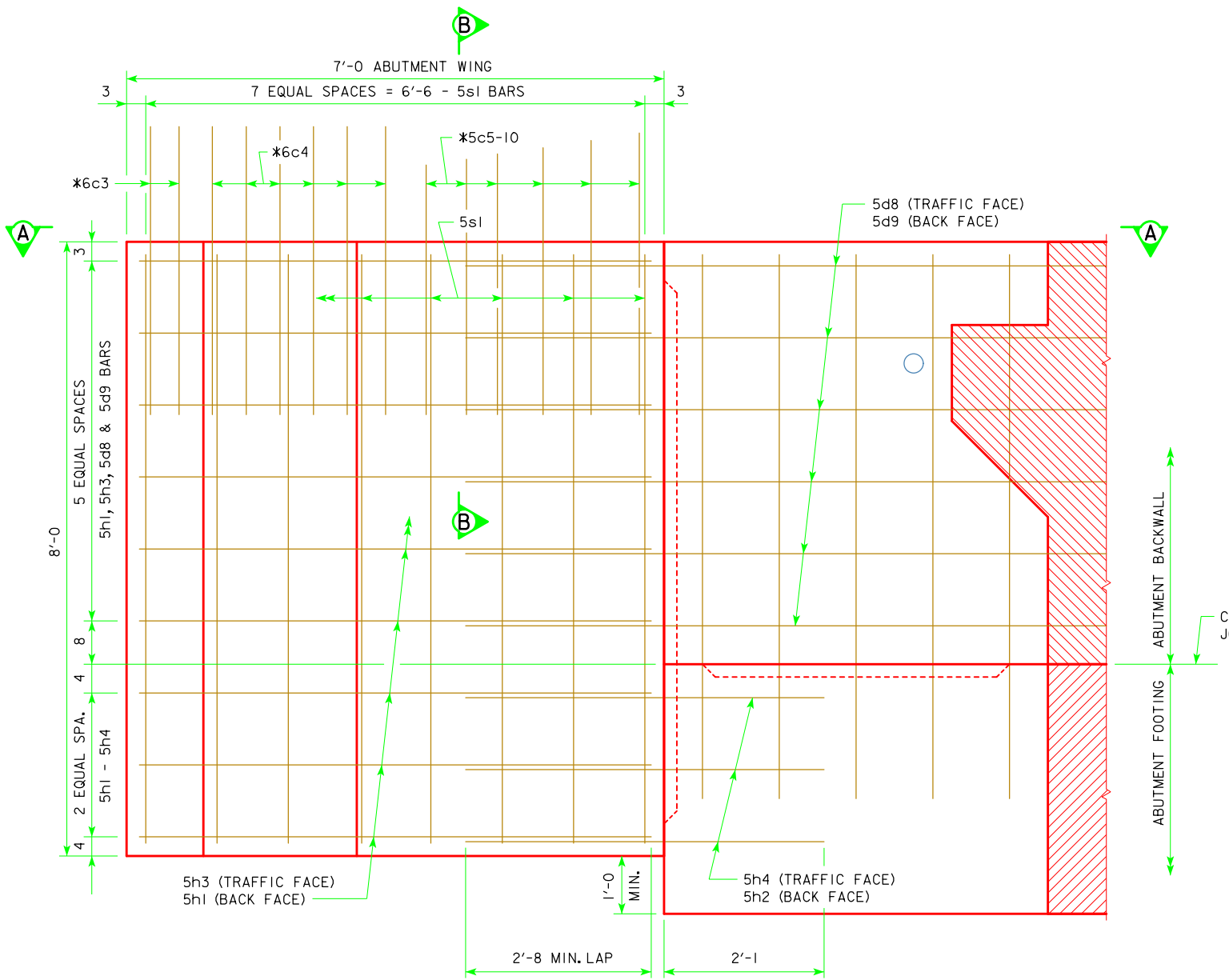
NOTE:
REINFORCING STEEL QUANTITY AND CONCRETE QUANTITY ARE TO BE ADDED TO THE SUPERSTRUCTURE QUANTITIES SHOWN ELSEWHERE IN THESE PLANS.

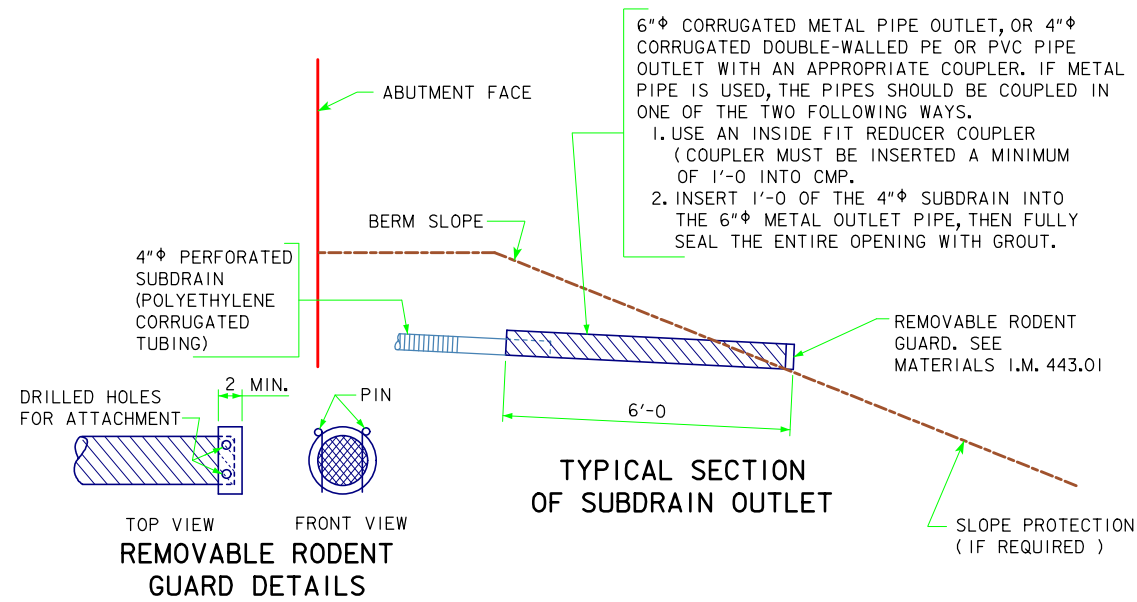
| | | | |
|----------------------|---------------------------------|---|-------------|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | WING DETAILS | H30SI-39-12 |



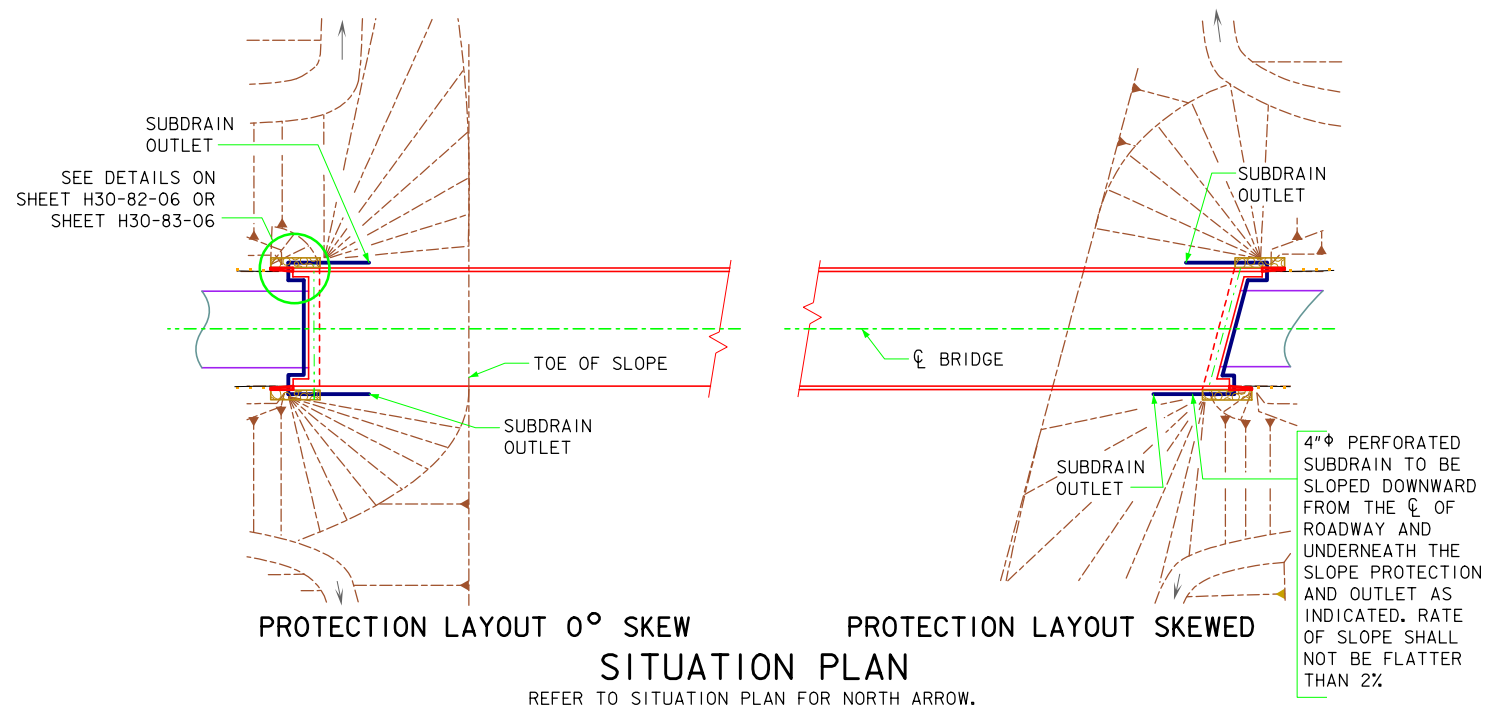
*** BARRIER RAIL END SECTION BARS TO BE PLACED WITH ABUTMENT WING.**

SEE END SECTION DETAILS IN THESE PLANS FOR DETAILS OF BARRIER RAIL END SECTION. REINFORCING BARS 6c3, 6c4, 5c5-10, 6d2 & 4t1 ARE INCLUDED IN THE SUPERSTRUCTURE QUANTITIES.





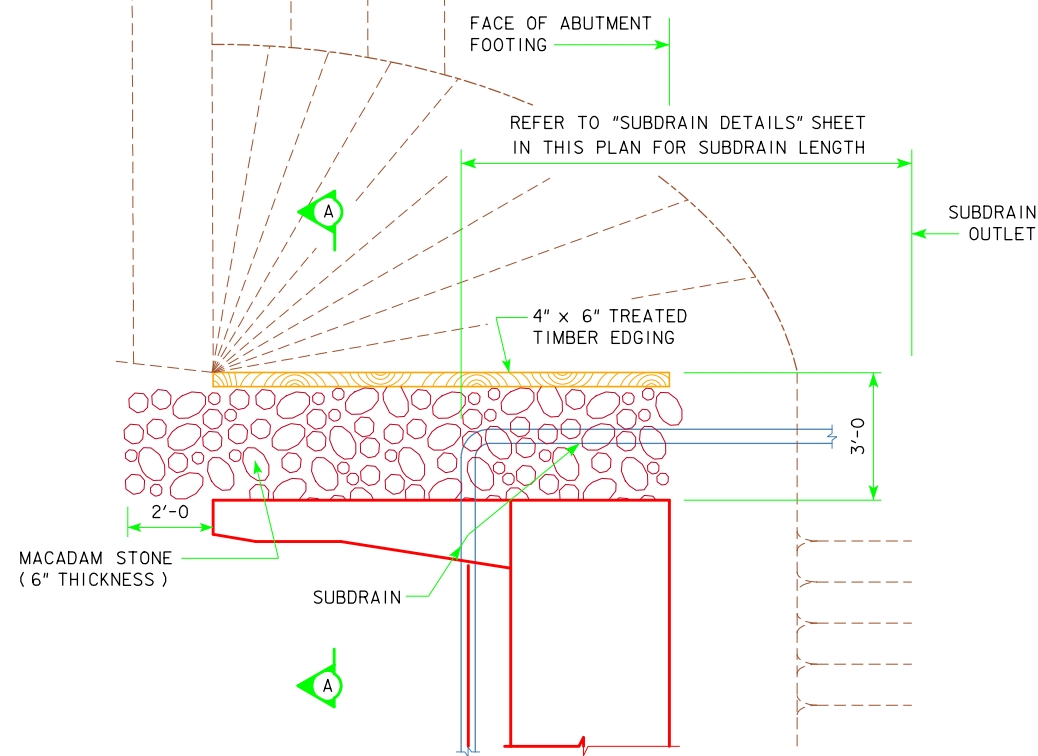
OUTLET DETAILS



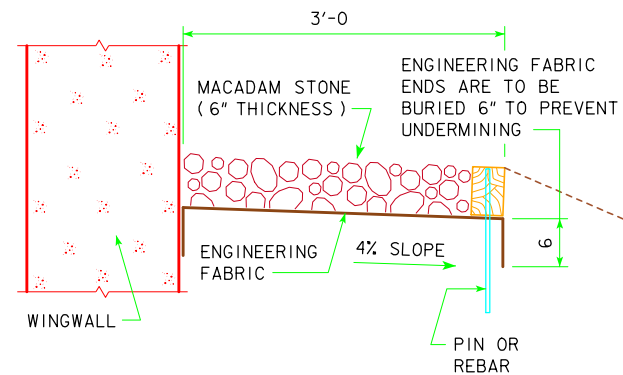
NOTE:
SEE ABUTMENT BACKFILL DETAILS SHEET FOR DETAILS NOT SHOWN ON THIS SHEET WHICH ARE PERTINENT TO THIS STRUCTURE.

| | | | |
|----------------------|---------------------------------|---|-------------|
| LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | SUBDRAIN DETAILS | H30SI-40-12 |

REVISED 09-14 - THE MACADAM STONE AREA WAS MODIFIED TO EXTEND 2 FEET IN FRONT OF THE BRIDGE WING.



TOP VIEW OF WING ARMORING



SECTION A-A

SUBDRAIN NOTES:

SEE H30SI-40-12 AND "GENERAL ELEVATION DATA" SHEETS FOR DETAILS OF PLACING ALL SUBDRAINS AND SUBDRAIN OUTLETS REQUIRED FOR THIS STRUCTURE.

THE BRIDGE CONTRACTOR IS TO INSTALL SUBDRAINS BEHIND THE ABUTMENT. THE SUBDRAINS SHALL BE 4" IN DIAMETER AND MEET THE REQUIREMENTS OF SECTION 4143.01 B OF THE CURRENT I.D.O.T. STANDARD SPECIFICATION. THE SUBDRAIN OUTLET SHALL CONSIST OF A 6'-0 LENGTH OF PIPE WITH A REMOVABLE RODENT GUARD.

THE DIMENSIONS SHOWN FOR THE PROPOSED SUBDRAINS ARE BASED ON THE PROPOSED GRADING LAYOUT OF BRIDGE BERMS. THE DIMENSIONS SHOWN ARE FOR ESTIMATING ONLY. REQUIRED LENGTHS AND GENERAL LOCATIONS OF SUBDRAINS ARE SUBJECT TO CHANGE DUE TO FIELD ADJUSTMENTS OF THE GRADING LAYOUT.

THE COST OF FURNISHING AND PLACING SUBDRAIN (INCLUDING EXCAVATION), GRANULAR BACKFILL, POROUS BACKFILL, AND SUBDRAIN OUTLET IS TO BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL CONCRETE (BRIDGE)". NO EXTRA PAYMENT WILL BE MADE.

MACADAM STONE WING ARMORING NOTES:

MACADAM STONE SHALL BE PLACED ALONG THE SIDE OF THE WING AND ABUTMENT FOOTING. THIS IS TYPICAL AT EACH CORNER OF THE BRIDGE UNLESS OTHERWISE NOTED IN THE PLANS. THE MACADAM STONE AT THESE LOCATIONS SHALL BE UNDERLAYED WITH ENGINEERING FABRIC.

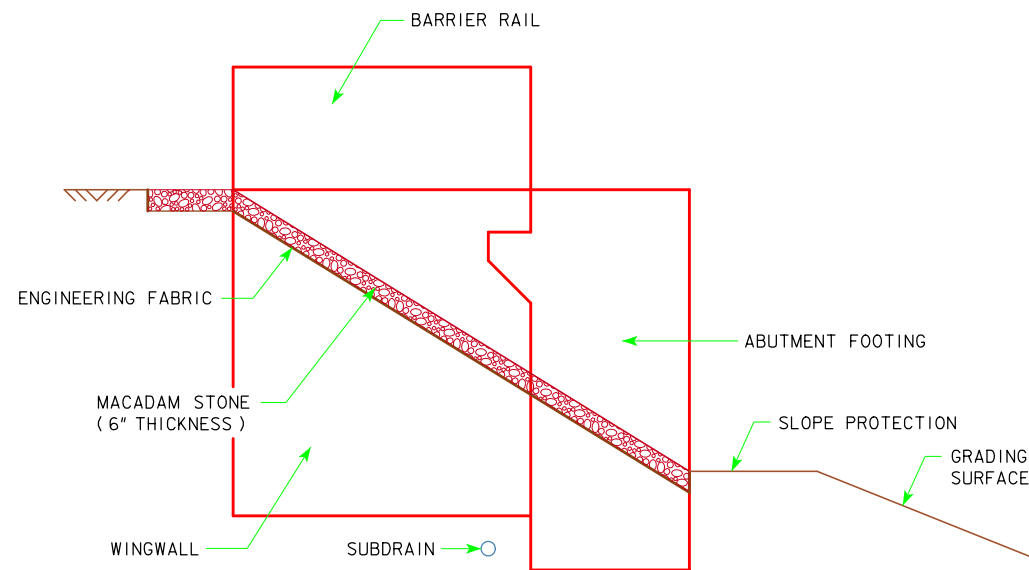
THE BRIDGE BERM SLOPE SHALL BE COMPACTED AND SHAPED AS SHOWN ON THESE PLANS, THE SITUATION PLAN, AND AS DIRECTED BY THE ENGINEER. THE BERM SLOPE SHALL BE FIRM WHEN THE ENGINEERING FABRIC AND MACADAM STONE ARE PLACED.

THE ENGINEERING FABRIC SHALL BE IN ACCORDANCE WITH 4196.01, B, 3, OF THE STANDARD SPECIFICATIONS. ENGINEERING FABRIC IS LAPPED THE LAPS SHALL BE A MINIMUM OF ONE FOOT IN LENGTH, SHINGLE FASHION WITH UP SLOPE LAP PIECE ON TOP AND STAPLED FOR CONTINUITY.

THE MACADAM STONE SHALL MEET THE REQUIREMENTS OF THE CURRENT STANDARD SPECIFICATIONS.

THE MACADAM STONE SHALL BE DEPOSITED, SPREAD, CONSOLIDATED AND SHAPED BY MECHANICAL OR HAND METHODS THAT WILL PROVIDE UNIFORM DEPTH AND DENSITY AND PROVIDE UNIFORM SURFACE APPEARANCE.

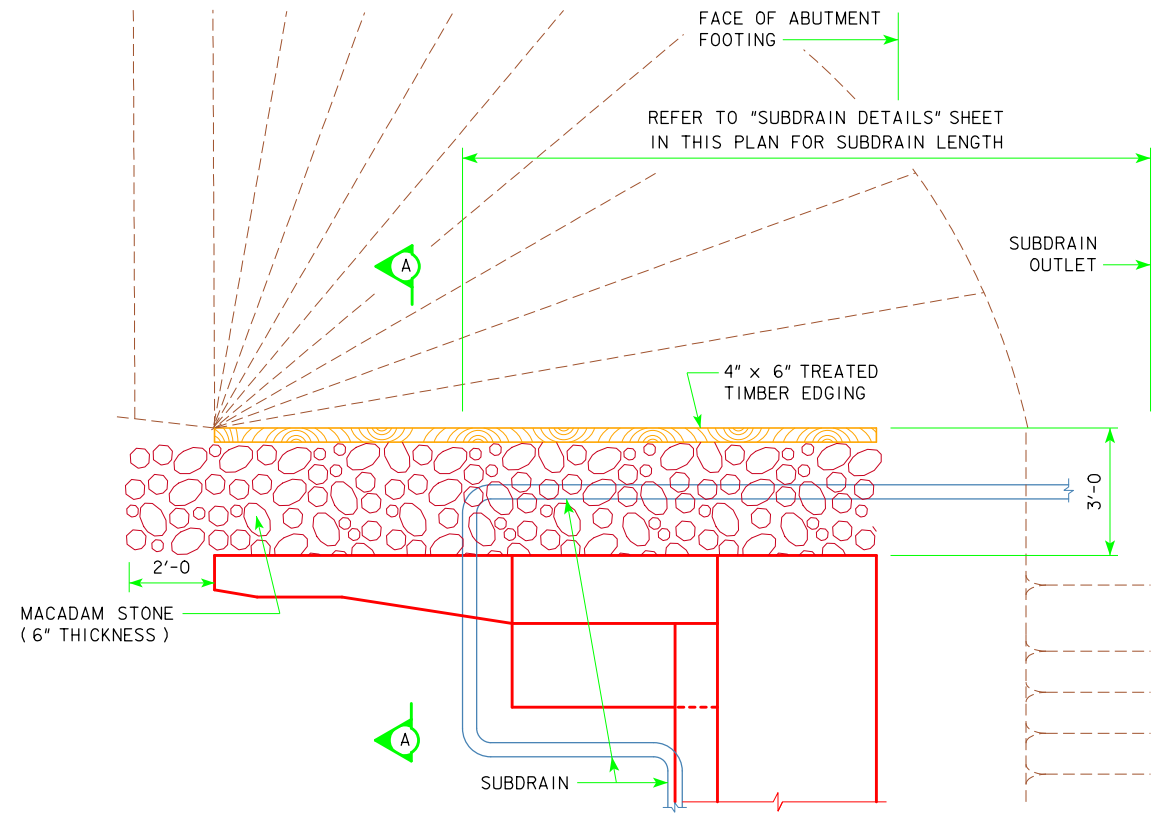
PAYMENT FOR THE BRIDGE WING ARMORING WILL BE BID PER SQUARE YARD. COST WILL INCLUDE ENGINEERING FABRIC, MACADAM STONE, EXCAVATION, SHAPING, AND COMPACTION TO DIMENSIONS SHOWN IN THESE PLANS. BID ITEM SHALL BE "BRIDGE WING ARMORING - MACADAM STONE".



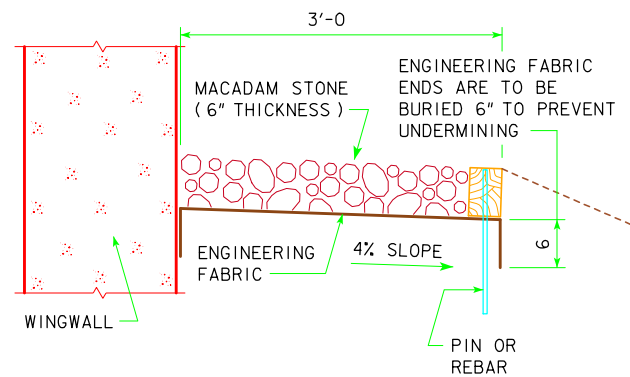
PROFILE VIEW OF WING ARMORING
(SHOWN FOR INTEGRAL ABUTMENT)

| | | | | |
|----------------------|-------|---------------------------------|---|--------------------|
| LATEST REVISION DATE | 09-14 | APPROVED BY BRIDGE ENGINEER | | |
| | | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | | | WING ARMORING DETAILS A & B BEAMS | H30SI-41-12 |

REVISED 09-14 - THE MACADAM STONE AREA WAS MODIFIED TO EXTEND 2 FEET IN FRONT OF THE BRIDGE WING.



TOP VIEW OF WING ARMORING WITH WING EXTENSION



SECTION A-A

SUBDRAIN NOTES:

SEE H30SI-40-12 AND "GENERAL ELEVATION DATA" SHEETS FOR DETAILS OF PLACING ALL SUBDRAINS AND SUBDRAIN OUTLETS REQUIRED FOR THIS STRUCTURE.

THE BRIDGE CONTRACTOR IS TO INSTALL SUBDRAINS BEHIND THE ABUTMENT. THE SUBDRAINS SHALL BE 4" IN DIAMETER AND MEET THE REQUIREMENTS OF SECTION 4143.01 B OF THE CURRENT I.D.O.T. STANDARD SPECIFICATION. THE SUBDRAIN OUTLET SHALL CONSIST OF A 6'-0 LENGTH OF PIPE WITH A REMOVABLE RODENT GUARD.

THE DIMENSIONS SHOWN FOR THE PROPOSED SUBDRAINS ARE BASED ON THE PROPOSED GRADING LAYOUT OF BRIDGE BERMS. THE DIMENSIONS SHOWN ARE FOR ESTIMATING ONLY. REQUIRED LENGTHS AND GENERAL LOCATIONS OF SUBDRAINS ARE SUBJECT TO CHANGE DUE TO FIELD ADJUSTMENTS OF THE GRADING LAYOUT.

THE COST OF FURNISHING AND PLACING SUBDRAIN (INCLUDING EXCAVATION), GRANULAR BACKFILL, POROUS BACKFILL, AND SUBDRAIN OUTLET IS TO BE INCLUDED IN THE PRICE BID FOR "STRUCTURAL CONCRETE (BRIDGE)". NO EXTRA PAYMENT WILL BE MADE.

MACADAM STONE WING ARMORING NOTES:

MACADAM STONE SHALL BE PLACED ALONG THE SIDE OF THE WING AND ABUTMENT FOOTING. THIS IS TYPICAL AT EACH CORNER OF THE BRIDGE UNLESS OTHERWISE NOTED IN THE PLANS. THE MACADAM STONE AT THESE LOCATIONS SHALL BE UNDERLAYED WITH ENGINEERING FABRIC.

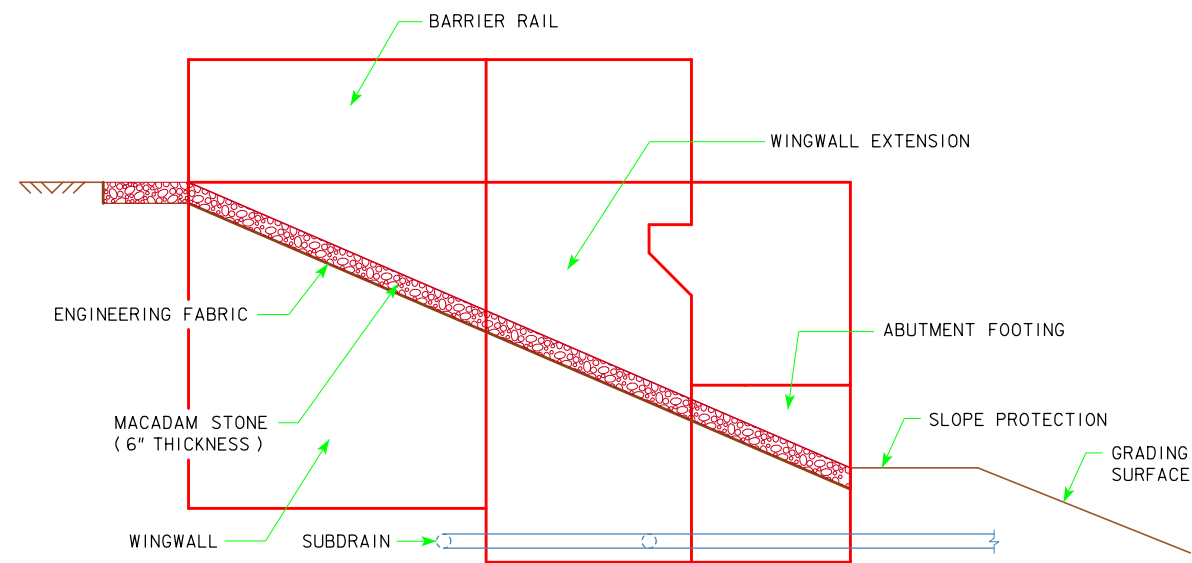
THE BRIDGE BERM SLOPE SHALL BE COMPACTED AND SHAPED AS SHOWN ON THESE PLANS, THE SITUATION PLAN, AND AS DIRECTED BY THE ENGINEER. THE BERM SLOPE SHALL BE FIRM WHEN THE ENGINEERING FABRIC AND MACADAM STONE ARE PLACED.

THE ENGINEERING FABRIC SHALL BE IN ACCORDANCE WITH 4196.01, B, 3, OF THE STANDARD SPECIFICATIONS. ENGINEERING FABRIC IS LAPPED THE LAPS SHALL BE A MINIMUM OF ONE FOOT IN LENGTH, SHINGLE FASHION WITH UP SLOPE LAP PIECE ON TOP AND STAPLED FOR CONTINUITY.

THE MACADAM STONE SHALL MEET THE REQUIREMENTS OF THE CURRENT STANDARD SPECIFICATIONS.

THE MACADAM STONE SHALL BE DEPOSITED, SPREAD, CONSOLIDATED AND SHAPED BY MECHANICAL OR HAND METHODS THAT WILL PROVIDE UNIFORM DEPTH AND DENSITY AND PROVIDE UNIFORM SURFACE APPEARANCE.

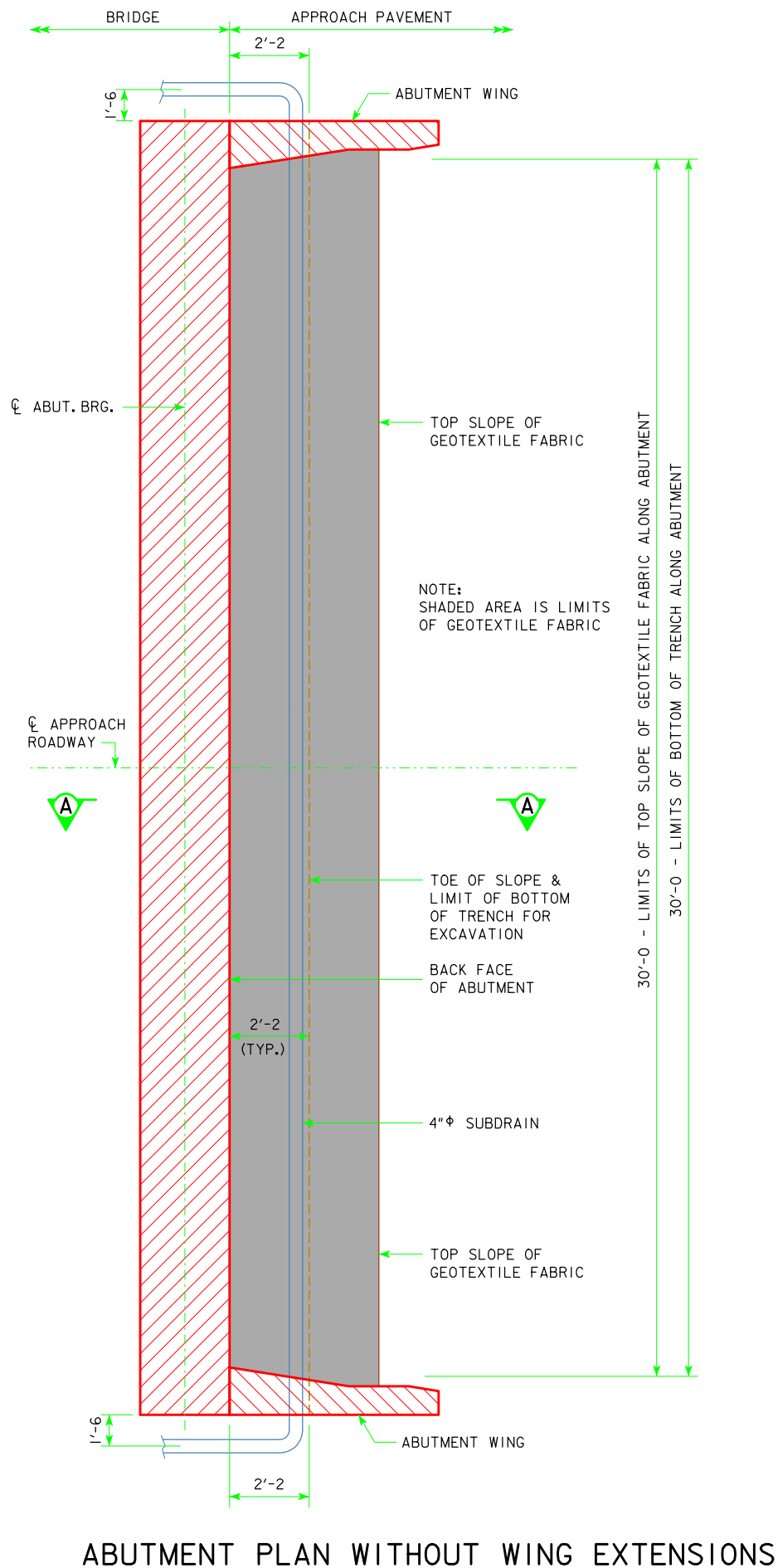
PAYMENT FOR THE BRIDGE WING ARMORING WILL BE BID PER SQUARE YARD. COST WILL INCLUDE ENGINEERING FABRIC, MACADAM STONE, EXCAVATION, SHAPING, AND COMPACTION TO DIMENSIONS SHOWN IN THESE PLANS. BID ITEM SHALL BE "BRIDGE WING ARMORING - MACADAM STONE".



PROFILE VIEW OF WING ARMORING WITH WING EXTENSION
(SHOWN FOR INTEGRAL ABUTMENT WITH WING EXTENSIONS)

| | | | |
|----------------------|-----------------------------|--|--|
| LATEST REVISION DATE | 09-14 | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE |
| | | | PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 |
| | APPROVED BY BRIDGE ENGINEER | | WING ARMORING DETAILS C & D BEAMS |

REVISED 09-14 - TECHNICAL DATA INFORMATION TABLE WAS REMOVED AND IS LOCATED IN THE STANDARD SPECIFICATIONS.



ABUTMENT PLAN WITHOUT WING EXTENSIONS

ABUTMENT BACKFILL PROCESS:

THE BASE OF THE EXCAVATION SUBGRADE BEHIND THE ABUTMENT IS TO BE GRADED WITH A 4% SLOPE AWAY FROM THE ABUTMENT FOOTING AND A 2% CROSS SLOPE IN THE DIRECTION OF THE SUBDRAIN OUTLET. THIS EXCAVATION SHAPING IS TO BE DONE PRIOR TO BEGINNING INSTALLATION OF THE GEOTEXTILE AND BACKFILL MATERIAL.

AFTER THE SUBGRADE HAS BEEN SHAPED THE GEOTEXTILE FABRIC SHALL BE INSTALLED IN ACCORDANCE WITH THE DETAILS SHOWN. THE FABRIC IS INTENDED TO BE INSTALLED IN THE BASE OF THE EXCAVATION AND EXTENDED VERTICALLY UP THE ABUTMENT BACKWALL, ABUTMENT WING WALLS, AND EXCAVATION FACE TO A HEIGHT THAT WILL BE APPROXIMATELY 1 TO 2 FOOT HIGHER THAN THE HEIGHT OF THE POROUS BACKFILL PLACEMENT AS SHOWN IN THE "GRANULAR BACKFILL DETAILS" ON THIS SHEET. THE STRIPS OF THE FABRIC PLACED SHALL OVERLAP APPROXIMATELY 1 FOOT AND SHALL BE PINNED IN PLACE. THE FABRIC SHALL BE ATTACHED TO THE ABUTMENT BY USING LATH FOLDED IN THE FABRIC AND SECURED TO THE CONCRETE WITH SHALLOW CONCRETE NAILS. THE FABRIC PLACED AGAINST THE EXCAVATION FACE SHALL BE PINNED.

WHEN THE FABRIC IS IN PLACE, THE SUBDRAIN SHALL BE INSTALLED DIRECTLY ON THE FABRIC AT THE TOE OF THE REAR EXCAVATION SLOPE. A SLOT WILL NEED TO BE CUT IN THE FABRIC AT THE POINT WHERE THE SUBDRAIN EXITS THE FABRIC NEAR THE END OF THE ABUTMENT WING WALL.

POROUS BACKFILL IS THEN PLACED AND LEVELED, NO COMPACTION IS REQUIRED.

THE REMAINING WORK INVOLVES BACKFILLING WITH GRANULAR BACKFILL, SURFACE FLOODING, AND VIBRATORY COMPACTION. THE GRANULAR BACKFILL MATERIAL SHALL HAVE 4% OR LESS PASSING THE #200 SIEVE (I.E. WASHED CONCRETE SAND). THE GRANULAR BACKFILL SHALL BE PLACED IN INDIVIDUAL LIFTS, SURFACE FLOODED, AND COMPACTED WITH VIBRATORY COMPACTION TO ENSURE FULL CONSOLIDATION. LIMIT THE LOOSE LIFTS TO NO MORE THAN 2 FEET OF THICKNESS.

START SURFACE FLOODING FOR EACH SAND LIFT AT THE HIGH POINT OF THE SUBDRAIN AND PROGRESS TO THE LOW POINT WHERE THE SUBDRAIN EXITS THE FABRIC. TO ENSURE UNIFORM SURFACE FLOODING, WATER RUNNING FULL IN A 2-INCH DIAMETER HOSE SHOULD BE SPRAYED IN SUCCESSIVE 6-FOOT TO 8-FOOT INCREMENTS FOR 5 MINUTES WITHIN EACH INCREMENT

LIFT PLACEMENT, FLOODING, AND COMPACTION SHALL PROGRESS UNTIL THE REQUIRED FULL THICKNESS OF THE ABUTMENT BACKFILL HAS BEEN COMPLETED.

WATER REQUIRED FOR FLOODING, SUBDRAINS, POROUS BACKFILL, GRANULAR BACKFILL, AND GEOTEXTILE FABRIC FURNISHED AT THE BRIDGE ABUTMENTS WILL NOT BE MEASURED SEPARATELY FOR PAYMENT.

THE COST OF WATER REQUIRED FOR FLOODING, SUBDRAINS, POROUS BACKFILL, GRANULAR BACKFILL, AND GEOTEXTILE FABRIC FURNISHED AT THE BRIDGE ABUTMENTS SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID FOR STRUCTURAL CONCRETE.

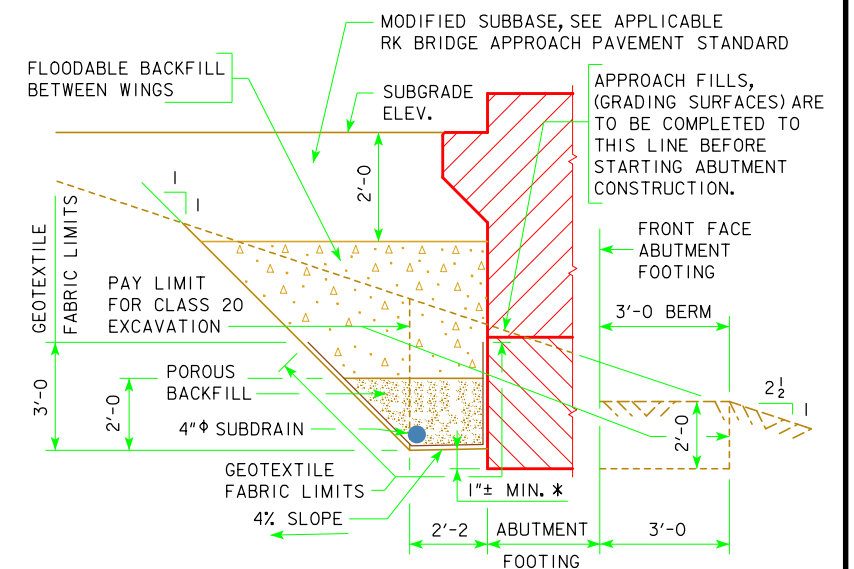
NOTE:
SEE SUBDRAIN DETAILS SHEET FOR DETAILS NOT SHOWN ON THIS SHEET WHICH ARE PERTINENT TO THIS STRUCTURE.

NOTE:

SUBDRAIN SHALL SLOPE DOWNWARD 2% FROM CL APPROACH ROADWAY WHEN OUTLETTING BOTH SIDES OF THE ABUTMENT.

SUBDRAIN SHALL SLOPE DOWNWARD 2% FROM HIGH END WHEN OUTLETTING AT ONE END OF THE ABUTMENT.

THE GEOTEXTILE FABRIC SHALL BE IN ACCORDANCE WITH ARTICLE 4196.01, B, 6 OF THE STANDARD SPECIFICATIONS. IF THE ENGINEERING FABRIC IS LAPPED THE LAPS SHALL BE A MINIMUM OF ONE FOOT IN LENGTH, SHINGLE FASHION WITH UP SLOPE LAP PIECE ON TOP AND STAPLED FOR CONTINUITY.



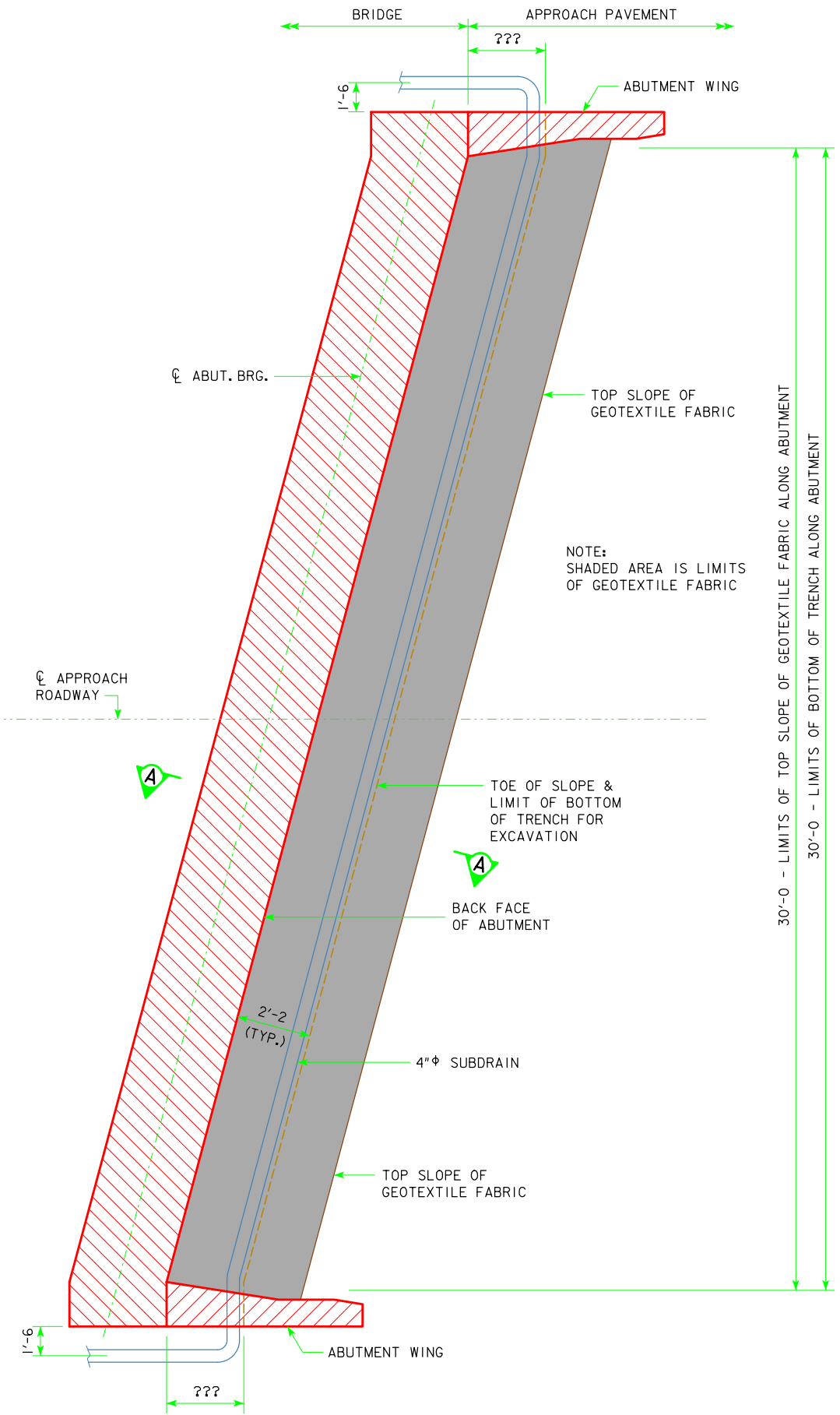
SECTION A-A
BACKFILL DETAILS

NOTE: GEOTEXTILE FABRIC WILL BE ATTACHED TO FACE OF ABUTMENT FOOTING AND WINGS.

* DIMENSION VARIES DUE TO 2% SUBDRAIN SLOPE.

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| 09-14 LATEST REVISION DATE | APPROVED BY BRIDGE ENGINEER | | |
| | | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| ABUTMENT BACKFILL DETAILS AT BACKFACE OF ABUTMENTS | | H30S1-43-12 | |

REVISED 09-14 - TECHNICAL DATA INFORMATION TABLE WAS REMOVED AND IS LOCATED IN THE STANDARD SPECIFICATIONS.



SKEWED ABUTMENT PLAN WITHOUT WING EXTENSIONS
(SKEWED LEFT AHEAD SHOWN, SKEWED RIGHT AHEAD SIMILAR)

ABUTMENT BACKFILL PROCESS:

THE BASE OF THE EXCAVATION SUBGRADE BEHIND THE ABUTMENT IS TO BE GRADED WITH A 4% SLOPE AWAY FROM THE ABUTMENT FOOTING AND A 2% CROSS SLOPE IN THE DIRECTION OF THE SUBDRAIN OUTLET. THIS EXCAVATION SHAPING IS TO BE DONE PRIOR TO BEGINNING INSTALLATION OF THE GEOTEXTILE AND BACKFILL MATERIAL.

AFTER THE SUBGRADE HAS BEEN SHAPED THE GEOTEXTILE FABRIC SHALL BE INSTALLED IN ACCORDANCE WITH THE DETAILS SHOWN. THE FABRIC IS INTENDED TO BE INSTALLED IN THE BASE OF THE EXCAVATION AND EXTENDED VERTICALLY UP THE ABUTMENT BACKWALL, ABUTMENT WING WALLS, AND EXCAVATION FACE TO A HEIGHT THAT WILL BE APPROXIMATELY 1 TO 2 FOOT HIGHER THAN THE HEIGHT OF THE POROUS BACKFILL PLACEMENT AS SHOWN IN THE "GRANULAR BACKFILL DETAILS" ON THIS SHEET. THE STRIPS OF THE FABRIC PLACED SHALL OVERLAP APPROXIMATELY 1 FOOT AND SHALL BE PINNED IN PLACE. THE FABRIC SHALL BE ATTACHED TO THE ABUTMENT BY USING LATH FOLDED IN THE FABRIC AND SECURED TO THE CONCRETE WITH SHALLOW CONCRETE NAILS. THE FABRIC PLACED AGAINST THE EXCAVATION FACE SHALL BE PINNED.

WHEN THE FABRIC IS IN PLACE, THE SUBDRAIN SHALL BE INSTALLED DIRECTLY ON THE FABRIC AT THE TOE OF THE REAR EXCAVATION SLOPE. A SLOT WILL NEED TO BE CUT IN THE FABRIC AT THE POINT WHERE THE SUBDRAIN EXITS THE FABRIC NEAR THE END OF THE ABUTMENT WING WALL.

POROUS BACKFILL IS THEN PLACED AND LEVELED, NO COMPACTION IS REQUIRED.

THE REMAINING WORK INVOLVES BACKFILLING WITH GRANULAR BACKFILL, SURFACE FLOODING, AND VIBRATORY COMPACTION. THE GRANULAR BACKFILL MATERIAL SHALL HAVE 4% OR LESS PASSING THE #200 SIEVE (I.E. WASHED CONCRETE SAND). THE GRANULAR BACKFILL SHALL BE PLACED IN INDIVIDUAL LIFTS, SURFACE FLOODED, AND COMPACTED WITH VIBRATORY COMPACTION TO ENSURE FULL CONSOLIDATION. LIMIT THE LOOSE LIFTS TO NO MORE THAN 2 FEET OF THICKNESS.

START SURFACE FLOODING FOR EACH SAND LIFT AT THE HIGH POINT OF THE SUBDRAIN AND PROGRESS TO THE LOW POINT WHERE THE SUBDRAIN EXITS THE FABRIC. TO ENSURE UNIFORM SURFACE FLOODING, WATER RUNNING FULL IN A 2-INCH DIAMETER HOSE SHOULD BE SPRAYED IN SUCCESSIVE 6-FOOT TO 8-FOOT INCREMENTS FOR 5 MINUTES WITHIN EACH INCREMENT

LIFT PLACEMENT, FLOODING, AND COMPACTION SHALL PROGRESS UNTIL THE REQUIRED FULL THICKNESS OF THE ABUTMENT BACKFILL HAS BEEN COMPLETED.

WATER REQUIRED FOR FLOODING, SUBDRAINS, POROUS BACKFILL, GRANULAR BACKFILL, AND GEOTEXTILE FABRIC FURNISHED AT THE BRIDGE ABUTMENTS WILL NOT BE MEASURED SEPARATELY FOR PAYMENT.

THE COST OF WATER REQUIRED FOR FLOODING, SUBDRAINS, POROUS BACKFILL, GRANULAR BACKFILL, AND GEOTEXTILE FABRIC FURNISHED AT THE BRIDGE ABUTMENTS SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE BID FOR STRUCTURAL CONCRETE.

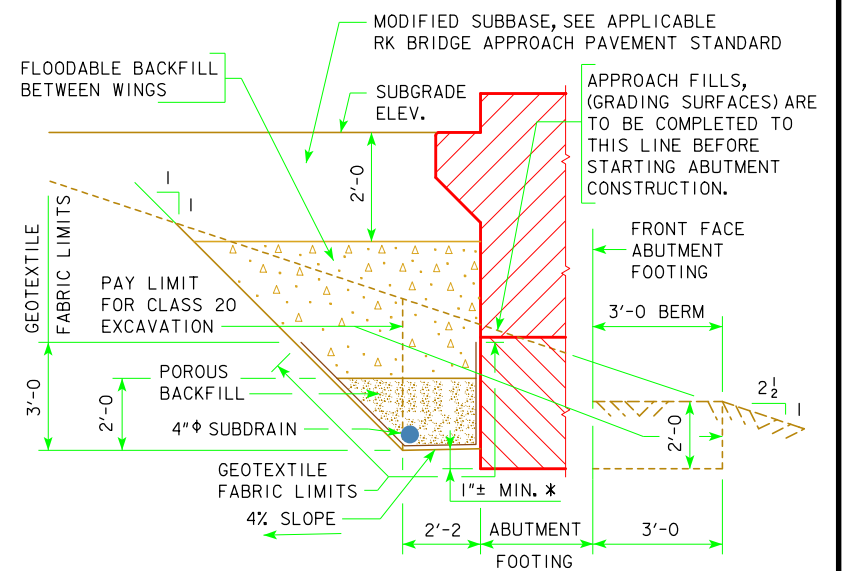
NOTE:
SEE SUBDRAIN DETAILS SHEET FOR DETAILS NOT SHOWN ON THIS SHEET WHICH ARE PERTINENT TO THIS STRUCTURE.

NOTE:

SUBDRAIN SHALL SLOPE DOWNWARD 2% FROM CL APPROACH ROADWAY WHEN OUTLETTING BOTH SIDES OF THE ABUTMENT.

SUBDRAIN SHALL SLOPE DOWNWARD 2% FROM HIGH END WHEN OUTLETTING AT ONE END OF THE ABUTMENT.

THE GEOTEXTILE FABRIC SHALL BE IN ACCORDANCE WITH ARTICLE 4196.01, B, 6 OF THE STANDARD SPECIFICATIONS. IF THE ENGINEERING FABRIC IS LAPPED THE LAPS SHALL BE A MINIMUM OF ONE FOOT IN LENGTH, SINGLE FASHION WITH UP SLOPE LAP PIECE ON TOP AND STAPLED FOR CONTINUITY.



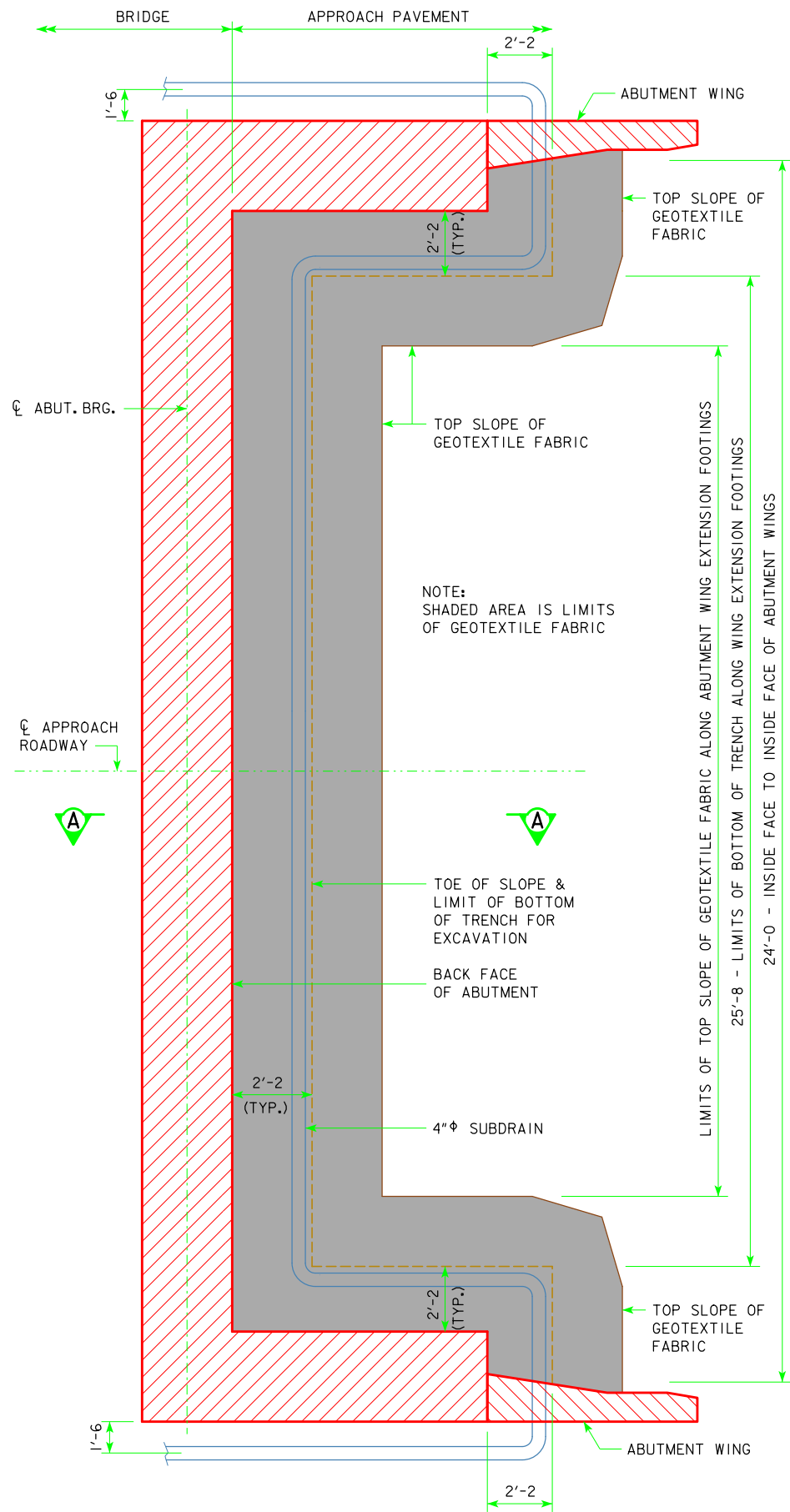
SECTION A-A
BACKFILL DETAILS

NOTE: GEOTEXTILE FABRIC WILL BE ATTACHED TO FACE OF ABUTMENT FOOTING AND WINGS.

* DIMENSION VARIES DUE TO 2% SUBDRAIN SLOPE.

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| 09-14 LATEST REVISION DATE <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | ABUTMENT BACKFILL DETAILS AT BACKFACE OF ABUTMENTS | H30SI-44-12 |

REVISED 09-14 - TECHNICAL DATA INFORMATION TABLE WAS REMOVED AND IS LOCATED IN THE STANDARD SPECIFICATIONS.



ABUTMENT PLAN WITH WING EXTENSIONS

ABUTMENT BACKFILL PROCESS:

THE BASE OF THE EXCAVATION SUBGRADE BEHIND THE ABUTMENT IS TO BE GRADED WITH A 4% SLOPE AWAY FROM THE ABUTMENT FOOTING AND A 2% CROSS SLOPE IN THE DIRECTION OF THE SUBDRAIN OUTLET. THIS EXCAVATION SHAPING IS TO BE DONE PRIOR TO BEGINNING INSTALLATION OF THE GEOTEXTILE AND BACKFILL MATERIAL.

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POROUS BACKFILL IS THEN PLACED AND LEVELED, NO COMPACTION IS REQUIRED.

THE REMAINING WORK INVOLVES BACKFILLING WITH GRANULAR BACKFILL, SURFACE FLOODING, AND VIBRATORY COMPACTION. THE GRANULAR BACKFILL MATERIAL SHALL HAVE 4% OR LESS PASSING THE #200 SIEVE (I.E. WASHED CONCRETE SAND). THE GRANULAR BACKFILL SHALL BE PLACED IN INDIVIDUAL LIFTS, SURFACE FLOODED, AND COMPACTED WITH VIBRATORY COMPACTION TO ENSURE FULL CONSOLIDATION. LIMIT THE LOOSE LIFTS TO NO MORE THAN 2 FEET OF THICKNESS.

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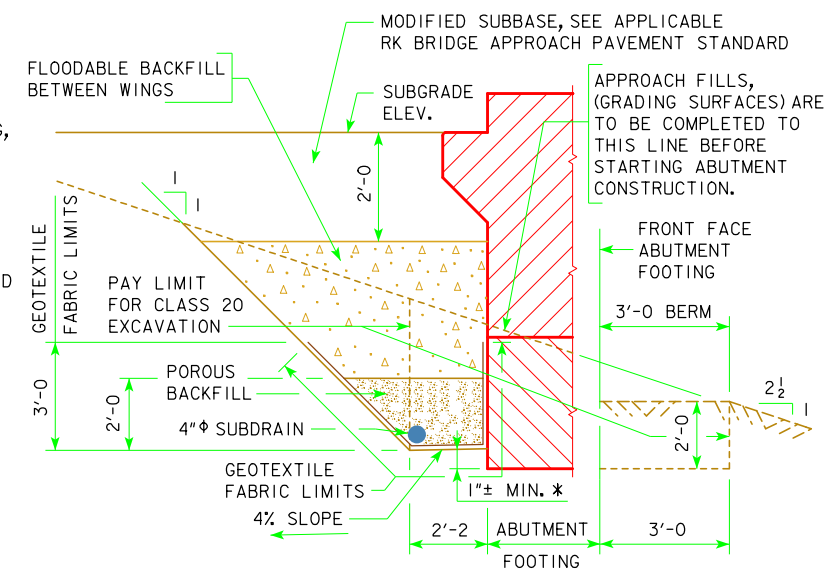
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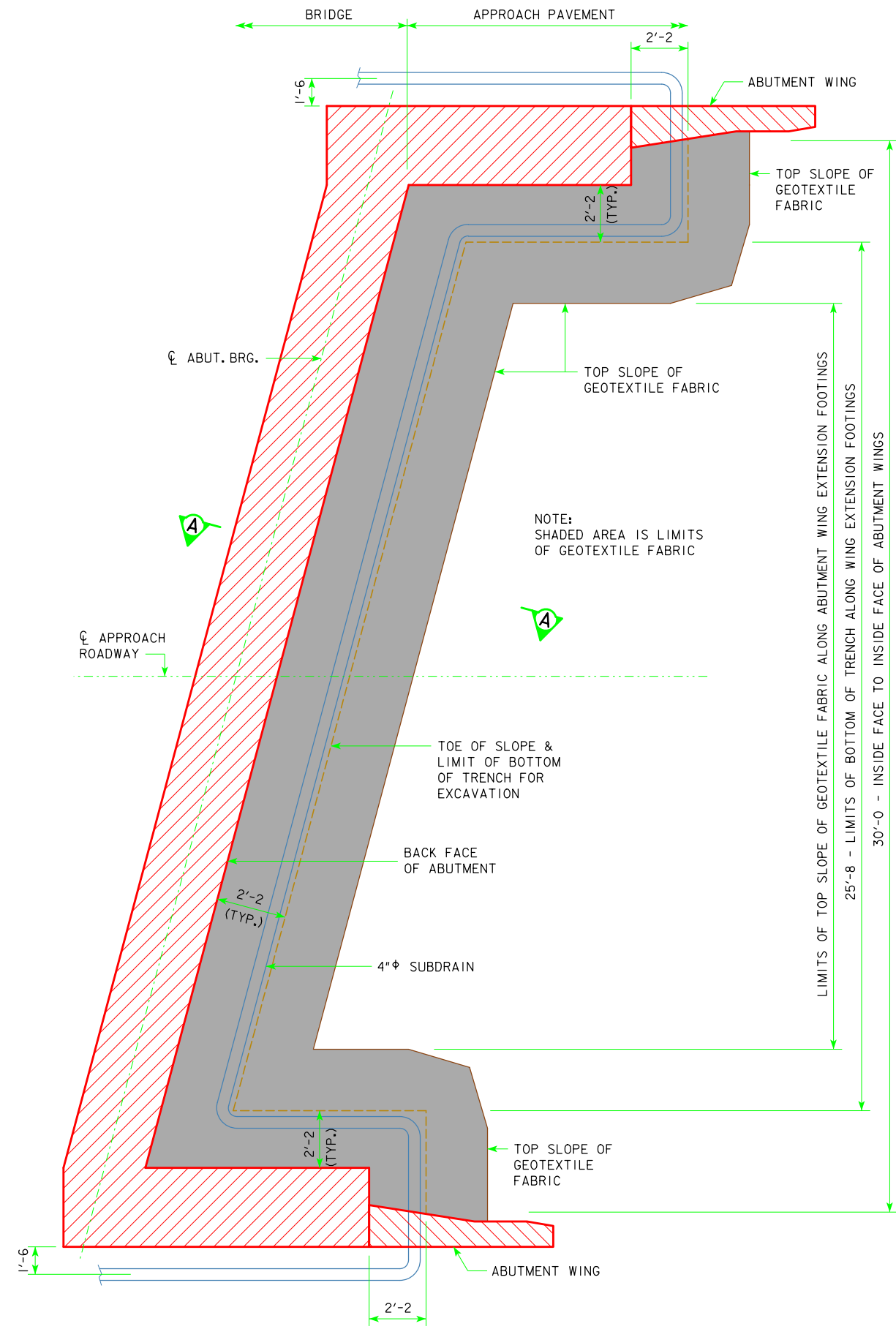
**SECTION A-A
BACKFILL DETAILS**

NOTE: GEOTEXTILE FABRIC WILL BE ATTACHED TO FACE OF ABUTMENT FOOTING AND WINGS.

* DIMENSION VARIES DUE TO 2% SUBDRAIN SLOPE.

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| 09-14 LATEST REVISION DATE <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | ABUTMENT BACKFILL DETAILS AT BACKFACE OF ABUTMENTS | H30SI-45-12 |

REVISED 09-14 - TECHNICAL DATA INFORMATION TABLE WAS REMOVED AND IS LOCATED IN THE STANDARD SPECIFICATIONS.



ABUTMENT BACKFILL PROCESS:

THE BASE OF THE EXCAVATION SUBGRADE BEHIND THE ABUTMENT IS TO BE GRADED WITH A 4% SLOPE AWAY FROM THE ABUTMENT FOOTING AND A 2% CROSS SLOPE IN THE DIRECTION OF THE SUBDRAIN OUTLET. THIS EXCAVATION SHAPING IS TO BE DONE PRIOR TO BEGINNING INSTALLATION OF THE GEOTEXTILE AND BACKFILL MATERIAL.

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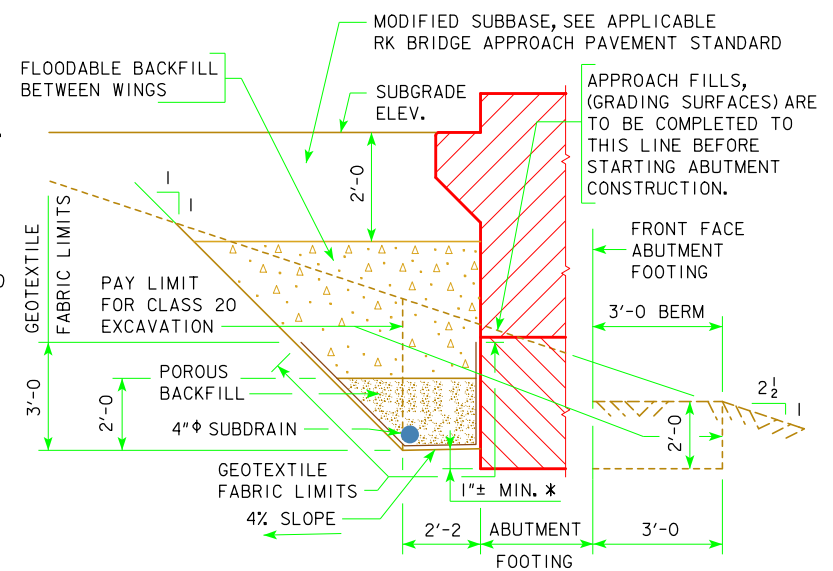
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NOTE:

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SUBDRAIN SHALL SLOPE DOWNWARD 2% FROM HIGH END WHEN OUTLETTING AT ONE END OF THE ABUTMENT.

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**SECTION A-A
BACKFILL DETAILS**

NOTE: GEOTEXTILE FABRIC WILL BE ATTACHED TO FACE OF ABUTMENT FOOTING AND WINGS.

* DIMENSION VARIES DUE TO 2% SUBDRAIN SLOPE.

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| 09-14 LATEST REVISION DATE <i>Thomas L. Mc Donald</i> APPROVED BY BRIDGE ENGINEER | | |
| | STANDARD DESIGN - 30' ROADWAY, SINGLE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES APRIL, 2012 | |
| | ABUTMENT BACKFILL DETAILS AT BACKFACE OF ABUTMENTS | H30S1-46-12 |